

1 We sincerely appreciate your thorough review and constructive comments on our manuscript. Your feedback  
2 has provided valuable guidance and will help improve the manuscript's structure and clarity. We have carefully  
3 considered each comment. Please find our detailed responses below, with author comments (AC) highlighted  
4 in blue.

5 **RC1's comments:**

6 This manuscript presents a case study for a modeling framework that combines numerical models and  
7 laboratory experiments to predict slope factor-of-safety values under pre- and post-fire conditions for a  
8 landscape in Victoria, Australia, burned by the 2016 Wye River-Jamieson Track wildfire. This location  
9 experienced shallow landslides in response to rainfall approximately 10 months after the fire was contained.  
10 The authors predict factors of safety using a hydrological model that simulates subsurface and overland flow  
11 given input rainfall, and they use controlled laboratory burn experiments on similar soils collected outside of  
12 the burn area to parameterize certain model values. The results of their modeling experiments demonstrate a  
13 widespread increase in slope instability as indicated by factors of safety less than 1 after the fire due to  
14 increased soil saturation and diminished soil cohesion that overlaps with the location of observed landslides.  
15 The focus of this study and the predictive nature of their methods are of scientific value and would be of  
16 interest to the community.

17 However, this reviewer has multiple concerns that need to be addressed before publication can be  
18 recommended. These include model selection in the pre-fire versus post-fire cases, choice of model parameter  
19 values, details of the controlled laboratory burn experiments, a need for expanded literature review, and  
20 insufficient consideration of uncertainty. Each of these topics is described in greater detail below. If these can  
21 be satisfactorily addressed, as well as the line-by-line comments at the end of this comment, then this reviewer  
22 could recommend publication to the editor.

23 1. Model selection: two distinct subcategories of the model are chosen for the pre-fire and post-fire cases,  
24 namely the "fine sand" model and the "medium-coarse sand" model. These selections are made on the  
25 basis of measured grain size distributions and hydraulic conductivity in the controlled burn laboratory  
26 experiment. However, it is unclear the impact that this choice alone has on the modeling results, as  
27 different equations are used to calculate the factor of safety in these models (i.e., equation 17 for the  
28 post-fire case, and equation 18 in the pre-fire case). The post-fire factor of safety equation depends on  
29 the depth to groundwater and the distance from the groundwater level to the slip surface. On the other  
30 hand, the pre-fire factor of safety equation depends on the depth of the wetting front from the surface.  
31 All else held equal, this reviewer wonders how this difference in physical process representation  
32 impacts the modeling results.

33 **AC:** Thank you for your valuable comments. We have provided the following responses to the issues  
34 you raised and will revise the manuscript accordingly.

In this study, the fine sand model was applied under pre-fire conditions, whereas the medium-coarse sand model was adopted post-fire. This selection was based on experimental results indicating that soil particle size increased and permeability improved after the fire, thereby shifting the dominant hydrological processes. Laboratory particle-size distribution tests indicated that the pre-fire soil was composed of fine sand, making it appropriate to apply the fine sand model for slope stability analysis (Ozaki et al., 2021; Wakai et al., 2019). This model focuses on the position and downward advancement of the wetting front and is therefore suitable for low-permeability materials. During the initial stage of rainfall, water is retained by capillary forces in the unsaturated zone, and the wetting front gradually progresses downward. The groundwater level responds slowly, and full saturation does not occur until the wetting front reaches the initial groundwater level. Consequently, the fine sand model employs **Eq. (1)** to calculate the factor of safety ( $F_s$ ), emphasizing the influence of wetting front depth. After the fire, the soil was exposed to high temperatures, and laboratory tests showed an increase in particle size and enhanced permeability. As a result, the soil was classified as coarse sand. In the medium-coarse sand model, under higher-permeability conditions, rainfall supply exceeds capillary retention. When under intense rainfall, water can penetrate the unsaturated zone in a short time, significantly raising the groundwater level (Nguyen et al., 2022). The slope response is therefore governed by groundwater-level rise, and the slip surface can rapidly reach saturation and failure conditions. Accordingly, **Eq. (2)** is used to characterize the reduction in shear strength induced by the rise in groundwater level. Both models are supported by published literature and documented parameter tests, and each has clearly defined applicable material domains (Nguyen et al., 2022; Ozaki et al., 2021; Wakai et al., 2019). Thus, they should not be interchanged.

$$F_s = \frac{\tau_f}{\tau} = \frac{c' + \gamma_{sat} \cdot H \cdot \cos^2 \theta \cdot \tan \varphi'}{\gamma_{sat} \cdot H \cdot \sin \theta \cdot \cos \theta}, \quad (1)$$

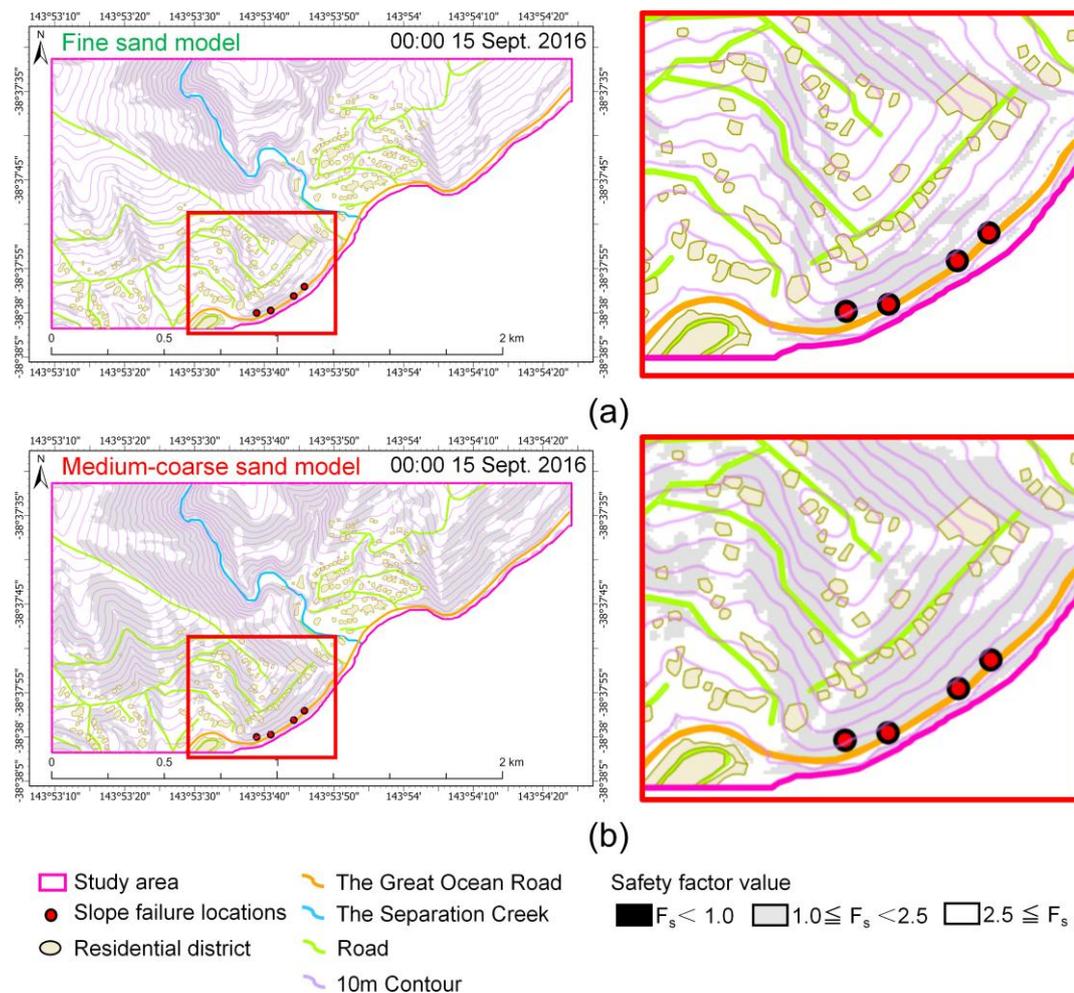
$$F_s = \frac{\tau_f}{\tau} = \frac{c' + [\gamma_t \cdot h_1 + (\gamma_{sat} - \gamma_w) h_2] \cos^2 \theta \cdot \tan \varphi'}{(\gamma_t \cdot h_1 + \gamma_{sat} \cdot h_2) \sin \theta \cdot \cos \theta}, \quad (2)$$

where  $\tau$  is the shear stress due to the sliding direction component of gravity of soil;  $\tau_f$  is the shear strength of soil, which is the maximum shear resistance;  $\gamma_t$  is the wet unit weight of soil;  $\gamma_{sat}$  is the saturated unit weight of soil;  $\gamma_w$  is the unit weight of water;  $h_1$  is the depth from the ground surface to the groundwater level;  $h_2$  is the depth from the groundwater level to the slip surface, which may correspond to the surface of the base layer;  $H$  is the depth from the ground surface to the wetting front;  $\theta$  is the slope inclination angle;  $c'$  is the cohesion of soil; and  $\varphi'$  is the angle of shear resistance of soil.

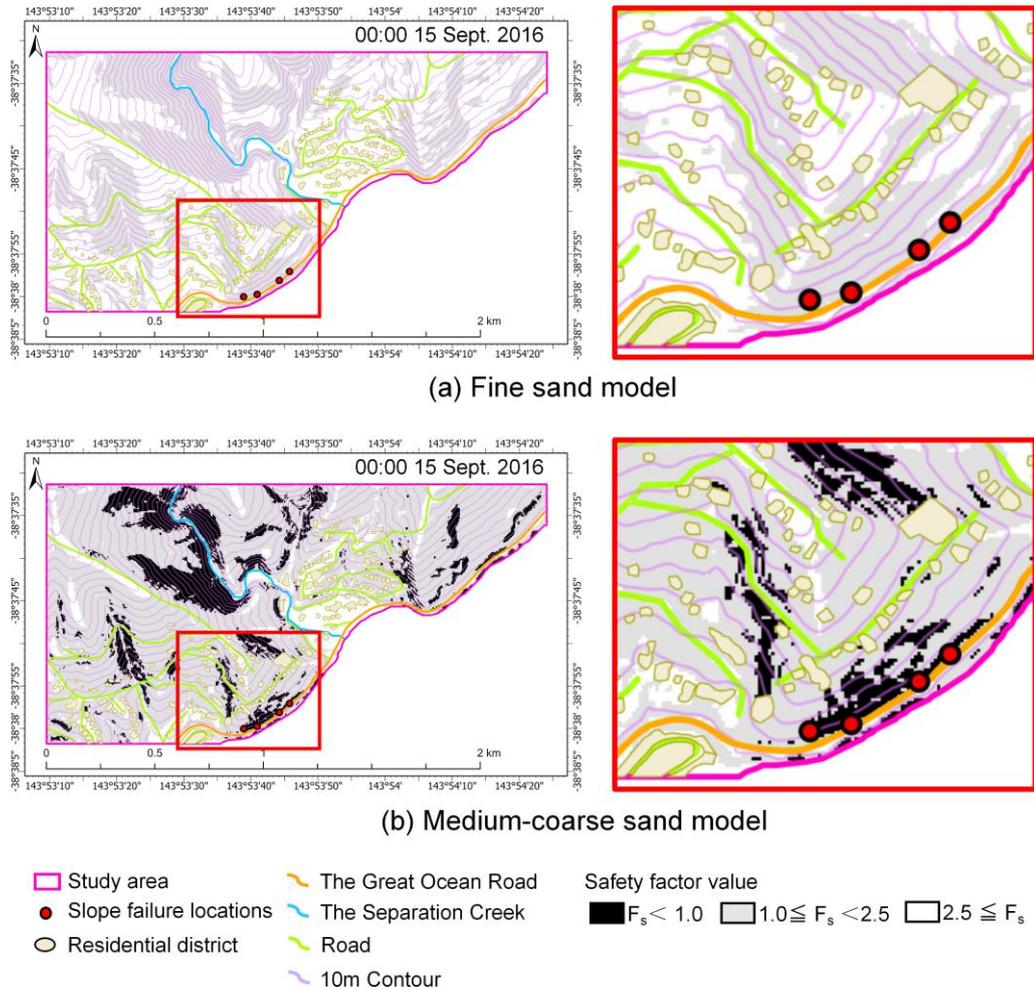
Following your suggestion, we conducted a sensitivity analysis: under the same rainfall and soil parameters, we switched only the  $F_s$  calculation formula (fine sand model versus medium-coarse sand model) to compare their effects on the  $F_s$  results. Under the pre-fire parameter conditions,  $F_s$  values across the study area were calculated using both the fine sand model (the primary model in this study) and the medium-coarse sand model (**Figure R1a** and **Figure R1b**). The results from both models show that  $F_s > 1$  across the entire area, with no unstable points. However, the absolute  $F_s$  values at the same locations differed significantly between the two models, with an average difference of about 20% and a maximum difference exceeding 50%. This indicates that even when the slope stability assessment results are consistent, the model structure has a substantial influence on the quantitative  $F_s$

74 values. Furthermore, as shown in **Figure R2**, the  $F_s$  results were calculated under post-fire parameter  
 75 conditions. **Figure R2a** and **Figure R2b** represent the simulation results from the fine sand model and  
 76 the medium-coarse sand model (the primary model in this study), respectively. It can be observed that  
 77 the  $F_s$  values obtained using the fine sand model are all greater than 1, indicating that the entire area  
 78 is classified as stable. However, when compared with the zones where landslides were observed, the  
 79 fine sand model clearly underestimates the actual landslide hazard. In contrast, the medium-coarse  
 80 sand model identifies multiple points with  $F_s < 1$ , which correspond well with the locations of the  
 81 observed landslides.

82 Based on the above results, under pre-fire parameter conditions, the material properties remain far  
 83 from the instability threshold. Therefore, all points were classified as stable ( $F_s > 1$ ), regardless of the  
 84 model structure chosen. However, after the fire, material strength decreased, and only the model that  
 85 correctly matched the underlying physical mechanisms (the medium-coarse sand model) can identify  
 86 the actual landslide locations, while the mismatched model (the fine sand model) underestimated the  
 87 hazard. Therefore, it is difficult to describe the soil changes examined in this study using a single  
 88 unified formula. In this study, we propose to apply separate models for pre-fire and post-fire conditions  
 89 precisely to ensure that the physical mechanisms are properly matched.



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 91 **Figure R1: Maps of safety factor under pre-fire conditions: (a) fine sand model and (b) medium-coarse sand model.**



(a) Fine sand model

(b) Medium-coarse sand model

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**Figure R2: Maps of safety factor under post-fire conditions: (a) fine sand model and (b) medium-coarse sand model.**

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- Choice of model parameters: a key choice is made by the authors in determining the depth of soil and the initial position of the groundwater table. Whereas the maximum soil depth is taken to be 2m in the pre-fire case, the maximum soil depth is 0.3m in the post-fire case. This is likely appropriate when considering the depth of mobilizable material as fire modifications to the near-surface soil decrease soil cohesion, as the authors describe on lines 354-357. However, in the hydrological modeling component of the study, the authors state on lines 363-364 that "...the initial groundwater level is assumed to be at the bottom of the soil depth." That is, the initial groundwater level is taken to be 2.0m below the surface in the pre-fire case and 0.3m below the surface in the post-fire case. Considering that the depth to groundwater and the distance between the groundwater level and the potential landslide slip surface are key inputs to the post-fire factor of safety equation, it is unclear what impact this difference in groundwater level assumption has on the study results.

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**AC:** Thank you for your valuable comments. Regarding the specification of soil thickness and initial groundwater level, and their impacts on the simulation results, our responses are as follows. The manuscript will be revised accordingly.

108 In this study, “soil thickness” is defined as the thickness of the mobilizable shallow soil layer that is  
 109 susceptible to failure under rainfall in shallow landslides. The spatial distribution of this parameter is  
 110 based on the inverse relationship between slope angle and soil thickness proposed by Saulnier et al.  
 111 (1997) (Eq. 3), which requires defining the minimum and maximum soil thickness values for the study  
 112 area. Under the pre-fire case, soil thickness represents the total soil depth from the surface to bedrock,  
 113 and the maximum thickness (2.0 m) is based on the soil depth map of Australia (Rossel et al., 2014).  
 114 Under the post-fire case, to more accurately reflect changes in slope physical mechanisms induced by  
 115 fire, the maximum thickness is not taken as the full depth to bedrock but is determined by two factors:  
 116 first, fire enhances shallow soil hydrophobicity, creating an impermeable layer above the bedrock;  
 117 second, root reinforcement is reduced after the fire. Some studies indicated that root concentrations of  
 118 Eucalyptus are particularly high in the surface 0.3 m of soil and decline sharply with depth (Baldwin  
 119 and Stewart, 1987; Grant et al., 2012). Thus, after the fire, reinforcement loss at this depth is most  
 120 significant, making it the most sensitive layer for potential slip surfaces. Therefore, the maximum  
 121 thickness is set to 0.3 m to reflect the mobilizable soil thickness under the combined influence of root  
 122 loss and soil hydrophobicity.

$$123 \quad y_i = y_{max} \left[ 1 - \frac{\tan(x_i) - \tan(x_{min})}{\tan(x_{max}) - \tan(x_{min})} (1 - \alpha) \right], \quad (3)$$

124 where  $y_i$  is the soil depth distribution;  $\alpha = y_{min}/y_{max}$ ;  $y_{min}$  and  $y_{max}$  are the minimum and  
 125 maximum values of effective soil depth, respectively;  $x_i$  is the slope angle at element  $i$ ; and  $x_{max}$   
 126 and  $x_{min}$  are the maximum and minimum values of slope angle, respectively.

127 Regarding the initial groundwater level, the source literature for the models used in this study (Nguyen  
 128 et al., 2022; Ozaki et al., 2021; Wakai et al., 2019) states that, in the absence of measured groundwater  
 129 data, the initial groundwater level is generally assumed to lie immediately above the impermeable  
 130 layer (i.e., at the base of the soil layer or the bedrock interface). This assumption facilitates parameter  
 131 consistency and model initialization, and has also been widely employed in several representative  
 132 models in the field of shallow landslide modeling (Baum et al., 2002; Baum et al., 2008; Baum et al.,  
 133 2010; Iverson, 2000). Because groundwater monitoring data are lacking in the study area, and  
 134 precipitation recorded at the Lorne (Mount Cowley) rain station during the 24 hours (September 11)  
 135 preceding the simulation period (September 12–14) totaled only 1 mm (data provided by the Bureau  
 136 of Meteorology), it is reasonable to assume that the initial groundwater level coincides with the top of  
 137 the impermeable layer.

138 We also recognize that the above parameter settings involve certain scientific simplifications and  
 139 limitations due to spatial variability. In future work, we will incorporate additional field observations  
 140 and sensitivity analyses to refine the model assumptions.  
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142 3. Controlled burn laboratory experiments: the authors used a laboratory furnace to simulate burn  
143 conditions using soil samples collected from a nearby unburned area with similar bioclimate and  
144 pedogenic characteristics. There are multiple points:

145 1. Why were burn experiments conducted at such an extreme temperature (800 °C), and how  
146 dependent are the measured soil properties on this temperature? Most controlled laboratory  
147 burn experiments go up to ~500-600 °C at the surface, even for the high-severity burn cases  
148 (e.g., Doerr et al., 2004; Massman, 2015; Moody et al., 2005; Moody et al., 2009; Wieting et  
149 al., 2017); even when surface soil temperatures exceed 800 °C, subsurface temperatures are  
150 unlikely to approach this value (Robichaud and Hungerford, 2000).

151 2. With regard to the minidisk infiltrometers, did the authors use the equation of Vandervaere et  
152 al. (2000) to compute hydraulic conductivity, or a different method? And, were only 5  
153 measurements taken using the minidisk infiltrometer for the unburned soil? The fit of the curve  
154 to data in Figure 8a shows considerable scatter, and estimates of hydraulic conductivity from  
155 minidisk infiltrometers tend to improve with longer measurement times due to its scaling with  
156  $(t^{0.5})^2$ .

157 **AC:** Thank you for your valuable comment. We have provided a response below and will revise the  
158 manuscript accordingly.

159 We determined 800°C as the maximum temperature for the experiment, mainly referring to relevant  
160 studies indicating that the peak temperature at the soil surface could indeed reach or even exceed  
161 800°C under high-intensity wildfire conditions (Soto et al., 1991; Mataix-Solera et al., 2011; Goudie  
162 et al., 1992). On the other hand, we also recognize that soils have low heat transfer and temperature  
163 decreases rapidly with depth. As you pointed out, this maximum high temperature is primarily  
164 confined to the surface layer of soils. Therefore, this maximum temperature setting for this study is  
165 focused on simulating the worst-case situation for shallow surface soils on slopes under the influence  
166 of high-severity fires. We will make this point clear in the revised manuscript. In addition, we  
167 recognize the complexity of profile temperature gradients and soil response at different depths, and  
168 consider it as a direction for future research expansion.

169 In this study, hydraulic conductivity was calculated using the method proposed in the manual of the  
170 Mini Disk Infiltrator (Zhang, 1997). Specifically, cumulative infiltration ( $I$ ) was plotted against the  
171 square root of time ( $\sqrt{t}$ ), and the resulting data were fitted with the following function:

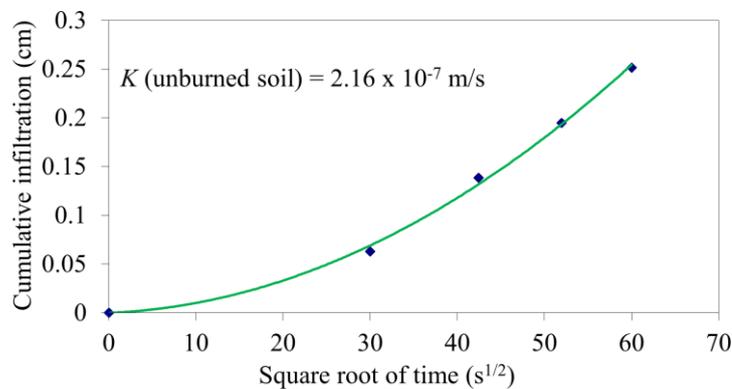
$$172 \quad I = C_1\sqrt{t} + C_2t \quad , \quad (4)$$

173 where  $C_1$  ( $\text{m s}^{-1}$ ) and  $C_2$  ( $\text{m s}^{-1}$ ) are parameters.  $C_1$  is related to soil sorptivity, and  $C_2$  is a fitting  
174 parameter. The hydraulic conductivity ( $K$ ) was then computed from:

$$175 \quad K = C_1/A \quad , \quad (5)$$

176 where  $A$  is a value related to the van Genuchten parameters for the soil texture, the suction rate, and  
177 the radius of the infiltrometer disk;  $A$  values for common soil textures and suctions are provided in  
178 the instrument manual (Carsel and Parrish, 1988). The Mini Disk Infiltrometer infiltrates water at a  
179 suction of  $-0.5$  to  $-6$  cm and has a radius of 2.25 cm.

180 In the Mini Disk Infiltrometer test on unburned soil, the suction was set to  $-2$  cm and the experiment  
181 was run for 3600 s. Infiltration was recorded every 900 s, yielding five data points and a cumulative  
182 infiltration volume of 4 mL (equivalent to 0.25 cm of infiltration). We attempted to extend the test,  
183 but because the soil has very low permeability, the infiltration rate remained slow. Therefore, adding  
184 further data points was of limited practical value. We found an error in Figure 8a in the original  
185 manuscript and will correct it in the revised version (Figure R3), but this correction does not affect  
186 the input parameters and the main conclusions.



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188 **Figure R3: Infiltrometer test results for hydraulic conductivity before burning.**

189 4. Expanded literature review on post-fire infiltration rates: a throughline of the manuscript is that  
190 infiltration increases after fire. For instance, the authors state in the first line of the manuscript that  
191 “Bushfire... leads to... increased soil infiltration.” As a general statement, this is not true. The  
192 response of soil hydraulic properties and infiltration rates to fire is complex (e.g., Shakesby and Doerr,  
193 2006), but it has most commonly been found to substantially decrease after fire due to hydrophobicity  
194 effects and return to pre-fire levels over a number of years (e.g., Cerdà, 1998; Ebel, 2019; Ebel et al.,  
195 2022; Larson-Nash et al., 2018; Lucas-Borja et al., 2022; Moody et al., 2015; Nyman et al., 2014;  
196 Robichaud, 2000; see McGuire et al. (2024) and references therein for a thorough review). The authors  
197 allude to this fact towards the end of the manuscript on lines 314-318. This is not to say that infiltration  
198 rates can be higher in the first year after fire (e.g., Hoch et al., 2021; Perkins et al., 2022), but the issue  
199 is more nuanced than currently stated. The manuscript would be strengthened by a more thorough  
200 description of the literature on this topic in the Introduction and would provide the authors a base from  
201 which to argue that soil hydrophobicity had diminished at their study site at the time of the landslides,  
202 less than 1 year after fire containment, resulting in higher infiltration rates than before the fire. The  
203 authors are encouraged to use the citations included here in addition to others they may wish to include.  
204

205 **AC:** Thank you for your constructive comments, which have provided valuable guidance for  
206 improving our manuscript. Below are our detailed responses and planned revisions.

207 We agree with the reviewer that our original statement in the first line of the manuscript, “*Bushfire...  
208 leads to... increased soil infiltration*” is overly generalized. This wording does not reflect the  
209 complexity and temporal variability of post-fire infiltration responses and is therefore not sufficiently  
210 accurate. Our initial intention was to emphasize that vegetation loss after a relatively long timescale  
211 post-fire may lead to increased infiltration rates. However, such a generalization may cause  
212 misunderstanding, and we will correct this statement in the revised manuscript. Additionally, we  
213 acknowledge the reviewer’s point that many studies reported an initial decrease in soil infiltration after  
214 fire due to the development of hydrophobicity in the surface soil layer. In the revised manuscript, we  
215 will clarify that post-fire changes in soil infiltration are complex, typically characterized by an initial  
216 decrease followed by a potential increase over time as a result of vegetation loss.

217  
218 We agree with the suggestion that the literature review requires expansion. We have reviewed the  
219 references listed by the reviewer and are providing the following brief summary of the content and  
220 findings of each study. In the revised introduction, we will add and discuss the references listed by the  
221 reviewer above as well as other relevant studies. Based on the expanded literature review, we will  
222 clearly present the widely accepted view that post-fire infiltration rates typically show a significant  
223 initial decrease followed by gradual recovery over subsequent years. In addition, we will highlight the  
224 specific context of our study site, where higher soil infiltration rates were observed approximately 10  
225 months after the fire. To explain this phenomenon, we will include additional supporting references,  
226 and provide a more comprehensive discussion of the scenarios and mechanisms that may lead to  
227 increased infiltration rates in the short term after fire, thereby supporting the validity of the increased  
228 infiltration proposed in this study.

- 229 • Cerdà (1998) investigated changes in overland flow and infiltration after a rangeland fire in a  
230 Mediterranean scrubland using simulated rainfall experiments. The study found that as  
231 vegetation gradually recovered, infiltration rates increased each year, while overland flow  
232 decreased from 45% immediately after the fire to less than 6% five and a half years later. Most  
233 of the reduction in overland flow occurred within the first two years after the fire.
- 234  
235 • Ebel (2019) analyzed 73 sets of plot-scale field-saturated hydraulic conductivity ( $K_{fs}$ ) data  
236 collected within one year after fire and found that  $K_{fs}$  decreased significantly, reaching 46%  
237 or less of the values observed in unburned plots. No significant differences in  $K_{fs}$  were  
238 observed between wildfire and prescribed fire, or between moderate and high fire severity.  
239 The study also suggested that variations in  $K_{fs}$  were more strongly influenced by the  
240 measurement method than by spatial scale.
- 241  
242 • Ebel et al. (2022) conducted four years of continuous monitoring following the 2013 Black  
243 Forest Fire in Colorado, USA, assessing six plots with different initial fire severities. The  
244 study analyzed temporal changes in soil-physical and -hydraulic properties—including plot-

245 scale field-saturated hydraulic conductivity ( $K_{fs}$ ), sorptivity ( $S$ ), wetting front potential  
246 ( $\psi_f$ )—as well as surface cover. The results showed that, after the fire, soil bulk density  
247 increased and porosity decreased, with these changes being most pronounced in high-severity  
248 burn areas. Over time, both soil physical and hydraulic properties showed a trend of recovery,  
249 although the recovery was slower in areas with high fire severity.

- 251 • Hoch et al. (2021) examined temporal changes in rainfall thresholds for debris flow initiation  
252 following wildfire, using three to four years of continuous field and remote sensing  
253 observations from three burn areas in the southwestern United States, combined with  
254 hydrological modeling. The study found that in the first year after the fire, soil infiltration  
255 rates were at their lowest, so even rainfall events with a one-year recurrence interval could  
256 trigger debris flows. As soil and vegetation recovered, infiltration capacity increased, and the  
257 rainfall threshold for debris flow initiation rose substantially, so that by the third year, rainfall  
258 events with recurrence intervals of 10 to 25 years were required to trigger debris flows.
- 260 • Larson-Nash et al. (2018) assessed the effects of high-severity fire on small-scale soil  
261 infiltration and erosion over five years following the 2003 Hot Creek Fire in Idaho, USA. The  
262 results indicated that, due to slow vegetation recovery, the median vegetation cover in burned  
263 areas was only 6–8% five years after the fire. Infiltration rates in burned plots consistently  
264 remained lower than those in unburned plots, and rill sediment yield remained significantly  
265 higher. Relative infiltration rates at shallow depths (1 and 3 cm) showed a moderate to strong  
266 correlation with total non-steady infiltration, indicating that shallow soil hydrophobicity was  
267 the primary factor limiting infiltration.
- 269 • Lucas-Borja et al. (2022) assessed the variability of soil hydraulic conductivity ( $K$ ) and soil  
270 water repellency (SWR) under different fire severities and soil depths in Spanish pine forests  
271 and reforested areas. The results indicated that higher fire severity caused more pronounced  
272 reductions in surface  $K$  and SWR, with maximum decreases of up to 80–90% observed even  
273 under low to moderate fire severity. Reforested areas were highly sensitive even to low-  
274 severity fires, whereas surface soil water repellency in natural pine forests was less affected.
- 276 • McGuire et al. (2024) highlighted that wildfire significantly enhances sediment transport  
277 processes by altering soil infiltration capacity, vegetation cover, and sediment availability,  
278 with fire severity being a critical factor influencing the trajectories of these geomorphic state  
279 variables. Approximately 87% of post-fire debris flow events occur within two years after a  
280 wildfire, while landslide-type debris flows tend to occur more than one year after the fire.  
281 Changes in geomorphic state following wildfire are most pronounced in the early stages, but  
282 may gradually recover to pre-fire conditions within several years or transition to a new steady  
283 state, demonstrating diverse recovery pathways.
- 285 • Moody et al. (2015) investigated the effects of burn severity on soil hydraulic properties by  
286 measuring field-saturated hydraulic conductivity ( $K_{fs}$ ) and sorptivity ( $S$ ) of soil samples from

287 the Black Forest Fire area in Colorado, USA, using a laboratory tension infiltrometer under  
288 different levels of burn severity, as indicated by the differenced normalized burn ratio (dNBR).  
289 The results showed that soil hydraulic properties ( $K_{fs}$  and  $S$ ) declined significantly under  
290 high burn severity (dNBR > 420), and the study was the first to establish a quantitative  
291 relationship between these properties and burn severity, providing a basis for post-fire  
292 hydrological prediction.

- 293
- 294 • Nyman et al. (2014) quantitatively evaluated the effects of surface storage ( $H$ ), macropore  
295 hydraulic conductivity ( $K_{mac}$ ), and matrix hydraulic conductivity ( $K_{mat}$ ) on soil infiltration  
296 in three fire-affected areas in southeastern Australia following the 2009 Black Saturday  
297 wildfires. A simplified two-layer infiltration model was used in combination with laboratory  
298 tension infiltrometer experiments on soil samples. The results showed that the wildfire  
299 increased surface soil water repellency, resulting in initial suppression of infiltration, followed  
300 by gradual recovery of macropore flow and matrix hydraulic conductivity.
  - 301
  - 302 • Perkins et al. (2022) investigated the multi-stage recovery of soil hydraulic properties  
303 following the 2017 Nuns and Tubbs wildfires in Northern California. Forty-one monitoring  
304 sites were established in grassland, shrubland, and forest areas, where field-saturated hydraulic  
305 conductivity ( $K_{fs}$ ) was repeatedly measured using a tension disc infiltrometer over a 3.5-year  
306 period. The study found that  $K_{fs}$  declined significantly after the fire, but partially and rapidly  
307 recovered after the first rainy season, exhibiting a complex, seasonal, multi-stage recovery  
308 process.
  - 309
  - 310 • Robichaud (2000) investigated the effects of prescribed fire on soil infiltration rates in  
311 Northern Rocky Mountain forests, USA. Simulated rainfall experiments were conducted in  
312 unburned, low-severity, and high-severity burn areas across different forest types to measure  
313 infiltration rates. The results showed that runoff rates increased significantly at the onset of  
314 rainfall in high-severity burn areas, surface soil water repellency was enhanced, and  
315 infiltration rates temporarily decreased by 10% to 40%. Infiltration rates in unburned and low-  
316 severity burn areas showed little change.
  - 317
  - 318 • Shakesby and Doerr (2006) systematically reviewed the effects of wildfire on soil hydraulic  
319 properties and infiltration rates, highlighting the complexity of these responses influenced by  
320 fire severity, soil type, and environmental conditions, with significant regional variations.  
321 Furthermore, the long-term and large-scale impacts of wildfire on hydrological and  
322 geomorphological processes remain to be thoroughly investigated. This reference is also cited  
323 in Lines 26–27 of the original manuscript: “*Bushfires not only cause immediate losses but*  
324 *also lead to lasting hydrological and geomorphological changes, affecting soil*  
325 *physicochemical properties for months to years (Shakesby and Doerr, 2006).”*
  - 326

327 5. Uncertainty of the results: as alluded to in points 1 & 2, the study currently does not consider the  
328 uncertainty involved with model selection and parameter choices. This makes it difficult for this  
329 reviewer to place confidence in the general predictive ability of the method. The manuscript would  
330 benefit from one of a sensitivity analysis of models and model parameters, incorporation of uncertainty  
331 through e.g., a Monte Carlo sampling strategy, or inclusion of additional observations in order to  
332 separate the data into calibration and validation datasets. However, this reviewer acknowledges that  
333 this may difficult or not possible to accomplish due to the limited availability of target data.

334 **AC:** Thank you for your valuable suggestions. We agree with the reviewer that the modeling and  
335 parameter uncertainty analyses will contribute to the persuasiveness of the study. However, due to the  
336 limited observed data and research conditions, we are currently having difficulties in conducting the  
337 suggested analyses above. We will explain these limitations in the revised manuscript and consider  
338 them as a direction for future research.

339 Lines 104-106: how deep-seated were these landslides?

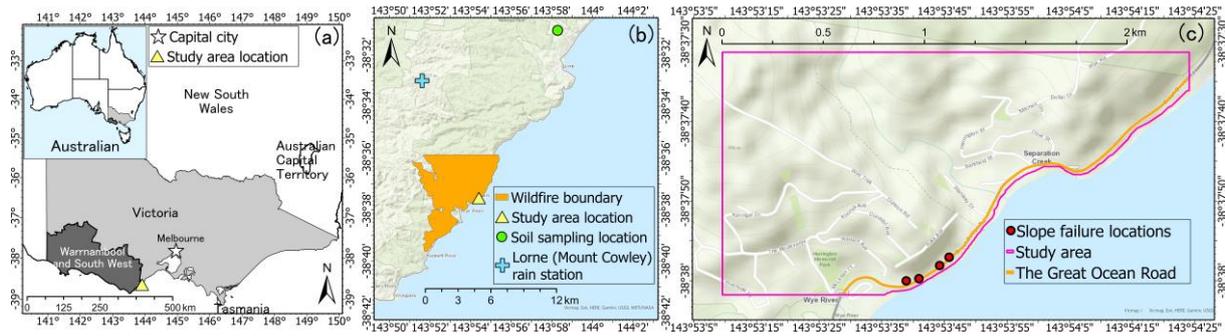
340 **AC:** Thank you for your valuable comment. Currently, explicit information regarding the depth of the Paddy's  
341 Path landslide is not available. Based on the field photographs (**Figure R4**), it is inferred to be a shallow  
342 landslide. We will include this clarification in the revised manuscript.



343  
344 **Figure R4:** The Paddy's Path landslide observed along the Great Ocean Road during the rainfall period (Colac Herald, 2016;  
345 Colls and Miner, 2021).

346 Line 111: please zoom out on the context map in Figure 1a, readers unfamiliar with Australia will be unable  
347 to discern the location of the study area within the continent

348 **AC:** Thank you for this helpful suggestion. We have revised **Figure 1** to better show the location of the study  
349 area within the Australian continent, as shown below.



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**Figure 1: (a) Location of the study area (created using ArcGIS; data from the Australian Bureau of Statistics). (b) Spatial relationship among the wildfire-affected boundary, study area, and soil sampling locations (adapted from Colls and Miner, 2021). (c) Extent of the study area and slope failure locations (created using ArcGIS; data from ESRI).**

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Lines 137-139: Can the burn severity map be added as an overlay to Figure 1? This would help the reader to understand the burn severity within the study area. This sentence would likely fit better in Section 2.

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**AC:** Thank you for your valuable comments. We agree that adding a burn severity map as an overlay in Figure 1 will enhance the understanding of readers. However, on-site measurements of soil burn severity and sufficiently high-resolution dNBR products are currently difficult to obtain. Therefore, we are unable to provide a burn severity map in the figure. We appreciate your understanding and constructive suggestions. Additionally, we will move the relevant sentence to Section 2 in the revised manuscript.

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Line 177: please include how more information about the infiltrometer measurements (e.g., the suction head used, the duration of time/volume that measurements were taken for) and what equations were used to calculate hydraulic conductivity.

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**AC:** Thank you for your valuable comments. We have provided a response below and will revise the manuscript accordingly. In this study, hydraulic conductivity was calculated using the method proposed in the manual of the Mini Disk Infiltrometer (Zhang, 1997). For further details, please read the response to the general comment 3 regarding hydraulic conductivity measurement (p. 6 of this document). The measurement details are as follows: For the unburned soil, a suction head of  $-2$  cm was used, with a total measurement duration of 3600 s, and infiltration was recorded every 900 s, resulting in five data points and a total cumulative infiltration of 4 mL (equivalent to 0.25 cm of infiltration). For the burned soil, a suction head of  $-1$  cm was used, with a total duration of 540 s, infiltration recorded every 60 s, resulting in ten data points and a total infiltration of 30.8 mL (equivalent to 1.94 cm of infiltration). In both cases, the measurement was continued until the infiltration rate reached a steady state.

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Line 189: please include the equation(s) used to fit  $c$  and  $\phi$  to the data

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**AC:** Thank you for your helpful suggestion. In this study, we used the Mohr-Coulomb failure criterion ( $\tau = c + \sigma \tan \phi$ ) to determine the cohesion ( $c$ ) and internal friction angle ( $\phi$ ), where  $\tau$  is the shear strength and  $\sigma$  is the normal stress. Specifically, for the unburned soil, the fitted equation is  $\tau = 0.8193 \sigma + 6.4119$ ; for the burned soil, the fitted equation is  $\tau = 0.8463 \sigma + 0.4462$ . The fitted values were rounded to two decimal places. We will include these equations in the revised manuscript to improve clarity.

380 Line 223: by “lateral infiltration of groundwater”, do the authors mean lateral motion?

381 **AC:** Thank you for your insightful comment. The term “lateral infiltration of groundwater” refers to the lateral  
382 movement of groundwater within the saturated zone of the slope. To avoid any ambiguity, we will revise  
383 “lateral infiltration” to “lateral seepage flow” in the manuscript.

384 Line 266: relating to general comment 1 above, why is the horizontal component of groundwater motion not  
385 accounted for in the pre-fire case?

386 **AC:** Thank you for your important comment. In the pre-fire case, we adopted a fine sand model that considers  
387 only the vertical infiltration of rainwater. According to the source literature of this model (Ozaki et al., 2021),  
388 both FEM numerical analysis and parametric studies demonstrated that, for slopes composed of fine-grained,  
389 low-permeability soils, rainwater infiltration is predominantly vertical. Thus, water movement can be  
390 reasonably represented using a one-dimensional vertical seepage model, and the horizontal component can be  
391 justifiably neglected. Only in the case involving coarser-grained soils with higher permeability, or where the  
392 slope is particularly steep or has complex subsurface structures, is it necessary to include horizontal flow in  
393 the analysis. We will provide further clarification regarding the neglect of the horizontal component in the  
394 revised manuscript.

395 Lines 294-364: Sections 4.5, 5.1, and 5.2 seem to be closely related to each other; the authors may disagree,  
396 but they could be placed together in their own separate section

397 **AC:** Thank you for your helpful suggestion. We agree with your idea and plan to combine Sections 4.5, 5.1,  
398 and 5.2 into a separate section in the revised manuscript.

399 Line 371: Figure 12 is not referenced in the text

400 **AC:** Thank you for pointing this out. We accept this criticism and will reference Figure 12 in the revised  
401 manuscript to ensure it is clearly linked to the text.

402 Line 372: how were the initial saturation degrees determined? Please provide details in the text on why they  
403 are different for each case

404 **AC:** Thank you for your valuable comment. Below, we has provided an explanation of how the initial degree  
405 of saturation was determined. Based on this explanation, we will include a more detailed description in the  
406 revised manuscript regarding how the initial saturation degrees and the related parameters were determined.

407 Given the difficulty in directly obtaining the initial saturation degree ( $S_{r0}$ ),  $S_{r0}$  was calculated using the  
408 following equation proposed by Lambe and Whitman (1969), based on laboratory test results for water content  
409 ( $w$ ), soil particle density ( $\rho_s$ ), and void ratio ( $e$ ). And  $\rho_w$  is the density of water.

$$410 \quad S_r = \frac{w\rho_s}{e\rho_w} , \quad (6)$$

411 The difference in  $S_{r0}$  between the pre-burned and post-burned conditions mainly results from experimentally  
412 determined differences in water content, soil particle density, and void ratio. Below, we have provided an  
413 explanation of how these parameters  $w$ ,  $\rho_s$ , and  $e$  in the  $S_{r0}$  calculation formula were determined.

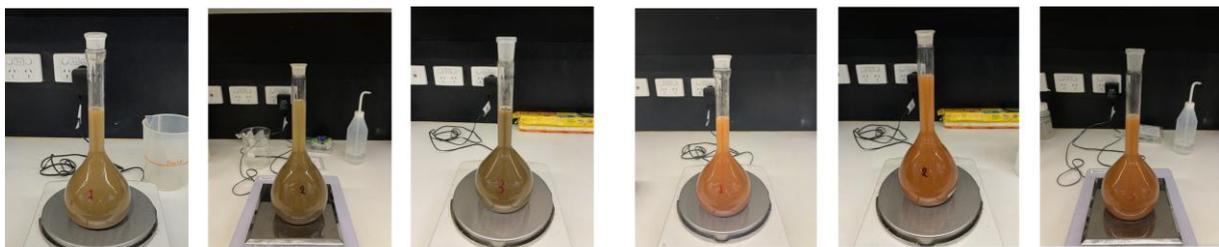
414

- 415 1) Moisture content,  $w$  (%)
- 416 • **Unburned:** Undisturbed soil samples consisted of four cylinder samples A; the mean of four
- 417 measurements was used (see **Figure R5** below).
- 418 • **Burned:** The water content for the burned case was referenced from the unburned condition.
- 419 2) Soil particle density,  $\rho_s$  ( $\text{g cm}^{-3}$ )
- 420 • **Unburned:** Disturbed soil samples were tested using three specimens; the mean of their results was
- 421 used (see **Figure R6a** below).
- 422 • **Burned:** Disturbed soil samples after burning were tested using the same procedures as the unburned
- 423 samples (see **Figure R6b** below).
- 424 3) Void ratio,  $e$
- 425 • **Unburned:** The value was calculated based on the measured  $\rho_s$ ,  $\rho_t$  and  $w$ , using **Eq. (7)** shown
- 426 below (Lambe and Whitman, 1969).
- 427 
$$e = \frac{\rho_s(1+w)}{\rho_t} - 1, \quad (7)$$
- 428 • **Burned:** For simplicity, the moisture content of burned soil samples was assumed to be zero; the same
- 429 calculation method as for unburned samples was applied.
- 430 4) Wet density,  $\rho_t$  ( $\text{g cm}^{-3}$ )
- 431 • **Unburned:** Undisturbed soil samples consisted of four cylinder samples A; the mean of four
- 432 measurements was used (see **Figure R5a** below).
- 433 • **Burned:** Considering the practical difficulties of direct measurement, the dry density measured in the
- 434 laboratory was adopted (see **Figure R7** below).



435 (a) Wet undisturbed cylinder samples A (b) Undisturbed cylinder samples A after being dried

436 **Figure R5: Cylinder samples A for measuring pre-burn water content and wet density.**



437 (a) unburned soil

(b) burned soil

438 **Figure R6: Soil particle density tests of unburned soil (a) and burned soil (b).**



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440 **Figure R7: Undisturbed cylinder samples B for measuring post-burning dry density.**

441 Line 373: Please move the Results and Discussion from Section 5.3 to a stand-alone section, e.g. Section 6  
442 **AC:** Thank you for your constructive suggestion. We will move the Results and Discussion to a new stand-  
443 alone section in the revised manuscript.

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