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# An integrated method for assessing vulnerability of buildings caused by debris flows

in mountainous areas

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  - Abstract: The vulnerability assessment of buildings in future scenarios is critical to decrease potential losses caused by debris flows in mountainous areas due to the complex topographical condition that could increase the environmental vulnerability to climate change. However, the lack of reliable methods limits the accurate estimation of physical damage and the associated economic loss. Therefore, an integrated method of physical vulnerability matrix and machine learning model was developed to benefit the estimation of damage degree of buildings caused by a future debris-flow event. By considering the building structures (reinforced-concrete (RC) frame and non-RC frame), spatial positions between buildings and the debris-flow channels (horizontal distance (HD) and vertical distance (VD)), and impact pressure  $(P_1)$  to buildings, a physical vulnerability matrix was proposed to link physical damage with the four factors. In order to overcome the difficulty in estimating the possible impact pressure to buildings, an ensemble machine learning (ML) model (XGBoost) was developed with the involvement of geological factors. Additionally, the HD and VD were decided based on the satellite images. The Longxihe Basin, Sichuan, China was selected as a case study. The results show that the ML model can achieve a reliable impact pressure prediction because the mean absolute percentage error (MAPE), root mean squared error (RMSE), and mean absolute error (MAE) values are 9.53%, 3.78 kPa, and 2.47 kPa. Furthermore, 13.9% of buildings in the Longxihe Basin may suffer severe damage caused by a future

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- 24 debris-flow event, and the highest economic loss is found in a residential building, reaching  $5.1 \times 10^5$  €.
- 25 Overall, our work can provide scientific support for the site selection of future constructions.
- 26 Keywords: Debris flow, geological factors, building, machine learning, vulnerability assessment



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#### 1. Introduction

difficulty in timely warning (Santi et al., 2011). These events can devastate entire settlements in their path and pose significant threat to natural environment (Immerzeel et al., 2020), causing destruction of aquatic biodiversity, along with damage to properties and finally leading to considerable economic losses worldwide each year (Qiu et al., 2022; Alene et al., 2024; Sridharan et al., 2024). In European Alps, this disaster claimed an economic loss of at least 5 € billion from 1988 to 2012 (Fuchs, 2009; Guzzetti et al., 2005). Moreover, a similar average annual loss is also found in China, approximately 0.17 € billion of annual loss was recorded during the time period of 2005 and 2015 (Miao and Liu, 2020). In this case, a reliable estimation of the potential economic loss caused by debris flows is essential since it can provide guidance for decision-makers about where to place the infrastructures and buildings. The buildings are the most susceptible element to debris flows, and they are responsible for most of the economic loss (Fuchs, 2009; Wei et al., 2018). Therefore, in order to calculate the potential economic loss, it is critical to estimate the damage degree of the buildings since economic loss is linked to the physical vulnerability of a property and its economic value. The physical vulnerability quantifies the damage degree of a property, and the methods that are used to decide the physical vulnerability include mechanical method (Ruggieri et al., 2023, 2022), vulnerability matrices, vulnerability curves, vulnerability indicators (Papathoma-Köhle et al., 2017). The mechanical methods derive the vulnerability functions of buildings based on numerical models, which may achieve relatively high evaluation accuracy but highly rely on controlled laboratory experiments to obtain input data. As a result, this method itself is time-consuming and costly for regional application (Paudel et al., 2021; Qiu et al., 2022). Three vulnerability curves were derived using numerical modelling to relate the vulnerability to debris-flow intensity, including flow height, flow velocity, and

Debris flows are among the most frequent and costly natural hazards due to climate change and

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kinematic viscosity (Quan Luna et al., 2011). Although these three curves can suggest the physical vulnerability of a building at risk but fail to consider the impacts of building structures on damage degree. Therefore, a brick structure and a reinforced-concrete frame were included in the development of vulnerability curves by Zhang et al. (2018). However, the involvement of limited building types restricts the application of the curves when the determination of physical vulnerabilities for different building types is required. Therefore, considering the limitations of vulnerability curves, different vulnerability matrixes of buildings have also been developed by many studies due its advantages in interaction understanding between the debris-flow process and elements at risk and easily readable by non-experts (Bründl et al., 2009; Kang and Kim, 2016; Zanchetta et al., 2004). In contrast, these developed matrixes ignored the spatial position (horizontal distance and vertical distance) between the buildings and the debris-flow channels, which would misestimate the damage degree of a building caused by a debris-flow event. As for the vulnerability indicators, this method considers the characteristics of buildings without relating the debris-flow process when evaluating the damage degrees (Fuchs et al., 2019). Therefore, it is crucial to establish a comprehensive assessment matrix that takes into account the structural types, spatial positions between buildings and the debris-flow channels, and debris-flow intensities to estimate the potential damages of the buildings. Additionally, the possible damage degree of the buildings in future scenarios was not considered by the past studies (Papathoma-Köhle et al., 2017). Therefore, this study focuses on conducting an assessment of the potential physical damage of a building due to a future debris-flow event. Among the four factors in deciding the physical damage of buildings (building structure, spatial locations (HD and VD), and impact pressure  $(P_t)$ ), impact pressure remains an unsolved problem since HD and VD can be determined based on the satellite images. In this case, a machine learning model was developed to predict the impact pressure to a building because this method can uncover intricate and

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concealed relationships between various input variables and an output result (Khosravi et al., 2021; Jiang 73 74 et al., 2023). To leverage the benefits of rapid processing and handling large-scale data, we employ an ensemble model, specifically extreme gradient boosting (XGBoost). This choice is made due to 75 XGBoost's ability to partition data into smaller components, facilitating parallel computation and 76 multithreading to enhance processing speed (Chen and Guestrin, 2016). 77 78 In this paper, we proposed an integrated method of physical vulnerability matrix and machine learning model to estimate the physical damage of a building caused by a future debris-flow event, 79 finally estimating the economic loss of this property. The buildings in the Longxihe Basin, Sichuan, 80 Chian, were extracted to conduct a case study to test the efficiency and reliability of this method in 81 82 physical damage estimation and corresponding economic loss. The formation of terrain in this area is 83 affected by severe tectonic activities, which can produce abundant loose materials for potential debris

## 2. Methodology

flows.

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To estimate the economic loss of buildings caused by a future debris-flow event, several steps are comprised in this study (see Fig. 1):



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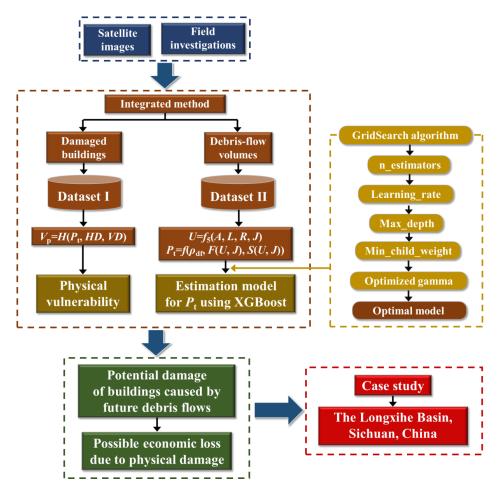


Figure 1. Flow chart of this study

- (1) The historical debris-flow events in Gyirong, Tibet Tibetan Autonomous Region, and the Sichuan Basin (Fig. 2) from the past ten years are investigated based on satellite images and field investigations to collect information regarding the debris-flow volumes and damaged buildings.
- 93 (2) We categorize the collected historical debris flows into two datasets (dataset I and dataset II) for 94 the development of a physical vulnerability matrix and a prediction model, respectively.
- 95 (3) The dataset I includes the debris-flow events that caused damages to the buildings. Therefore, 96 this dataset is employed for the development of a physical vulnerability matrix.
  - (4) The dataset II is composed of the debris-flow events that occurred in areas without the





distribution of buildings, and, therefore, no property loss is caused by these events. Therefore, this dataset was used for model training and utilize this model to estimate the debris-flow intensity in future scenarios, such as debris-flow impact pressure to buildings. This dataset is shown in Table 6 of Appendix. A.

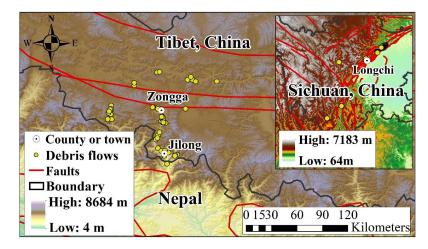


Figure 2. The collected historical debris flows in the Tibet Plateau and the Sichuan Basin

### 2.1 Physical vulnerability matrix

Vulnerability refers to the damage degree of a property when subjected to a hazard event, such as a landside and a debris-flow event (Fell, 1994). Normally, a physical vulnerability was used to quantify the damage degree of a property. To obtain the future economic loss of a building at risk, a physical vulnerability matrix of the buildings was proposed. The determination of physical vulnerability relied on the impact pressure ( $P_t$ ) to buildings, the horizontal distance (HD), and the vertical distance (VD) between the building and the nearest debris-flow channel, as indicated by Eq. (1). The determination details of the three parameters in Eq. (1) are demonstrated in the following sections.

$$V_{p} = H\left(P_{t}, HD, VD\right) \tag{1}$$

### 2.1.1 Calculation of impact pressure

In order to propose a physical vulnerability matrix, the first step is to link the impact pressure to





damage degree. As suggested by (Jakob et al., 2012; Kang and Kim, 2016),  $P_1$  can effectively reflect the energy of debris flows and possible damage degree of buildings. However, past studies usually utilized debris-flow magnitude to decide the physical vulnerability since a greater magnitude may indicate a more significant impact force (Dai et al., 2002). This impact force cannot represent the actual damage of a building during a debris-flow event because the catchment with a potential large-scale debris-flow event may not cause severe damage to the buildings. The reason behind this uncertainty could be due to the moderate slope gradient and frictional resistance of a debris-flow channel, which could decrease the kinetic energy of the travelling mass. Consequently, only a slight or moderate damage to buildings could be caused. Therefore, impact pressure can better reflect the damage degree of buildings when subjected to different debris-flow magnitudes, which can be calculated through considering the dynamic overpressure and hydrostatic pressure (Eq. (2)) (Zanchetta et al., 2004):

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$$P_{t} = \frac{1}{2} \rho_{df} g h + \rho_{df} v^{2} = f \left( \rho_{df}, h, v \right)$$
 (2)

where  $P_t$  (kPa) represents the impact pressure to buildings, and g is the gravitational acceleration. v (m/s) represents the mean flow velocity, and  $\rho_{df}$  is the mean density of materials for a debris-flow event. h (m) is the flow depth. As for the debris-flow velocity (v) at peak discharge, it can be calculated using the equation proposed by Rickenmann (1999). This equation considers the debris-flow datasets in different regions, such as Italy, China, Japan, U.S.A, and Columbia, which enables its feasibility to be used in wider and different areas.

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$$v = 2.1Q_p^{0.33}J^{0.33} = f_1(Q_p, J)$$
 (3)

This equation illustrates that the velocity can be decided by  $Q_P$  (m<sup>3</sup>/s) and channel gradient (J) (Cui et al., 2013). The calculation of  $Q_P$  can be determined based on the equation (Eq. (4)):

136 
$$Q_p = (U/152.97)^{1/1.266} = f_2(U)$$
 (4)

Therefore, the  $Q_p$  can be calculated based on the estimated volume (U (m<sup>3</sup>)) of historical debris



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- 138 flows. However, the absence of flow depth (h) also hampers the calculation of impact pressure.
- 139 Therefore, an equation is used to calculate the flow depth (Koch, 1998). This formula has been proven to
- perform well in the numerical simulation of viscose debris flows (Eq. (5)):

141 
$$h = (v/C_1J^{0.5})^{10/3} = f_3(v,C_1,J) = f_3(f_1(Q_p,J),C_1,J)$$
 (5)

- where  $C_1$  represents the dimensional empirical coefficient. This value of parameter is indicated by a
- semi-theoretical relationship (Eq. (6)) (Rickenmann, 1999):

$$C_1 = 10Q_p^{2/25} = f_4(Q_p) = f_4(f_2(U))$$
(6)

- Therefore, the impact pressure can be described as a function of debris-flow volume and channel
- gradient, and the impact pressures of dataset I are calculated based on Eqs. (2)-(6) (see Table 1).

### 2.1.2 Determination of HD and VD values

HD and VD values were also introduced here since the actual damage will be significant if a building stands close to the debris-flow channel (Sturm et al., 2018). They can be estimated through high-resolution satellite images, such as Gaofen, Ziyuan, WorldView, and GeoEye. In this study, Gaofen-2 satellite images are employed for determining the HD and VD values. This satellite can capture panchromatic (black and white) images with a spatial resolution reaching 0.8 m and multispectral (color) images with a spatial resolution up to 3.2 m. Therefore, the resolution of satellite images used for buildings identification is 0.8 m. As for the building clusters that are hard to be separated into individual buildings manually, a 'fishnet' tool in GIS was used to automatically divide these clusters into building segments. Furthermore, the rectangle segments were converted into points so that each point represents a building. As a result, the HD and VD values of a building can be decided. The damaged buildings are mainly distributed on the accumulation fans. Therefore, even though a large HD is observed, the VD is small due to the mild slope and smooth topography of the alluvial fans (Marcato et al., 2012). By considering the impact pressure, HD, and VD values, a physical vulnerability



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matrix can be established to evaluate the physical damage of a building caused by a debris-flow event.

## 2.2 Economic loss of a building at risk

The economic loss of a building caused by a debris-flow event can be estimated based on multiplication of its physical vulnerability and economic value.

$$V_{e} = V_{p} \times M = H(P_{t}, HD, VD) \times M; M = P \times A$$
(7)

where,  $V_e$  and M represent the economic loss and the economic value of a building, respectively. P is the unit price of a building, and A represents the area of a building. Therefore, estimating  $V_p$  holds paramount importance in estimating economic loss. However,  $V_p$  ( $H(P_t, HD, VD)$ ) is represented by the proposed physical vulnerability matrix. In this context, determining  $P_t$  plays a critical role in economic loss estimation. Therefore, to forecast the possible economic loss caused by a future debris-flow event, we need to estimate the impact pressure to buildings caused by a future debris-flow event.

## 2.3 Prediction model development

To predict the future impact pressure to buildings when a debris-flow event occurs, determining factors is essential. Therefore, we further developed Eq. (5) by integrating Eq. (4) and Eq. (6) to this equation:

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$$h = f_3(f_1(f_2(U),J), f_4(f_2(U)), J) = F(U,J)$$
 (8)

177 Additionally, Eq. (3) can be rewritten as:

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$$v = f_1(Q_p, J) = f_1(f_2(U), J) = S(U, J)$$
 (9)

Therfore, the determination of impact pressure reslies on U and J:

$$P_{t} = f\left(\rho_{dt}, F\left(U, J\right), S\left(U, J\right)\right) \tag{10}$$

However, the debris-flow volume is closely related to a set of geomorphic factors, as suggested by Huang et al. (2020). They are catchment area (A), channel length (L), topographic relief (R), and mean slope of the main channel (J). The catchment area can reflect the debris availability and capacity of





generating and containing the volume of loose materials for a debris-flow catchment. As for the channel length, it is related to the entrained and transported sediment volume (Marchi et al., 2019). Therefore, this parameter can also impact the final volume of a debris-flow event. R is defined as the terrain fluctuation and roughness of a catchment. To calculate this value, we need to first decide the optimal statistical unit in this area using the change-point model. Then, the subtraction value between the maximum value and minimum values of an optimal statistical unit is calculated. Finally, we utilized the maximum subtraction value to represent the R value of a catchment. J is defined as the ratio of the elevation difference of the main channel and channel length. A longer distance could be achieved for a debris-flow event if a steep channel exists in a catchment (de Haas and Densmore, 2019). In this case, U can be described as a function of A, L, R, and J:

$$U = f_5(A, L, R, J) \tag{11}$$

Furthermore, substituting Eq. (11) to Eq. (10):

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$$P_{t} = f\left(\rho_{df}, F\left(f_{5}\left(A, L, R, J\right), J\right), S\left(f_{5}\left(A, L, R, J\right), J\right)\right)$$
(12)

Therefore,  $P_t$  can be described as a complex function of geomorphology-related factors, including A, L, R, and J. To find the complicated correlations among them, an ensemble machine learning model (extreme gradient boosting (XGBoost)) was employed here to establish the relationship and then utilize this relationship to estimate the potential impact pressure to buildings when a future debris-flow event occurs. The basic mechanism of XGBoost is to constantly develop a new decision tree which acts as a weak learner and fits the residual error of the last prediction. After the training of a total of k trees, the final prediction result is the sum of the score of each leaf node in each developed tree. Overall, the target function of regression is placed in Appendix. A. Additionally, the database II that is used for impact pressure prediction is presented in Table 6 of Appendix. A.

## 2.4 Model assessment





After the impact pressure prediction, three assessment indexes were used to evaluate the prediction performance, including MAPE (Mean Absolute Percentage Error), RMSE (Root Mean Square Error), and MAE (Mean Absolute Error):

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$$MAPE = \frac{1}{m} \sum_{i=1}^{m} \frac{|y_i - y_{ipre}|}{y_i}$$
 (13)

$$RMSE = \sqrt{\frac{1}{m} \sum_{i=1}^{m} (y_i - y_{ipre})^2}$$
 (14)

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$$MAE = \frac{1}{m} \sum_{i=1}^{m} |y_i - y_{ipre}|$$
 (15)

where  $y_i$  is the actual value, and  $y_{ire}$  represents the prediction value. m is the number of prediction values.

#### 3. Result analysis

### 3.1 The relationship between the damage degree and $P_t$

Fig. 3 shows the different damage degrees of buildings in dataset I. The buildings were classified into two types, including RC-frame (reinforced concrete) and non-RC frame (masonry, wooden structure, and light steel frame). As indicated in Figs. 3(e)-(f), The masonry buildings suffer severe damage, and the light steel frame buildings and wooden structure buildings are destroyed (Figs. 3(g)-(h)) even though the impact pressure to buildings was estimated to be less than 30 kPa. However, the main structure of the reinforced concrete building can stay undamaged (Fig. 3(b)) when severe damage is found on the masonry structure (see a dashed circle in Fig. 3(b)) during the same debris-flow event. This resistance ability difference indicates the difference in physical vulnerabilities between the RC and the non-RC frames, which can also be seen in Fig. 3(a). Moreover, moderate damage to the RC frame with unreinforced masonry infill walls is found in Fig. 3(c) when a small-scale debris-flow event occurs. Additionally, the RC frame suffers extensive damage when the impact pressure exceeds 100 kPa based on the estimated debris-flow volume. Therefore, the identifications of different damage degrees for buildings provide us with access to proposing a classification standard for the physical vulnerability of



# 229 buildings.

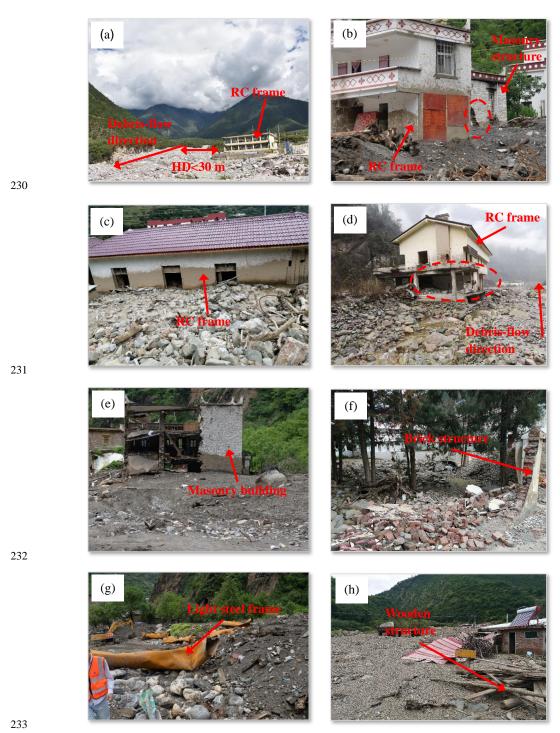


Figure 3. Photographs of the damaged residential buildings caused by debris flows during the field





investigations on the Qinghai-Tibetan Plateau

## 3.2 Determination of HD and VD thresholds

The field investigations and statistical results show that the non-RC frame buildings are destroyed or suffer structural damage when the *HD* is less than 30 m (Fig. 4(a)). The damaged buildings cannot be repaired, and reconstruction is required. In consistent with the conclusion of past study (Wei et al., 2022), the residential buildings, such as brick structures (Fig. 4(b)) and the RC frame buildings (Fig. 4(c)), are partially buried by the debris-flow sediments without structural damage when the *HD* is greater than 100 m but less than 160 m. Therefore, 160 m is another *HD* threshold to classify the inundated and slightly affected areas. The upper limit of *HD* value for the historical debris flows during the field investigations is 230 m because almost 94% of *HD* values are less than 230 m (see Table 1).











Figure 4. Examples of the determination of the HD thresholds

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Table 1. Dataset I for physical vulnerability matrix.

| No. | Year | Lon<br>(°) | Lat<br>(°) | Number<br>of         | Impact pressure   | Maximum HD (m) | Maximum VD (m) |
|-----|------|------------|------------|----------------------|-------------------|----------------|----------------|
|     |      | ( )        | ( )        | damaged<br>buildings | $P_{\rm t}$ (kPa) | 112 ()         | , 2 (m)        |
| 1   | 2006 | 85.3278    | 28.3735    | 21                   | 16.1              | 162            | 12             |
| 2   | 2007 | 85.5683    | 29.1875    | 13                   | 40.6              | 141            | 12             |
| 3   | 2007 | 85.5528    | 28.8717    | 7                    | 37.5              | 13             | 7              |
| 4   | 2008 | 85.6241    | 29.1869    | 21                   | 41.0              | 119            | 3              |
| 5   | 2010 | 86.0872    | 29.1625    | 11                   | 35.5              | 54             | 2              |
| 6   | 2013 | 85.3112    | 28.7649    | 53                   | 24.1              | 284            | 29             |
| 7   | 2015 | 85.2928    | 28.4174    | 9                    | 117.4             | 160            | 2              |
| 8   | 2015 | 85.3608    | 28.4074    | 22                   | 31.1              | 131            | 107            |
| 9   | 2015 | 85.3542    | 28.7159    | 7                    | 17.5              | 82             | 13             |
| 10  | 2015 | 84.7653    | 28.7559    | 38                   | 132               | 74             | 15             |
| 11  | 2015 | 85.4566    | 28.3868    | 3                    | 5.1               | 32             | 10             |
| 12  | 2015 | 85.4413    | 28.3827    | 1                    | 32.7              | 17             | 6              |
| 13  | 2015 | 85.0105    | 29.1208    | 3                    | 5.2               | 133            | 2              |
| 14  | 2015 | 85.2579    | 29.2603    | 9                    | 9.8               | 146            | 2              |
| 15  | 2015 | 85.2759    | 29.2652    | 6                    | 14.8              | 228            | 10             |
| 16  | 2015 | 85.0083    | 29.1493    | 4                    | 14.6              | 171            | 3              |

In order to support the thresholds determination of HD, we further analyzed the frequencies of HD

values for the damaged buildings, as depicted in Table 1, through dividing the HD values into several

intervals. The frequency and accumulative frequency results are shown in Fig. 5.





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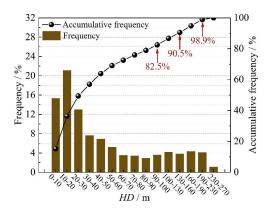


Figure 5. The frequency and accumulative frequency distributions of the 228 damaged buildings.

As depicted in Fig. 5, the highest proportion occurs in the range of 10 to 20 m, accounting for 20.1%, followed by a 15% percentage of HD values falling between 20 to 30 m. Therefore, the proportion falling within the range of 0 to 30 m is 49.4%, and approximately 82.5% of the HD values is measured under 100 m. Following the suggestion of Liu et al. (2020), a probability of 50% is considered a threshold for debris-flow warning, which implies that 30 m in this study can serve as a threshold. Moreover, the accumulative frequency of 80% is selected as another threshold based on Wei et al. (2018), corresponding to the HD value of 100 m. Furthermore, 90.5% of the damaged buildings have HD value less than 160 m, and nearly 98.9% of the damaged buildings fall within the HD range of 0 to 230 m. As a result, 160 m and 230 m are selected as additional two thresholds. In addition to the determination of HD threshold values, the maximum flow depth (h) in the debris-flow channel is used as a reference to decide the VD thresholds since the buildings are mostly situated along the channels (Fig. 4(a) and Fig. 6). Therefore, calculating the elevation difference between the buildings and the nearest debris-flow channel is critical to evaluate the safety of the buildings. For example, both the masonry buildings in Fig. 4(a) and Fig. 6 are close to the debris-flow channel. However, no severe damage is observed for the building in Fig. 6 because it has a considerable vertical distance from the main channel. To decide the VD thresholds, the h values of the historical debris flows are presented in Table 6 of Appendix. A. The





average depth of the debris flows is 2.6 m, and nearly all the VD values are less than 4 m. Therefore, 4 m serves as the first threshold, suggesting that the most severe damage to the buildings may be caused when the VD is less than 4 m. Whilst a debris-flow depth value of as high as 10 m is suggested (Xie et al., 2013), which can be found in curved channels. Consequently, we utilize 10 m to indicate the moderate damage of buildings when the VD is less than 10 m but greater than 4 m. Moreover, a vertical distance of 14 m above the river level is considered to record the river gauging on the Iowa River using a digital video camera (Creutin et al., 2003), which indicates a safe VD value to avoid damage caused by the river discharge. Therefore, 15 m is used as the upper limit of the VD values in this paper.



Figure 6. Example of the determination of the VD threshold

### 3.3 Physical vulnerability matrix ( $h(P_t, HD, VD)$ )

The proposed physical vulnerabilities of residential buildings are listed in Table 2. Extensive damage or even complete damage may occur when a non-RC building is located near the debris-flow channel with HD less than 30 m and VD less than 4 m. However, a significant improvement in resistance ability can be observed when the non-RC frame is replaced by the RC frame considering the same impact pressure, HD and VD values. In general, the buildings hardly suffer damage when the VD is



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greater than 10 m. Therefore, the economic loss of a building can be calculated based on the proposed physical vulnerabilities and economic values.

Table 2. Physical vulnerability matrix

|                                 | Table 2. Physical vulnerability matrix   |   |  |                                  |  |   |                    |  |  |  |
|---------------------------------|--|---|--|----------------------------------|--|---|--------------------|--|--|--|
| $P_{t}$                         | Building   | HD<3  | 0 m  |                                  | 30 < <i>l</i>                                    | <i>HD</i> < 100 m   |                    |  |  |  |
| (kPa)                           | structure  | 4<  | 4< <i>VD</i> <   | 10< <i>VD</i> <                  | 4<   | 4< <i>VD</i> <  | 10< <i>VD</i> <    |  |  |  |
|                                 |  | VD  | 10   | 15                               | VD   | 10  | 15                 |  |  |  |
| <30                             | RC frame   | 0.3   | 0.2  | 0.1                              | 0.2  | 0.1   | /                  |  |  |  |
|                                 | Non-RC   | 0.8   | 0.7  | 0.6                              | 0.7  | 0.6   | 0.4                |  |  |  |
|                                 | frame  |   |  |                                  |  |   |                    |  |  |  |
| 30-70                           | RC frame   | 0.6   | 0.5  | 0.4                              | 0.5  | 0.4   | 0.2                |  |  |  |
|                                 | Non-RC   | 1   | 0.9  | 0.8                              | 0.9  | 0.8   | 0.6                |  |  |  |
|                                 | frame  |   |  |                                  |  |   |                    |  |  |  |
| 70-100                          | RC frame   | 0.7   | 0.6  | 0.5                              | 0.6  | 0.5   | 0.3                |  |  |  |
|                                 | Non-RC   | 1   | 1  | 0.9                              | 1  | 0.9   | 0.7                |  |  |  |
|                                 | frame  |   |  |                                  |  |   |                    |  |  |  |
| >100                            | RC frame   | 0.8   | 0.7  | 0.6                              | 0.7  | 0.6   | 0.4                |  |  |  |
|                                 | Non-RC   | 1   | 1  | 0.9                              | 1  | 1   | 0.8                |  |  |  |
|                                 | C  |   |  |                                  |  |   |                    |  |  |  |
|                                 | frame  |   |  |                                  |  |   |                    |  |  |  |
| $P_{t}$                         | Building Building  | 100 <f< td=""><td><i>HD</i>&lt;160 m</td><td></td><td>160&lt;</td><td><i>HD</i>&lt;230 m</td><td></td></f<>   | <i>HD</i> <160 m   |                                  | 160<   | <i>HD</i> <230 m  |                    |  |  |  |
| P <sub>t</sub> (kPa)            |  | 100 <f< td=""><td>#D&lt;160 m<br/>4<vd<< td=""><td>10&lt;<i>VD</i>&lt;</td><td>160&lt;<br/>4&lt;</td><td>HD&lt;230 m<br/>4<vd<< td=""><td>10&lt;<i>VD</i>&lt;</td></vd<<></td></vd<<></td></f<> | #D<160 m<br>4 <vd<< td=""><td>10&lt;<i>VD</i>&lt;</td><td>160&lt;<br/>4&lt;</td><td>HD&lt;230 m<br/>4<vd<< td=""><td>10&lt;<i>VD</i>&lt;</td></vd<<></td></vd<<> | 10< <i>VD</i> <                  | 160<<br>4<                                       | HD<230 m<br>4 <vd<< td=""><td>10&lt;<i>VD</i>&lt;</td></vd<<>   | 10< <i>VD</i> <    |  |  |  |
| -                               | Building   |   |  | 10< <i>VD</i> < 15               |  |   | 10< <i>VD</i> < 15 |  |  |  |
| -                               | Building   | 4<  | 4< <i>VD</i> <   |                                  | 4<   | 4< <i>VD</i> <  |                    |  |  |  |
| (kPa)                           | Building<br>structure  | 4<<br><i>VD</i>   | 4< <i>VD</i> <   |                                  | 4<   | 4< <i>VD</i> <  |                    |  |  |  |
| (kPa)                           | Building structure  RC frame   | 4<<br><i>VD</i><br>0.1  | 4< <i>VD</i> <<br>10   | 15                               | 4<<br><i>VD</i><br>/                             | 4< <i>VD</i> <<br>10  | 15                 |  |  |  |
| (kPa)                           | Building structure  RC frame Non-RC  | 4<<br><i>VD</i><br>0.1  | 4< <i>VD</i> <<br>10   | 15                               | 4<<br><i>VD</i><br>/                             | 4< <i>VD</i> <<br>10  | 15<br>/            |  |  |  |
| (kPa)                           | Building structure  RC frame Non-RC frame  | 4< <i>VD</i> 0.1 0.6  | 4< <i>VD</i> <<br>10<br>/<br>0.4   | 15                               | 4<<br><i>VD</i><br>/<br>0.4                      | 4< <i>VD</i> <<br>10  | 15 /               |  |  |  |
| (kPa)<br><30<br>30-70           | Building structure  RC frame Non-RC frame RC frame                                 | 4< <i>VD</i> 0.1 0.6 0.4  | 4 <vd<<br>10<br/>/<br/>0.4<br/>0.2<br/>0.6</vd<<br>  | 15<br>/<br>0.1                   | 4<<br>VD<br>/<br>0.4                             | 4< <i>VD</i> <<br>10<br>/<br>0.1                                | 15<br>/<br>/       |  |  |  |
| (kPa)                           | Building structure  RC frame Non-RC frame RC frame Non-RC                          | 4< <i>VD</i> 0.1 0.6 0.4  | 4< <i>VD</i> <<br>10<br>/<br>0.4   | 15<br>/<br>0.1                   | 4<<br>VD<br>/<br>0.4                             | 4< <i>VD</i> <<br>10<br>/<br>0.1                                | 15<br>/<br>/       |  |  |  |
| (kPa)<br><30<br>30-70           | Building structure  RC frame Non-RC frame RC frame Non-RC frame                    | 4< VD 0.1 0.6 0.4 0.8   | 4 <vd<<br>10<br/>/<br/>0.4<br/>0.2<br/>0.6</vd<<br>  | 15<br>/<br>0.1                   | 4<<br>VD<br>/<br>0.4<br>0.2<br>0.6               | 4< <i>VD</i> <<br>10<br>/<br>0.1                                | 15 /               |  |  |  |
| (kPa)<br><30<br>30-70           | RC frame Non-RC frame RC frame Non-RC frame RC frame Non-RC frame RC frame         | 4<<br>VD<br>0.1<br>0.6<br>0.4<br>0.8<br>0.5   | 4 <vd<<br>10<br/>/<br/>0.4<br/>0.2<br/>0.6</vd<<br>  | 15<br>/<br>0.1<br>/<br>0.3       | 4<<br>VD<br>/<br>0.4<br>0.2<br>0.6               | 4 <vd<<br>10<br/>/<br/>0.1<br/>/<br/>0.3</vd<<br>               | 15 /               |  |  |  |
| (kPa)<br><30<br>30-70           | Building<br>structure  RC frame Non-RC frame RC frame Non-RC frame RC frame Non-RC | 4<<br>VD<br>0.1<br>0.6<br>0.4<br>0.8<br>0.5   | 4 <vd<<br>10<br/>/<br/>0.4<br/>0.2<br/>0.6</vd<<br>  | 15<br>/<br>0.1<br>/<br>0.3       | 4<<br>VD<br>/<br>0.4<br>0.2<br>0.6               | 4 <vd<<br>10<br/>/<br/>0.1<br/>/<br/>0.3</vd<<br>               | 15 /               |  |  |  |
| (kPa)<br><30<br>30-70<br>70-100 | RC frame Non-RC frame Non-RC frame RC frame RC frame RC frame RC frame RC frame    | 4<<br>VD<br>0.1<br>0.6<br>0.4<br>0.8<br>0.5<br>0.9  | 4 <vd<<br>10<br/>/<br/>0.4<br/>0.2<br/>0.6<br/>0.3<br/>0.7</vd<<br>  | /<br>0.1<br>/<br>0.3<br>/<br>0.4 | 4<<br>VD<br>/<br>0.4<br>0.2<br>0.6<br>0.3<br>0.7 | 4 <vd<<br>10<br/>/<br/>0.1<br/>/<br/>0.3<br/>/<br/>0.4</vd<<br> | 15<br>/<br>/       |  |  |  |

### 3.4 Prediction model development and assessment

The debris flows in Table 6 (see Appendix. A) were divided into a training set and a validation set, and the training set is used to train the prediction model. The validation results are plotted in Fig. 7. Additionally, the performance of the developed model is assessed using the three indexes (Eqs. (14)-(16)). As indicated in Fig. 7, the prediction results show minor errors to the actual values, and the MAPE, RMSE and MAE values are 9.70%, 3.98 kPa and 2.74 kPa, respectively. RMSE value can reflect the extreme errors, and the calculated RMSE value can indicate that there are no extreme values observed in





the prediction results. Additionally, MAPE reflects the error percentage between the measured and predicted values, and the model is more reliable if the MAPE is closer to 0. Therefore, it can be concluded that this model performed well in predicting the volume of a future debris-flow event.

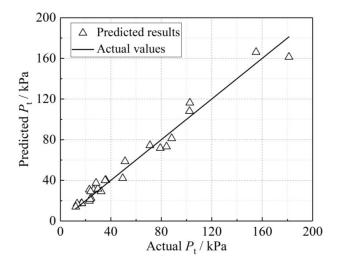


Figure 7. Plotted results of the prediction results

### 4 Case study

#### 4.1 Geological setting

We selected the Longxihe Basin (Fig. 8) in Dujiangyan, Sichuan Province, to conduct a case study (see Fig. 1 about the geographic location of this area), which is 15 km away from the epicenter of the 2008 Wenchuan earthquake. There are three faults crossing this area, namely the Southern Branch of the Yingxiu-Beichuan Fault, the Northern Branch of the Yingxiu-Beichuan Fault, and the Feilaifeng Structure. These faults and structures cause the incised valleys and uplifting of the land surface, resulting in large areas of exposed rocks. Additionally, this study area belongs to the subtropical monsoon climate, with annual precipitation reaching 1134.8 mm. Over 80 % of the annual rainfall occurs from May to September. Consequently, the abundant rainfall and complex geological structure give birth to frequent debris flows. It was reported that 13 debris-flow events occurred in this basin on 12<sup>th</sup> May, 24<sup>th</sup> June, 25<sup>th</sup> September 2008, and 17<sup>th</sup> July 2009. In particular, 45 debris-flow events were recorded on 13<sup>th</sup>





August 2010 due to a high-intense rainfall event, causing severe damage to 233 buildings and resulting in the entire economic loss of  $7.2\times10^7$  €. There are one town and two villages distributed in this basin. The impacts of the Wenchuan earthquake still pose threats to the local people since a time period of at least 20 years is required if the occurrence frequency of debris flows before the earthquake is expected (Yu et al., 2014).

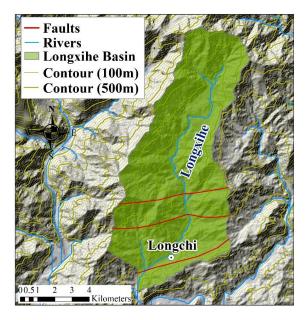


Figure 8. The Longxihe Basin, Dujiangyan, China

#### 4.2 Estimation of economic loss of buildings

#### 4.2.1 Determination of physical vulnerability

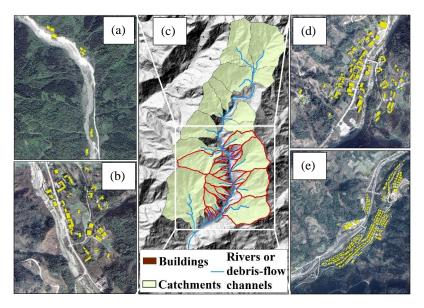
To estimate the potential physical damage of the buildings in the Longxihe Basin, the developed prediction model was applied to predict the potential impact pressure to buildings. As illustrated in Fig. 9(c), the debris-flow catchments within this basin were generated since we mainly focus on the regions with the distribution of buildings and estimate the possible economic loss of the buildings when debris flows occur. Therefore, we extracted a total of 386 buildings in three regions based on the satellite images (Fig. 9(a), Fig. 9(b), Fig. 9(d), and Fig. 9(e)). After that, we selected the catchments that are the





- nearest to the buildings to conduct analysis (see highlighted catchments with red lines in Fig. 9(c)). The
- 329 input information of these catchments for impact pressure prediction and the predicted results are all

## listed in Table 3.



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Figure 9. The debris-flow catchments and residential buildings in this area

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Table 3. Prediction results using developed prediction model

|     | 2                 |                 |     |        |               |
|-----|-------------------|-----------------|-----|--------|---------------|
| No. | $A / \text{km}^2$ | $L/\mathrm{km}$ | R/m | J      | Predicted     |
|     |                   |                 |     |        | $P_{t}$ (kPa) |
| 1   | 0.4226            | 0.70            | 116 | 0.3024 | 22.0          |
| 2   | 0.8849            | 1.00            | 123 | 0.3503 | 26.7          |
| 3   | 0.1447            | 0.25            | 113 | 0.4055 | 18.0          |
| 4   | 2.9068            | 0.91            | 145 | 0.1668 | 22.1          |
| 5   | 0.3637            | 0.58            | 125 | 0.2998 | 19.2          |
| 6   | 0.9317            | 0.88            | 130 | 0.2551 | 20.9          |
| 7   | 4.1780            | 1.84            | 141 | 0.0751 | 16.0          |
| 8   | 0.1632            | 0.61            | 117 | 0.3419 | 19.3          |
| 9   | 0.0932            | 0.69            | 112 | 0.3622 | 17.3          |
| 10  | 0.1087            | 0.69            | 112 | 0.3542 | 17.5          |
| 11  | 0.2355            | 0.73            | 159 | 0.6828 | 16.5          |
| 12  | 1.3027            | 1.46            | 145 | 0.3944 | 25.2          |
| 13  | 2.8095            | 1.30            | 158 | 0.2466 | 26.5          |
| 14  | 0.3802            | 0.89            | 129 | 0.4299 | 19.2          |
| 15  | 0.2177            | 0.70            | 136 | 0.5690 | 15.8          |
| 16  | 0.1529            | 0.84            | 162 | 0.6821 | 14.4          |
| 17  | 3.5789            | 2.23            | 153 | 0.3047 | 33.6          |
| 18  | 0.3179            | 0.69            | 127 | 0.5400 | 17.4          |
| 19  | 0.1970            | 0.74            | 96  | 0.4056 | 15.0          |
| 20  | 0.2201            | 0.90            | 110 | 0.4599 | 13.0          |
|     |                   |                 |     |        |               |





In addition to the predicted impact pressures to the buildings by the potential debris flows, the horizontal and vertical distances between each building and the nearest debris-flow channel were measured using GIS. As a result, the physical vulnerabilities of the buildings in Longxihe Basin can be decided based on the proposed physical vulnerability matrix, and the results are shown in Figs. 10(a)-(d).

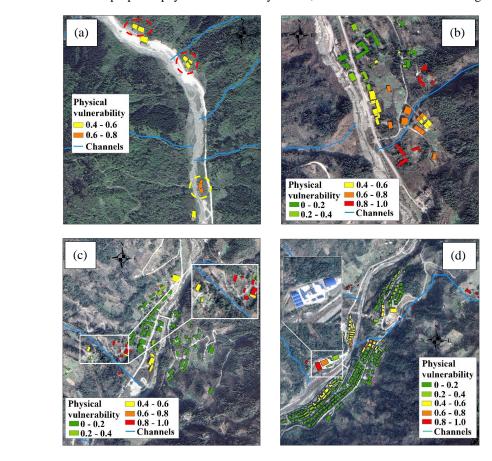


Figure 10. Physical vulnerabilities of the buildings in the Longchi Basin

Table 4. Statistical results of the buildings with different physical vulnerabilities

|            | 0 - 0.2 | 0.2 - 0.4 | 0.4 - 0.6 | 0.6 - 0.8 | 0.8 - 1.0 |
|------------|---------|-----------|-----------|-----------|-----------|
| Number     | 237     | 52        | 45        | 18        | 34        |
| Percentage | 61.4%   | 13.5%     | 11.6%     | 4.7%      | 8.8%      |

The statistical results in Table 4 illustrate that most buildings nearly suffer no damage when a debris-flow event occurs. This is because these buildings are RC-frame structures, which allow them to stay undamaged or only suffer slight damage even though they are close to the debris-flow channels.

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However, non-RC frame buildings may always suffer severe damage during a debris-flow event if their locations are near the channels. As indicated in Figs. 10(a)-(d), the buildings with high and very-high physical vulnerabilities are mainly brick and light steel structures. The difference in resistance ability allows a greater possibility for RC-frame buildings to keep structures undamaged during the same debris-flow event when compared to a non-RC building, which is consistent with the field investigation results in Fig. 3(b). Moreover, a non-RC frame building can also avoid damage even though it is close to the debris-flow channel. This is because a higher vertical distance to the debris-flow channel can allow this non-RC building to suffer no damage or light damage. Therefore, a comprehensive analysis by considering the structure type, spatial distances to debris-flow channel, and potential impact pressure is significant to establish a reliable physical vulnerability matrix to benefit the determination of the potential damage degree of buildings. In order to validate the efficiency and accuracy of our method in estimating the physical damages of buildings, the damaged buildings caused by debris flows on 13th August 2010 are employed here to assess the reliability of this method. As depicted in Fig. 11(a), the RC-frame buildings suffer a moderate damage (see red dashed circles in Fig. 10(a)) because there are no obvious damages of external or internal walls observed during the field investigations based on the HAZUS building classification scheme (Rojahn, 1988). However, the debris-flow event caused an extensive damage (see yellow dashed circles in Fig. 10(a)) to the brick structures due to the partly destroyed external or internal walls. As a result, evacuation of people is necessary and reconstruction is required. Overall, our proposed method can provide a reliable evaluation of physical vulnerability of buildings caused by a debris-flow event

and therefore benefit their estimation of economic loss.





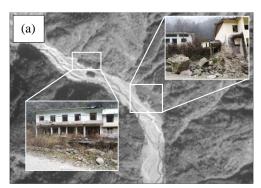




Figure 11. The damaged buildings caused by the debris-flow events.

## 4.2.2 Economic loss

Based on the estimated physical damage, we can further provide a reliable estimation of the economic loss. Six categories of buildings were identified in this study area based on the field investigations. They are residential buildings, factory buildings, office buildings, and livestock houses.

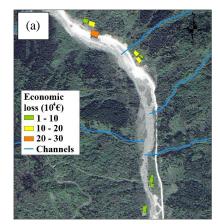
Table 5. Unit price (P) of a building in this area

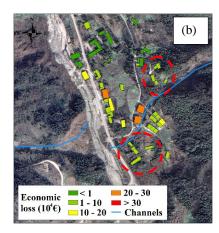
| Element   | Categories                                | Unit price   | Value based on                      |
|-----------|---|--------------|-------------------------------------|
|           | Residential buildings (RC-frame)          | 1050.44 €/m² | Average market price                |
|           | Residential buildings (Brick structure)   | 158.38 €/m²  | Construction cost                   |
| Duildings | Business buildings (RC-frame)             | 1371.47 €/m² | Average market price                |
| Buildings | Office buildings (RC-frame)               | 1050.44 €/m² | \                                   |
|           | Factory buildings (Light steel structure) | 237.57 €/m²  | Construction cost                   |
|           | Livestock houses (Brick structure)        | 7.92 €/m²    | Restoration and reconstruction cost |

The economic value of a residential building in this area is based on the market price, which is provided by the Housing and Urban-rural Construction Agency. As for the unit price of a business building, we refer to the price ratio of a residential building and a business building in the city center of Dujiangyan. The unit price of a business building is normally 30% higher than a residential building. An office building belongs to the national assets, which cannot be rented or sold. However, possible damage still cannot be avoided if a debris-flow event occurs, which therefore requires restoration or











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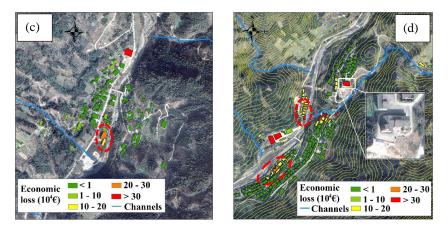


Figure 12. Economic loss of the residential buildings in the Longxihe Basin

The distribution characteristics of economic loss are different from physical vulnerability. For example, Figs. 10(a) illustrates that the buildings are more likely to suffer severe damage if they are close to the debris-flow channel, especially the non-RC frame structures. However, these non-RC frame buildings require lower reconstruction or restoration costs when compared to the RC-frame buildings (see Fig. 12(a)). In this case, the economic loss is low since it relies on the multiplication of physical vulnerability and economic value of a building (see red dashes in Fig. 12(b)). As indicated in Fig. 12(d), the factory buildings (see Fig. 10(d) and Fig. 12(d)) may suffer an economic loss of  $3.2 \times 10^5 \, \odot$ . As for the reason why a low unit price of a factory building (see Table 5) results in a high economic loss may be due to the large area of this factory building. Therefore, the site selection of a factory building is significant. Although the location of the factory buildings in mountainous areas can avoid noise pollution in urban development and decrease construction costs, the possible economic loss caused by natural hazards cannot be neglected. Additionally, the residential building should not be built on the outlet of the debris-flow catchment directly opposite (see red dash circles in Fig. 12(d)), especially when the foundation of the residential buildings is only slightly higher than the riverway (see yellow contours in Fig. 12(d)). For example, the highest economic loss is found in a residential building (see the image in Fig. 12(d)), reaching  $5.1 \times 10^5 \in$ . Therefore, at least a 4 m of residential building (RC frame) foundation





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is essential if the buildings are close to the debris-flow channel based on Table 2. Overall, the analysis of economic loss for buildings in mountainous areas can provide decision-makers with guidance about urban planning.

#### 5. Discussion

The proposed integrated method has been applied for the determination of the damage degree for buildings in the Longxihe Basin, Sichuan, China. The involvement of debris-flow intensities, building attributes, and spatial position between the buildings and debris-flow channel can help to suggest a more reasonable damage degree value caused by debris flows. Specifically, the debris-flow intensity is expressed in impact pressure here, which can indicate the possible consequence of a building if the flowing materials strike the building directly. However, an overestimation of the damage degree may be caused since the spatial positions between the building and debris-flow channel is not a one-dimensional problem. In general, the elevation of a building is greater than that of the debris-flow channel in the horizontal direction. This is because the long-term water flow and historical debris flows move the soils and rocks, causing erosion of the channel bottom and therefore decreasing its elevation. As a result, the elevation difference between the buildings and the debris-flow channel could cause a loss of impact pressure. Therefore, simply utilizing impact pressure is not enough to reflect the actual damage to a building. In contrast, the introduction of HD and VD is an effective supplement to improve the estimation of physical damage that the buildings may suffer. Furthermore, the damage degree may vary when subjected to different building structures. In this case, two major types of buildings are considered in this study to distinguish the impact resistance capacities of different building types. Overall, this developed matrix comprehensively describes the factors impacting the damage degree of buildings caused by debris flows.

By utilizing the proposed matrix, we can estimate the damage degree of a building. However, the

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possible damage in future scenarios is still unclear due to the change in debris-flow magnitude. Therefore, an ensemble machine learning (ML) model is used to predict the volume of a future debrisflow event so that the debris-flow intensities can be calculated based on the empirical relationships. This ML method can effectively avoid over-fitting when training prediction models due to the existence of a regular term. Most importantly, the strong ability in establishing a reliable relationship between a group of independent variables and a dependent variable enables a wider application of ML methods when compared to empirical and regression methods. Therefore, a precise prediction can be expected based on the established relationship using the ML method to indicate the potential damage to buildings caused by a future debris-flow event. However, we are also aware that the current sample size may not support a robustness performance in estimating impact pressure to buildings. For broader applications, continuous input of debris-flow data globally is essential, which may beyond the scope of this study. However, further improvement can also be achieved if the floors of buildings are considered when developing the physical vulnerability matrix. This is because the degree of loss presents a negative correlation with the number of floors (Fuchs et al., 2019). Nevertheless, the limitation cannot alter the fact that our work can benefit the subdivision of buildings in different vulnerability levels and provide suggestions about the site selection of future residential areas.

#### 6. Conclusion

In this paper, an integrated method for vulnerability assessment of buildings caused by future debris flows was proposed. This method includes a physical matrix and a machine learning model, in which this matrix was developed by considering the debris-flow process, building structure, and spatial positions between the buildings and debris-flow channels. To be more specific, the debris-flow process is represented by impact pressure ( $P_1$ ), which can be estimated based on the debris-flow volume through field investigations. As for the definition of spatial positions, HD and VD are used to describe the





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position relation between the buildings and the debris-flow channel. By combining the three parameters, the actual impact pressure on the buildings can be decided. However, the damage degree may vary for different building structures. Therefore, the building structure is further considered to provide a precise estimation of the buildings, including the RC frame and non-RC frame (brick structure, light steel structure, and masonry structure). Therefore, a total of six types of buildings are included in this study. They are residential buildings (RC frame and brick structure), business buildings (RC frame), office buildings (RC frame), factory buildings (light steel structure), and livestock houses (brick structure). At the same time, an ML model (XGBoost) was developed to predict the impact pressure to buildings caused by future debris flows. On the basis of the proposed physical vulnerability matrix and machine learning model, we selected the Longxihe Basin, Sichuan, China, to conduct a case study. The results show that the non-RC buildings may be more likely to suffer severe damage if they are close to the debris-flow channels. The buildings with high and very-high physical vulnerabilities are mainly brick and light steel structures. Consequently, the factory buildings occupy the highest economic loss, reaching 2.41×10<sup>5</sup> € due to their large area. In addition, the buildings may suffer severe economic loss if they are located the directly opposite of the outlet of the debris-flow catchment. Overall, our studies can achieve a reliable assessment of the physical damage and corresponding economic loss of buildings and therefore provide suggestions and scientific support for the future construction planning of buildings.

# CRediT authorship contribution statement

- Chenchen Qiu: Methodology, Software, Data curation, Writing Original draft preparation. Xueyu
- 474 Geng: Conceptualization, Visualization, Validation, Supervision, Writing Review & Editing.

### 475 **Declaration of competing interest**

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

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- 484 References
- 485 Alene, G.H., Irshad, S., Moraru, A., Depina, I., Bruland, O., Perkis, A. and Thakur, V., 2024. Virtual
- 486 reality visualization of geophysical flows: A framework. Environmental Modelling & Software, 177,
- 487 p.106063.
- 488 Bründl, M., Romang, H.E., Bischof, N., Rheinberger, C.M., 2009. The risk concept and its application in
- 489 natural hazard risk management in Switzerland. Nat. Hazards Earth Syst. Sci. 9, 801–813.
- 490 Chen, T., Guestrin, C., 2016. XGBoost: A scalable tree boosting system. Proc. ACM SIGKDD Int. Conf.
- 491 Knowl. Discov. Data Min. 13-17-Augu, 785–794. https://doi.org/10.1145/2939672.2939785
- 492 Creutin, J.D., Muste, M., Bradley, A.A., Kim, S.C., Kruger, A., 2003. River gauging using PIV
- 493 techniques: A proof of concept experiment on the Iowa River. J. Hydrol. 277, 182–194.
- 494 https://doi.org/10.1016/S0022-1694(03)00081-7
- Cui, P., Xiang, L.Z., Zou, Q., 2013. Risk assessment of highways affected by debris flows in Wenchuan
- 496 earthquake area. J. Mt. Sci. 10, 173–189. https://doi.org/10.1007/s11629-013-2575-y
- 497 Dai, F.C., Lee, C.F., Ngai, Y.Y., 2002. Landslide risk assessment and management: An overview. Eng.
- 498 Geol. 64, 65–87. https://doi.org/10.1016/S0013-7952(01)00093-X
- 499 Fell, R., 1994. Landslide risk assessment and acceptable risk. Can. Geotech. J. 31, 261–272.
- 500 Fuchs, S., 2009. Susceptibility versus resilience to mountain hazards in Austria-paradigms of
- vulnerability revisited. Nat. Hazards Earth Syst. Sci. 9, 337–352.
- 502 Fuchs, S., Keiler, M., Ortlepp, R., Schinke, R., Papathoma-Köhle, M., 2019. Recent advances in
- 503 vulnerability assessment for the built environment exposed to torrential hazards: Challenges and the
- 504 way forward. J. Hydrol. 575, 587–595.





Guzzetti, F., Stark, C.P., Salvati, P., 2005. Evaluation of flood and landslide risk to the population of 505 506 Italy. Environ. Manage. 36, 15-36. Huang, J., Hales, T.C., Huang, R., Ju, N., Li, Q., Huang, Y., 2020. A hybrid machine-learning model to 507 Geomorphology 508 estimate potential debris-flow volumes. 367, 107333. https://doi.org/10.1016/j.geomorph.2020.107333 509 510 Immerzeel, W.W., Lutz, A.F., Andrade, M., Bahl, A., Biemans, H., Bolch, T., Hyde, S., Brumby, S., Davies, B.J., Elmore, A.C. and Emmer, A., 2020. Importance and vulnerability of the world's water 511 towers. Nature, 577(7790), pp.364-369. 512 Jakob, M., Stein, D., Ulmi, M., 2012. Vulnerability of buildings to debris flow impact. Nat. Hazards 60, 513 241–261. https://doi.org/10.1007/s11069-011-0007-2 514 515 Jiang, H., Zou, Q., Zhu, Y., Li, Y., Zhou, B., Zhou, W., Yao, S., Dai, X., Yao, H. and Chen, S., 2024. Deep Learning Prediction of Rainfall-driven Debris Flows Considering the Similar Critical 516 517 Thresholds within Comparable Background Conditions. Environmental Modelling & Software, 518 p.106130. 519 Kang, H. sub, Kim, Y. tae, 2016. The physical vulnerability of different types of building structure to debris flow events. Nat. Hazards 80, 1475–1493. https://doi.org/10.1007/s11069-015-2032-z 520 521 Khosravi, K., Khozani, Z.S., Mao, L., 2021. A comparison between advanced hybrid machine learning algorithms and empirical equations applied to abutment scour depth prediction. J. Hydrol. 596, 522 523 126100. https://doi.org/10.1016/j.jhydrol.2021.126100 Koch, T., 1998. Testing various constitutive equations for debris flow modelling. IAHS-AISH Publ. 524 249-257. 525 Liu, S., Wei, L., Hu, K., 2020. Topographical and geological variation of effective rainfall for debris-526 flow occurrence from a large-scale perspective. Geomorphology 358, 107134. 527





Marcato, G., Bossi, G., Rivelli, F., Borgatti, L., 2012. Debris flood hazard documentation and mitigation 528 529 on the Tilcara alluvial fan (Quebrada de Humahuaca, Jujuy province, North-West Argentina). Nat. 530 Hazards Earth Syst. Sci. 12, 1873–1882. https://doi.org/10.5194/nhess-12-1873-2012 531 Miao, C., Liu, X., 2020. Characterization of acceptable risk for debris flows in China: Comparison in 532 debris-flow prone areas and nonprone areas. Int. J. disaster risk Reduct. 42, 101405. 533 Papathoma-Köhle, M., Gems, B., Sturm, M., Fuchs, S., 2017. Matrices, curves and indicators: A review of approaches to assess physical vulnerability to debris flows. Earth-Science Rev. 171, 272-288. 534 Paudel, B., Fall, M., Daneshfar, B., 2021. Gis-Based Assessment of Debris Flow Runout in Kulekhani 535 Watershed, Nepal. Geotech. Geol. Eng. 39, 2755–2775. https://doi.org/10.1007/s10706-020-01655-536 1 537 Qiu, C., Su, L., Zou, Q., Geng, X., 2022. A hybrid machine-learning model to map glacier-related debris 538 flow susceptibility along Gyirong Zangbo watershed under the changing climate. Sci. Total Environ. 539 540 818, 151752. https://doi.org/10.1016/j.scitotenv.2021.151752 541 Quan Luna, B., Blahut, J., Van Westen, C.J., Sterlacchini, S., Van Asch, T.W.J., Akbas, S.O., 2011. The 542 application of numerical debris flow modelling for the generation of physical vulnerability curves. Nat. Hazards Earth Syst. Sci. 11, 2047–2060. https://doi.org/10.5194/nhess-11-2047-2011 543 544 Rickenmann, D., 1999. Empirical relationships for debris flows. Nat. Hazards 19, 47-77. https://doi.org/10.1023/A:1008064220727 545 Rojahn, C., 1988. Rapid visual screening of buildings for potential seismic hazards: A handbook. 546 Federal Emergency Management Agency. 547 Ruggieri, S., Calò, M., Cardellicchio, A., Uva, G., 2022. Analytical-mechanical based framework for 548 549 seismic overall fragility analysis of existing RC buildings in town compartments. Bull. Earthq. Eng. 20, 8179-8216. 550





573

Ruggieri, S., Liguori, F.S., Leggieri, V., Bilotta, A., Madeo, A., Casolo, S., Uva, G., 2023. An 551 552 archetype-based automated procedure to derive global-local seismic fragility of masonry building 553 aggregates: META-FORMA-XL. Int. J. disaster risk Reduct. 95, 103903. Santi, P.M., Hewitt, K., VanDine, D.F., Barillas Cruz, E., 2011. Debris-flow impact, vulnerability, and 554 555 response. Nat. hazards 56, 371-402. 556 Sridharan, A., Gutjahr, G. and Gopalan, S., 2024. Markov-Switching Spatio-Temporal Generalized Additive Model for Landslide Susceptibility. Environmental Modelling & Software, 173, p.105892. 557 Sturm, M., Gems, B., Keller, F., Mazzorana, B., Fuchs, S., Papathoma-Köhle, M., Aufleger, M., 2018. 558 Understanding impact dynamics on buildings caused by fluviatile sediment transport. 559 Geomorphology 321, 45–59. https://doi.org/10.1016/j.geomorph.2018.08.016 560 Wei, L., Hu, K., Hu, X., 2018. Rainfall occurrence and its relation to flood damage in China from 2000 561 to 2015. J. Mt. Sci. 15, 2492-2504. 562 563 Wei, L., Hu, K., Liu, J., 2022. Automatic identification of buildings vulnerable to debris flows in 564 Sichuan Province, China, by GIS analysis and Deep Encoding Network methods. J. Flood Risk Manag. 15, 1–18. https://doi.org/10.1111/jfr3.12830 565 Xie, H., Liu, W., Zhao, J., Hu, K., 2013. Characteristics of Tangjiagou debris flow in Shimian of 566 567 Sichuan in July 14, 2012. J. Earth Sci. Environ. 35, 90–97. Yu, B., Wu, Y., Chu, S., 2014. Preliminary study of the effect of earthquakes on the rainfall threshold of 568 debris flows. Eng. Geol. 182, 130–135. https://doi.org/10.1016/j.enggeo.2014.04.007 569 Zanchetta, G., Sulpizio, R., Pareschi, M.T., Leoni, F.M., Santacroce, R., 2004. Characteristics of May 5-570 6, 1998 volcaniclastic debris flows in the Sarno area (Campania, southern Italy): relationships to 571

Zhang, S., Zhang, L., Li, X., Xu, Q., 2018. Physical vulnerability models for assessing building damage

structural damage and hazard zonation. J. Volcanol. Geotherm. Res. 133, 377–393.

https://doi.org/10.5194/nhess-2024-156 Preprint. Discussion started: 4 September 2024 © Author(s) 2024. CC BY 4.0 License.





574 by debris flows. Eng. Geol. 247, 145–158.

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## 576 **Appendix**. A

### 1. Mechanism of XGBoost

The mechanism of XGBoost is to constantly develop a new decision tree which acts as a weak learner and fits the residual error of the last prediction. After the training of a total of k trees, the final prediction result is the sum of the score of each leaf node in each developed tree. The target function of regression in XGBoost is:

$$L(\phi) = \sum_{i=1}^{m} l(y_i, y_i) + \sum_{k=1}^{t} \Omega(f_k)$$
(10)

where  $\sum_{i=1}^{m} l(y_i, y_i)$  represents the loss function, and  $\sum_{k=1}^{t} \Omega(f_k)$  is the regularisation term.  $y_i$  and  $y_i$ 

are prediction value and true value, respectively. m is the number of samples.  $f_k$  is the  $k_{th}$  tree model. As mentioned above, the newly generated tree needs to fit the residual error of the last prediction, and therefore the prediction result can be presented as:

587 
$$y_i^t = y_i^{(t-1)} + f_t(x_i)$$
 (11)

Substitute the Eq. (12) into Eq. (11) to rewrite the objective function as:

589 
$$L(\phi) = \sum_{i=1}^{m} l(y_i, y_i^{(t-1)} + f_t(x_i)) + \sum_{k=1}^{t} \Omega(f_k)$$
 (12)

Furthermore, Taylor's second order expansion is introduced to find  $f_k$  to minimize the objective

591 function:

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592 
$$L(\phi) = \sum_{i=1}^{m} \left[ l(y_i, y_i^{(t-1)}) + g_i f_i(x_i) + \frac{1}{2} h_i f_i^2(x_i) \right] + \sum_{k=1}^{t} \Omega(f_k) + constant$$
 (13)

where  $g_i$  is the first derivation, and the  $h_i$  represents the second derivation

### 594 2. Calculation results of impact pressure $P_t$

Table 6. Dataset II for developing the impact pressure prediction model

| No. | A        | L    | R   | J      | $P_t$ | No. | A        | L    | R   | J      | $P_t$ |
|-----|----------|------|-----|--------|-------|-----|----------|------|-----|--------|-------|
|     | $(km^2)$ | (km) | (m) |        | (kPa) |     | $(km^2)$ | (km) | (m) |        | (kPa) |
| 1   | 8.55     | 3.13 | 269 | 0.1051 | 40.9  | 42  | 0.05     | 0.18 | 85  | 0.1908 | 10.3  |
| 2   | 4.68     | 1.41 | 126 | 0.2162 | 47.4  | 43  | 0.06     | 0.23 | 81  | 0.3038 | 14.2  |





| 3  | 12.88 | 4.16 | 269 | 0.1246 | 56.0  | 44 | 0.33  | 0.50 | 162 | 0.2792 | 18.4  |
|----|-------|------|-----|--------|-------|----|-------|------|-----|--------|-------|
| 4  | 0.29  | 0.50 | 95  | 0.1638 | 13.2  | 45 | 0.05  | 0.20 | 107 | 0.2661 | 12.2  |
| 5  | 0.29  | 0.29 | 200 | 0.4122 | 23.0  | 46 | 1.37  | 1.11 | 160 | 0.1763 | 34.1  |
| 6  | 5.73  | 0.71 | 260 | 0.1175 | 49.4  | 47 | 4.83  | 1.96 | 277 | 0.2071 | 35.5  |
| 7  | 0.56  | 0.62 | 195 | 0.2475 | 29.3  | 48 | 1.33  | 0.50 | 258 | 0.5117 | 35.7  |
| 8  | 2.15  | 0.73 | 250 | 0.2736 | 24.2  | 49 | 0.17  | 0.62 | 231 | 0.4727 | 21.0  |
| 9  | 0.32  | 0.46 | 276 | 0.5452 | 23.0  | 50 | 12.47 | 3.61 | 366 | 0.1853 | 67.9  |
| 10 | 1.67  | 0.95 | 161 | 0.3699 | 32.3  | 51 | 0.46  | 0.88 | 189 | 0.3819 | 26.4  |
| 11 | 11.21 | 1.93 | 360 | 0.1512 | 34.1  | 52 | 1.63  | 1.98 | 148 | 0.3115 | 28.9  |
| 12 | 2.85  | 1.57 | 232 | 0.2568 | 28.3  | 53 | 1.34  | 1.00 | 158 | 0.1727 | 18.7  |
| 13 | 2.29  | 1.84 | 189 | 0.3581 | 46.6  | 54 | 0.24  | 0.43 | 151 | 0.2867 | 16.6  |
| 14 | 0.08  | 0.42 | 240 | 0.3561 | 16.6  | 55 | 0.39  | 0.75 | 120 | 0.1745 | 15.6  |
| 15 | 0.18  | 0.48 | 366 | 0.6976 | 13.0  | 56 | 0.02  | 0.1  | 132 | 0.5295 | 18.0  |
| 16 | 0.53  | 0.81 | 170 | 0.2943 | 22.5  | 57 | 2.56  | 1.23 | 127 | 0.0998 | 16.7  |
| 17 | 0.71  | 1.74 | 151 | 0.6494 | 166.9 | 58 | 1.62  | 0.71 | 229 | 0.1673 | 19.7  |
| 18 | 0.49  | 1.64 | 162 | 0.6494 | 181.2 | 59 | 0.49  | 1.41 | 182 | 0.3000 | 24.0  |
| 19 | 0.60  | 1.52 | 155 | 0.6469 | 155.1 | 60 | 0.21  | 0.66 | 215 | 0.5384 | 40.6  |
| 20 | 0.36  | 1.15 | 261 | 0.8214 | 127.6 | 61 | 0.29  | 1.31 | 133 | 0.5184 | 64.1  |
| 21 | 2.73  | 2.57 | 190 | 0.6771 | 88.2  | 62 | 0.85  | 1.75 | 163 | 0.4578 | 36.0  |
| 22 | 2.02  | 2.59 | 198 | 0.7028 | 94.9  | 63 | 1.71  | 2.06 | 145 | 0.3879 | 68.5  |
| 23 | 0.43  | 1.30 | 198 | 0.7729 | 94.7  | 64 | 1.27  | 2.16 | 183 | 0.3522 | 84.1  |
| 24 | 0.19  | 1.09 | 181 | 0.6873 | 79.2  | 65 | 0.89  | 2.07 | 127 | 0.3385 | 68.1  |
| 25 | 1.03  | 2.02 | 232 | 0.4369 | 51.2  | 66 | 0.49  | 1.20 | 168 | 0.5681 | 141.0 |
| 26 | 3.99  | 3.78 | 134 | 0.4061 | 36.8  | 67 | 0.75  | 1.58 | 327 | 0.5566 | 165.7 |
| 27 | 2.88  | 2.40 | 313 | 0.7107 | 66.5  | 68 | 0.37  | 0.52 | 199 | 0.3404 | 23.6  |
| 28 | 0.34  | 1.14 | 163 | 0.8571 | 102.6 | 69 | 0.77  | 0.76 | 115 | 0.1566 | 17.0  |
| 29 | 2.81  | 2.84 | 253 | 0.5250 | 80.8  | 70 | 0.31  | 0.87 | 178 | 0.1317 | 25.9  |
| 30 | 7.18  | 4.82 | 400 | 0.5139 | 102.4 | 71 | 0.36  | 0.35 | 261 | 0.4578 | 20.6  |
| 31 | 24.42 | 9.47 | 337 | 0.3153 | 20.2  | 72 | 2.62  | 1.39 | 321 | 0.3482 | 33.8  |
| 32 | 2.81  | 1.74 | 205 | 0.3191 | 31.8  | 73 | 0.84  | 1.39 | 199 | 0.4899 | 14.9  |
| 33 | 0.43  | 1.30 | 200 | 0.8012 | 47.5  | 74 | 2.72  | 2.56 | 528 | 0.1069 | 31.2  |
| 34 | 7.06  | 4.41 | 275 | 0.4473 | 84.1  | 75 | 5.85  | 0.86 | 365 | 0.2962 | 31.5  |
| 35 | 1.07  | 2.05 | 225 | 0.4431 | 71.0  | 76 | 2.61  | 1.28 | 388 | 0.5317 | 44.0  |
| 36 | 0.86  | 2.17 | 149 | 0.3979 | 70.6  | 77 | 5.45  | 2.82 | 261 | 0.5228 | 112.0 |
| 37 | 6.51  | 2.92 | 252 | 0.5029 | 110.7 | 78 | 3.51  | 0.99 | 227 | 0.3839 | 38.2  |
| 38 | 0.42  | 1.64 | 151 | 0.4813 | 149.0 | 79 | 7.09  | 2.29 | 293 | 0.1962 | 52.6  |
| 39 | 0.51  | 1.43 | 153 | 0.4899 | 153.1 | 80 | 0.02  | 0.21 | 110 | 0.4390 | 17.8  |
| 40 | 0.20  | 0.76 | 130 | 0.5520 | 51.6  | 81 | 2.06  | 1.92 | 160 | 0.3211 | 29.7  |
| 41 | 0.34  | 1.25 | 130 | 0.4942 | 56.5  |    |       |      |     |        |       |
|    |       |      |     |        |       |    |       |      |     |        |       |