

# CRHyME (Climatic Rainfall Hydrogeological Modelling Experiment): a new model for geo-hydrological hazard assessment at the basin scale

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**Abstract.** This work presents the new model CRHyME (Climatic Rainfall Hydrogeological Modelling Experiment), a tool for geo-hydrological hazard evaluation. CRHyME is a physically based and spatially distributed model written in Python language that represents an extension of the classic hydrological models working at the basin scale. CRHyME's main focus consists of  
10 simulating rainfall-induced geo-hydrological instabilities such as shallow landslides, debris flows, catchment erosion, and sediment transport into the river. These phenomena are conventionally decoupled from a hydrological routine while in CRHyME they are simultaneously and quantitatively evaluated within the same code through a multi-hazard approach. CRHyME has been tested on some case studies across Northern Italy. Among these, the Caldene catchment, a well-monitored basin of 27 km<sup>2</sup> located near Lecco city (Lombardy), was considered for the calibration of solid transport routine testing also  
15 the spatial scale dependence related to digital terrain resolution. CRHyME was applied across larger basins of the Valtellina (Alps) and Emilia's (Apennines) areas (~2600 km<sup>2</sup>) which have experienced severe geo-hydrological episodes triggered by heavy precipitation in the recent past. CRHyME's validation has been assessed through NSE (Nash–Sutcliffe Efficiency) and RMSE (Root Mean Square Error) hydrological error metrics while for landslides the ROC (Receiver Operating Characteristic) methodology was applied. CRHyME has been able to reconstruct the river discharge at the reference hydrometric stations  
20 located at the outlets of the basins, to estimate the sediment yield at some hydropower reservoirs chosen as a reference, and to individuate the location and the triggering conditions of shallow landslides and debris flows occurred. The good performance of CRHyME was reached assuring the stability of the code, a rather fast computation, and maintaining the numerical conservativity of water and sediment balances. CRHyME has been revealed as a suitable tool for the quantification of the geo-hydrological process, useful for Civil Protection multi-hazard assessment.

## 25 **1 Introduction**

Natural disasters are a critical issue in terms of economic losses and casualties (ISPRA, 2018). Only in 2020, the worldwide losses related to geohazard were quantified as 210 billion dollars and 8'200 victims (Munich Re, 2021). Among the natural disasters, the events linked to geo-hydrological phenomena, such as floods and landslides, certainly play a significant role. Floods and landslides represent serious geo-hydrological hazards in mountain environments (Gariano and Guzzetti, 2016).  
30 Among them, shallow landslide, debris flow failures and soil erosion are controlled by rainfall-triggering events of varying intensity and duration (Abbate et al., 2021a) while sediment transport is a hydrologically driven process occurring at the catchment scale (Brambilla et al., 2020; Papini et al., 2017; Longoni et al., 2016; Ballio et al., 2010). In Italy, a total area of 50'117 km<sup>2</sup>, which corresponds to 16.6% of the national territory is affected by high or very high landslide hazards and/or by a medium hydraulic hazard (ISPRA, 2018). In 2021, the number of victims of landslide and flood events was five and the  
35 evacuated people were around 1'000 (CNR and IRPI, 2021). Northern Italy has the highest mortality rate caused by landslides and floods in the country, varying in the range of 0.034 for Emilia Romagna and 0.085 for Piedmont (number of deaths and missing people per 100'000 people in one year).

Geo-hydrological hazards are complex and heterogeneous phenomena, so a great effort has been made in the past to understand their dynamics (Gariano and Guzzetti, 2016; Ceriani et al., 1994; Gao et al., 2018; Kim et al., 2020). There are many studies

40 concerning shallow landslide dynamics in the literature, based both on laboratory and field experiments (Guzzetti et al., 2007; Herrera, 2019; Meisina et al., 2013; Crosta et al., 2003; Iverson, 2000; Ivanov et al., 2020b), which highlighted the rainfall as the most important triggering factor. Conversely, debris flow and solid transport are primarily influenced by superficial soil water balance in terms of runoff generation through the infiltration mechanisms (Abbate et al., 2019; Jakob and Hungr, 2005; Vetsch et al., 2018). Even though the hydrological cycle is identified as the principal driver of geo-hydrological processes, a  
45 widely accepted methodology able to connect straightforwardly these hazard types and their predisposing and triggering factors is still missing (Gariano and Guzzetti, 2016; Bordoni et al., 2015).

This work will illustrate the potentialities of a new physically-based geo-hydrological model called CRHyME (Climatic Rainfall Hydrogeological Modelling Experiment). CRHyME is an extension of a classical rainfall-runoff hydrological model where geo-morphological dynamic aspects are also simulated. From the analysis of the literature (De Vita et al., 2018; Bemporad et al., 1997; Roo et al., 1996; Schellekens et al., 2020; Angeli et al., 1998; Gleick, 1989; Sutanudjaja et al., 2018; Van Der Knijff et al., 2010; Devia et al., 2015; Moges et al., 2021), rarely the geological and hydrological aspects have been jointly taken into account within the same framework. Lots of hydrological models adopted worldwide are interested mainly in flood propagation and water balance assessment (Sutanudjaja et al., 2018), proposing a very detailed and advanced description of the hydrological cycle. However, geo-hydrological hazard interaction is hardly taken into account (Shobe et al.,  
55 2017; Strauch et al., 2018; Campforts et al., 2020) and it represents one of their main limitations. Up to now, there are still few examples that can include the triggering analysis of shallow landslide and debris flow, or a solid transport quantification (Roo et al., 1996; Gariano and Guzzetti, 2016; Alvioli et al., 2018). In literature, some consider the erosion and solid transport mechanisms at the watershed scale (Vetsch et al., 2018; Tangi et al., 2019; Roo et al., 1996; Papini et al., 2017) while the stability of natural slopes is generally not included. Conversely, the slope stability or debris flow analysis is computed inside  
60 dedicated models (Iverson, 2000; Scheidl and Rickenmann, 2011; Harp et al., 2006; Milledge et al., 2014; Montrasio, 2008; Takahashi, 2009) where strong hypothesis and simplification for the hydrological parameterization are adopted.

Geo-hydrological phenomena have been historically decoupled and investigated separately. To fill this gap and make them more integrated within a numerical model, some attempts were recently proposed. In this regard, CHASM (Combined Hydrology and Stability Model) (Bozzolan et al., 2020) and Landlab (Strauch et al., 2018) represent the two latest modelling  
65 frameworks that have addressed the need to start evaluating the geo-hydrological hazard and risks jointly with hydrological and climatic aspects. The new methodological approaches shown by CHASM and Landlab models have been assessed thanks to the progressive increase of the GIS (Geographical Information Systems) data availability on a worldwide scale and thanks to the recent improvements in computer programming for environmental systems modelling. Indeed, the creation of efficient and open-source built-in functions for different language programs, such as Matlab, C++ or Python, has sped up and facilitated  
70 the implementation of self-made land-surface models. These functions have been already successfully implemented by PCR-GLOBWB-2 (Sutanudjaja et al., 2018) and WFLOW (Schellekens et al., 2020) models, as well as in the European hydrological model LISFLOOD (Van Der Knijff et al., 2010) and OPENLISEM (Roo et al., 1996).

Among the currently available modelling approaches and software solutions, a comprehensive multi-hazard model specifically designed for evaluating geo-hydrological threats is still needed. Geo-hydrological processes are many and happen  
75 simultaneously at a watershed scale. They require to be modelled together to better know their mutual influences and feedback, trying to overcome the theoretical subdivisions existing in the literature's methodologies. Moreover, diversified input data formats, their spatial and temporal resolution, and the scale dependency of geo-hydrological simulation (Devia et al., 2015; Moges et al., 2021) represent real challenges to be carefully addressed to not undermine the applicability of these integrated models. Under these premises, the main motivations aimed at the construction of the new CRHyME code are here presented:

- 80     ▪ Build an integrated but versatile model for simulating rainfall-induced geo-hydrological processes (flood, erosion, sediment transport and shallow landslide triggering);

- Allow fast and efficient calculations within a spatially distributed model, designed to operate at catchment scale without constraints on spatial and temporal input data resolution;
- Implementation of a code inside a robust framework, using Opens Source Python libraries which enable fast coding and easy sub-module modifications/integrations;
- Address code compatibility for assimilating input data from Opens Source datasets available at a worldwide scale, permitting a simulation reproducibly in any worldwide catchments.

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Starting from these considerations and taking inspiration from analogue models cited before, CRHyME (Fig. 1) was developed to try to improve overall geo-hydrological modelling, filling the existing gaps and issues. This paper presents the key features and applications of the code. Structure and constitutive equations are reported in the Material and Method section. Then some case studies developed across Italian territory were considered for the calibration and validation of the new model. In the Result section the main outcomes of CRHyME applications are reported and they are extensively commented on within the Discussion and Conclusions sections.

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## 2 Material and Methods

In this paragraph, the CRHyME model's main features, the sub-module structure and their constitutive equations, the input dataset required for its initialization, and a presentation of calibration and validation case studies are illustrated.

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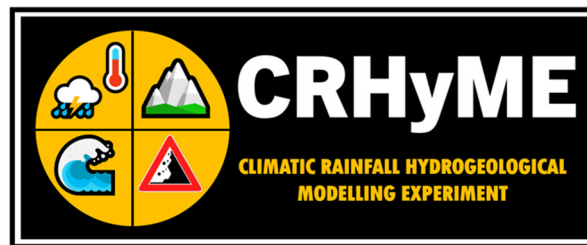


Figure 1: The CRHyME model logo.

### 2.1 Key model features

CRHyME aims to model together rainfall-induced hydrological and geological processes at the catchment scale, e.g. floods and landslides. In CRHyME these processes are evaluated simultaneously within hydrological routine: the bed-load sediment transport has been described considering the Erosion Potential Method (EPM) for simulating erosion sources (Longoni et al., 2016; Brambilla et al., 2020; Milanesi et al., 2015; Ivanov et al., 2020a) and the stream power laws for defining the transport capacity of the rivers (Vetsch et al., 2018); the shallow landslide failure assessment was carried out considering 4 infinite-slope stability models by Iverson (2000), Montrasio (2008), Harp et al. (2006) and Milledge et al. (2014); the debris flows behave intermediately among floods and landslides and their stability assessment was evaluated through the theory proposed by Takahashi (2009), Theule (2012) and Jakob and Jordan (2001).

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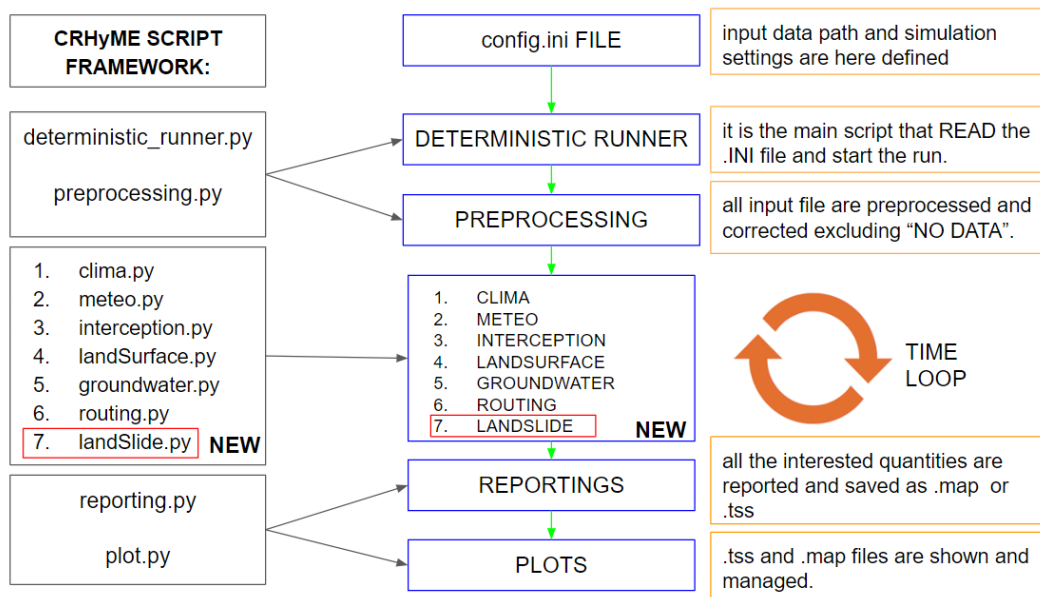
The CRHyME's code architecture is partially inherited by the PCR-GLOBWB-2 model (Sutanudjaja et al., 2018). This model is characterized by a well-organized framework that could guarantee the robustness and stability of the code, fast modelling and reduced time consumption thanks to embedded function parallelization, and no constraints on the spatial and temporal resolution of the input data. The PCR-GLOBWB-2 engine is based on PCRaster libraries (Karszenberg et al., 2010; Pebesma et al., 2007). The PCRaster Python libraries offer a series of standard functions for hydrological processing on calculation grids which can be easily "called" via Python scripts to perform individual operations. CRHyME's framework is organized within a modular structure which enables easier single-model updating and facilitating new features introduction. Python programming language is open-source, and its flexibility permits fast management of GIS databases which are essential for computing geo-hydrological hazard simulations over large domains.

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## 2.2 Model structure

The CRHyME model is composed of a series of modules that run successively in a time-loop as represented in Fig. 2. The simulations are initialised from a pre-compiled “.INP” file (see Appendix A) where all the settings and input data are specified (see Appendix B). The first 6 modules evaluate the hydrological cycle and constitute the “hydrological module”. The CRHyME’s novelty is the “landslide module” where slope instability conditions and sediment transport dynamics are simulated considering the computed soil moisture and the runoff respectively. The modules included in the code are:

1. CLIMA: elaborates precipitation and temperature data from reanalysis and climate datasets, using the NetCDF (Network Common Data Form, “.netcdf”) format (Bonanno et al., 2019; Sutanudjaja et al., 2018);
2. METEO: elaborates precipitation and temperature data from ground-based weather stations using the PCRaster standard format “.tss” (Karssenberget al., 2010) for data series and calculates the evapotranspiration;
3. INTERCEPTION: excludes from the net precipitation the canopy interception and computes the snow dynamic;
4. LANDSURFACE: evaluates the water balance in the superficial soil giving information about runoff, soil moisture and percolation losses;
5. GROUNDWATER: evaluates the water balance in the groundwater layer;
6. ROUTING: calculates the runoff routing across the watershed;
7. LANDSLIDE: identifies the triggering conditions for landslides and debris flows, and calculates erosion and bed-load sediment transport in rivers.



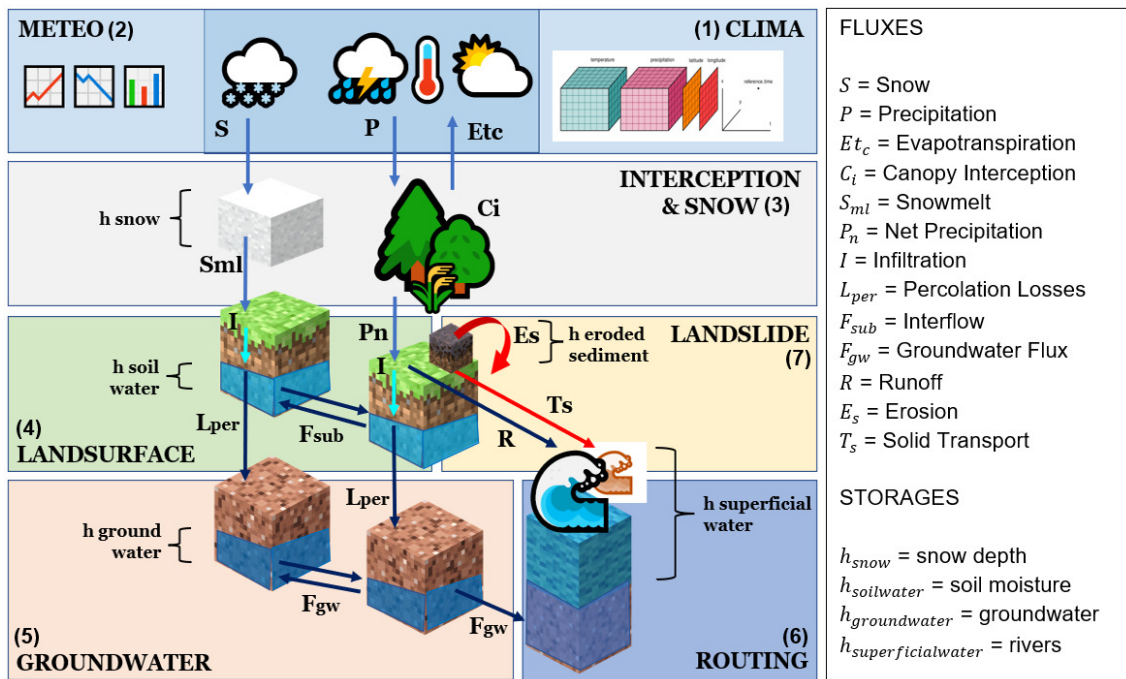
**Figure 2: Framework of the model CRHyME. Main Python scripts are listed, explaining their functionality and their link with the other parts of the code. For further details see Appendix A and B.**

### 2.2.1 Model initialization

A fundamental starting point for CRHyME’s code initialization consists of the choice of a suitable digital elevation model (DEM). From DEM all the essential data listed in the “.INP” file are derived: the “clone.map”, a 0-1 mask that defines the computational domain, and the “ldd.map”, the local drain direction map (Karssenberget al., 2010; Pebesma et al., 2007). In CRHyME, the HydroSHEDS DEM (Hydrological data and maps based on SHuttle Elevation Derivatives at multiple Scales, <https://www.hydrosheds.org>) (Lehner et al., 2008) was taken as a reference. The HydroSHEDS DEM is designed specifically for hydrological models and has been already pre-processed to guarantee the flow connectivity of the river network (hydrologically conditioned). Its spatial resolution is about 3-sec degree, which corresponds approximately to about 90 m at

the equator, and it was retained sufficiently accurately for medium-scale catchment analysis. Using the built-in PCRaster functions, the 'flow accumulation', the 'slope', the 'curvature' and the 'slope-aspect' data were reconstructed immediately from HydroSHEDS DEM. In addition to these morphological data, other informative layers are required for geo-hydrological assessment:

- the Corine Land Cover data (<https://land.copernicus.eu>) (Girard et al., 2018) is the European inventory of landcover that was considered for defining vegetation interception and soil infiltration coefficients, spatial evapotranspiration flux and root cohesion for landslide stability;
- the Soil Grids data at 250 m resolution from the world database ISRIC (International Soil Reference and Information Centre) — World Soil Information (<https://maps.isric.org/>) (Hengl et al., 2017), were considered for assessing soil physical properties such as depth and soil composition which are implemented inside infiltration, percolation, erosion and landslide stability routines;
- the hydraulic properties of soils, such as the permeability and porosity, from the European Soil Data Centre (ESDAC) database (<https://esdac.jrc.ec.europa.eu/>) and other worldwide repositories (Tóth et al., 2017; Ross et al., 2018; Huscroft et al., 2018), were considered for assessing superficial and groundwater hydrological balance.



**Figure 3: Scheme of the soil water and sediment balances and related mass-fluxes implemented in CRHyME. Fluxes and Storage variables constituting the model are listed.**

The datasets here described are freely available for the entire European area, but analogous could be found for other continents. Since they are provided with an open-source licence they have been implemented without restrictions. This choice aims to extend and generalise as much as possible the reproducibility of CRHyME's simulations in any worldwide catchments avoiding any constraint on territorial input data. Moreover, these databases provide free Web Feature Service (WFS) and Web Coverage Service (WCS) services, allowing to download them more easily and speeding up CRHyME initialization.

Temperature and rainfall data required by simulations were gathered from ground-based meteorological stations (Rete Monitoraggio ARPA Lombardia; Rete Monitoraggio ARPA Emilia) and locally available reanalysis databases (Bonanno et al., 2019). Temperature fields were built by combining the data series at each timestep, estimating the regression coefficient with respect to the station elevation and then using the DEM information to calculate the temperature distribution (Daly et al., 1997; Chow et al., 1988). For rain gauge precipitation, a simple IDW (Inverse Weight Distance) interpolator was implemented with a distance exponent equal to 2 while for rainfall data coming reanalysis data, a simple nearest-neighbour algorithm has

been adopted to downscale the precipitation field at DEM resolution (Daly et al., 1997; Chow et al., 1988; Abbate et al., 2021b; Terzago et al., 2018). CRHyME's timestep required for completing a single loop of all internal modules (Fig. 2) was assumed to be equal to the meteorological forcings timestep and could vary from a minimum of 5 min up to a maximum of 1 day. In this current work, the timestep selected for CRHyME's computations is 1 day.

## 180 2.2.2 Hydrological modules and equations

The hydrological modules (Fig. 2 and 3, from 1 to 6) evaluate the processes of transformation inflows-outflows using input maps of weather forcings consisting of precipitation  $P$  [mm timestep<sup>-1</sup>] and average, maximum, and minimum temperature  $T$  [C°]. In CRHyME, each cell of the terrain domain is considered like a tank that communicates in cascade to the others following the downstream river network (Brambilla et al., 2020; Roo et al., 1996; Sutanudjaja et al., 2018). Hydrological  
185 balance is schematized considering 4 imaginary layers where water can be temporarily stored:

1. Snow Storage, Eq. (1) where the snow balance is assessed by the quantity  $h_{snow}(t)$ , [mm];
2. Superficial Soil Storage, Eq. (2) and (3) where soil infiltration is computed and the superficial soil balance is assessed by the variable  $h_{soilwater}(t)$ , [mm];
3. Groundwater Soil Storage, Eq. (5) where the groundwater balance is assessed by the quantity  $h_{groundwater}(t)$ , [mm];
- 190 4. Runoff Storage, Eq. (6) where the runoff generated by an excess of infiltration and exfiltration is routed across the catchment and described by the variable  $h_{runoff}(t)$ , [mm].

The superficial soil storage is the core of hydrological balance assessment where all the water mass fluxes, in [mm timestep<sup>-1</sup>], are exchanged between atmosphere and terrain. Balances are schematized by Eq.(1), Eq. (2) and Eq. (3).

- Infiltration balance in Eq. (2) establishes the net water volume  $I(t)$  that enters the soil. From precipitation  $P(t)$  the net precipitation arriving at the soil interface  $P_n(t)$  is evaluated by subtracting the rainfall intercepted by tree leaves, e.g. Canopy Interceptions  $C_I(t)$  (Li et al., 2017; Nazari et al., 2019). When the temperature  $T$  is < 0°C, all the precipitation is stored as snowpack  $h_{snow}(t)$  Eq. (1) and released aftermath as snowmelt contribute  $S_{ml}(t)$  when temperature increases above 0°C following a degree-day approach (Chow et al., 1988; Cazorzi and Dalla Fontana, 1996).  $I(t)$  is estimated directly using the common infiltration methods proposed by Horton and SCS-CN (Chow et al., 1988; Chen and Young, 2006; Mishra et al., 2003; Morbidelli et al., 2018; Ravi et al., 1998; Smith and Parlange, 1978; Ross et al., 2018) and the runoff generated by an excess of precipitation at the surface  $R(t)$ , is obtained by the difference of  $P_n(t) - I(t)$ ;
- Superficial soil moisture balance in Eq. (3) permits to evaluate the quantity  $S_m(t)$  which is expressed dimensionless as a ratio between  $h_{soilwater}(t)$  [mm] and the product of terrain porosity  $n$  and the superficial soil depth ( $depth_{soil}$ ). Porosity and superficial soil depth are determined respectively from Tóth et al. (2017), Ross et al. (2018), Huscroft et al. (2018) and Hengl et al. (2017) databases. The other terms of the water balance are:
  - $ETc(t)$  evapotranspiration losses according to Hargreaves and Penman-Montheit formulations suggested by FAO guidelines (Raziei and Pereira, 2013; Allan et al., 1998);
  - $L_{per}(t)$  percolation losses are part of the volume that goes to the groundwater layer, evaluated as a function of the soil water balance in unsaturated conditions using Van Genuchten's functions and parameters (Jie et al., 2016; Van Genuchten, 1980; Daly et al., 2017; Groenendyk et al., 2015; Vitvar et al., 2002; Jackson et al., 2014; Klaus and Jackson, 2018);
  - Exfiltration  $Ex(t)$  is the leakage of water on the surface that occurs after the complete saturation of the superficial soil storage (ponding);
  - $F_{sub}(t)$ , expressed in [m<sup>3</sup> s<sup>-1</sup>], is the sub-surface lateral fluxes generated inside the superficial soil layer through the Dupuit approximation of the Darcy law for water filtration in soils. Here is a correction of the

saturated permeability  $K_s$  [ $\text{m s}^{-1}$ ] considering the relative permeability  $K_r$  [-] caused by the partial saturation conditions has been included in the formula (Van Genuchten, 1980).  $\Delta x$  and  $\Delta y$  represent the cell dimensions in [m].

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$\frac{dh_{snow}(t)}{dt} \cong \frac{\Delta h_{snow}(t)}{\Delta t} = S(t) - S_{ml}(t)$	(1)
$I(t) = (P(t) - C_i(t) + S_{ml}(t)) - R(t) = P_n(t) - R(t)$	(2)
$\frac{d(S_m(t) * depth_{soil} * n)}{dt} = \frac{dh_{soilwater}(t)}{dt} \cong \frac{\Delta h_{soilwater}(t)}{\Delta t} \pm \frac{F_{sub}(t)}{\Delta x * \Delta y} = I(t) - ETc(t) - Ex(t) - L_{per}(t)$	(3)

The groundwater reservoir depth ( $depth_{GW}$ ) has been modelled considering a spatial distribution described in Eq. (4) (Fan et al., 2007; de Graaf et al., 2015; Pelletier et al., 2016). According to these studies, as the superficial slope increases, the aquifer depth is reduced until it reaches the minimum value of 0 m, e.g., corresponding to the condition of complete absence.

$depth_{GW} = a / (1 + b * slope)$	(4)
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In Eq. (4) the slope is expressed as a tangent to the angle of inclination of the surface while  $a$  and  $b$  represent coefficients that are distinguished according to the depths of interest: where the depth of the bedrock is supposed to be low ( $< 10$  m, superficial bedrock), the suggested parameters are  $a = 20$  and  $b = 125$ , while if the bedrock depth is significative ( $> 10$  m, deep regolith)  $a = 120$  and  $b = 150$ . In CRHyME an intermediate condition has been adopted between superficial bedrock and deep regolith, therefore the parameters adopted are the following:  $a = 200$  and  $b = 125$ . This approximation has appeared sufficiently accurate concerning the fact that currently available data on groundwater aquifer depth and hydrogeological parameters are rather approximated, uncertain and with low resolution (Kobierska et al., 2015; Zomlot et al., 2015; Hayashi, 2020; Huscroft et al., 2018).

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$\frac{dh_{groundwater}(t)}{dt} \cong \frac{\Delta h_{groundwater}(t)}{\Delta t} \pm \frac{F_{GW}(t)}{\Delta x * \Delta y} = L_{per}(t) - Ex_{GW}(t)$	(5)
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The groundwater table is generated by the percolated water  $L_{per}(t)$  coming from the upper layer Eq. (5). The groundwater lateral flow  $F_{GW}(t)$ , expressed in [ $\text{m}^3 \text{s}^{-1}$ ], is then calculated using the Dupuit approximation according to which the filtration rate is given by the product of hydraulic permeability for the tangent of the slope of the impermeable substrate, supposed parallel to the slope (Klaus and Jackson, 2018; Anderson, 2005; Bresciani et al., 2014).  $Ex_{GW}(t)$  e.g., groundwater exfiltration, is the term that describes the leakage of water after the complete saturation of the groundwater storage, simulating the water springs.

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$\frac{dh_{runoff}(t)}{dt} \cong \frac{\Delta h_{runoff}(t)}{\Delta t} \pm \frac{F_{kin-dyn}(t)}{\Delta x * \Delta y} = R(t) + Ex(t) + Ex_{GW}(t)$	(6)
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Superficial runoff is defined as the sum of  $R(t)$ ,  $Ex(t)$  and  $Ex_{GW}(t)$  and it is stored in  $h_{runoff}(t)$  in Eq. (6).  $h_{runoff}(t)$  is propagated across the overland surface along the lines of maximum slope and inside the river network using two possible methods available in PCRaster libraries (kinematic and dynamic) that are deputed for the flow routing process (Chow et al., 1988; Lee and Pin Chun, 2012; Collischonn et al., 2017; Bancheri et al., 2020).  $F_{kin-dyn}(t)$  flux, expressed in [ $\text{m}^3 \text{s}^{-1}$ ], is derived from the simplification of De Saint Venant's one-dimensional equations of motion. The *kinematic algorithm* is generally applied in sections where the slopes are accentuated so it is possible to approximate the hydraulic gradient with the

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slope of the channel (Chow et al., 1988). The *dynamic algorithm* instead introduces further terms that allow a better simulation  
 250 of the outflow in correspondence to the flat areas when the other terms of the De Saint Venant equation are no longer negligible  
 (Chow et al., 1988), but requires precise information about the geometry of rivers sections to carry out the flood wave  
 propagation (Karssenberget al., 2010).

### 2.2.3 Geo-hydrological module and equations

To study geo-hydrological instability it is of paramount importance to analyse the triggering causes of landslides and the  
 255 dynamic of erosion and sediment transport processes (Guzzetti et al., 2005; Remondo et al., 2005; Montrasio and Valentino,  
 2016; Bovolo and Bathurst, 2012). In the following paragraphs, the “*landslide module*” features included in CRHyME are  
 described (Fig. 3, n° 7).

#### 2.2.3.1 Stability models for shallow landslides and debris flows

260 Shallow landslide triggering is strongly correlated with meteorological and climatic forcing (Abbate et al., 2021a). The abrupt  
 modifications of the local hydrology with the alternation of dry and wet conditions of soil induced by precipitation are  
 responsible for undermining the stability of the slopes (Iverson, 2000; Chen and Young, 2006). Here are described briefly the  
 four stability models included in CRHyME: 1) the Iverson model (Iverson, 2000), Eq. (7), 2) the Harp model (Harp et al.,  
 2006), Eq. (8), 3) the Milledge model (Milledge et al., 2014) and, Eq. (9), 4) the SLIP model (Montrasio, 2008; Montrasio and  
 265 Valentino, 2016), Eq. (10). In slope stability analysis, the limit equilibrium method based on Mohr-Coulomb criterion is usually  
 adopted to calculate slope stability. The one-dimensional theory considers the hypothesis of an infinitely extended slope  
 characterized by soil thickness  $Z$  [m], plane inclination  $\alpha$  [°], saturated soil  $\gamma_s$  and water  $\gamma_w$  specific weight [kN m<sup>-3</sup>]. The slope  
 stability is evaluated by the Factor of Safety ( $FS$ ), defined as the ratio between the resistant forces due to the friction and the  
 mobilizing forces due to the weight component parallel to the slope. In CRHyME, the one-dimensional model was  
 270 implemented by imagining each cell as a slope element for which the value of the safety factor  $FS$  is calculated. According to  
 the principle of effective stress, as the soil moisture increases, normal efforts are reduced by an aliquot equal to the pressure  
 generated by the water itself (Iverson, 2000).

$FS = \frac{\tan(\varphi)}{\tan(\alpha)} - \frac{\psi\gamma_w \tan(\varphi)}{\gamma_s Z \sin(\alpha) \cos(\alpha)} + \frac{c}{\gamma_s Z \sin(\alpha) \cos(\alpha)}$	(7)
$FS = \frac{\tan(\varphi)}{\tan(\alpha)} + \frac{m\gamma_w \tan(\varphi)}{\gamma_s \tan(\alpha)} + \frac{c}{\gamma_s Z \sin(\alpha)}$	(8)
$FS = \frac{2F_{rl} + F_{rb} + F_{rd} - F_{du}}{F_{dc}}$	(9)
$FS = \frac{N' \tan \varphi + C'}{W' \sin \alpha + F'}$	(10)

The key parameters of the Iverson (Iverson, 2000) Eq. (7) and Harp (Harp et al., 2006) models Eq. (8) are essentially 3: the  
 275 friction angle  $\varphi$  [°] and the cohesion of the soil  $c$  [kPa] which are a function of the terrain granulometry, and the superficial  
 soil moisture  $S_m(t)$  [m]. Inside the Iverson’s model, the soil moisture influence is described through the groundwater pressure  
 head of the local aquifer  $\psi = f(S_m(t))$ , expressed in [kPa], while inside the Harp model is described by the dimensionless  
 variable  $m = \frac{h_{soilwater}(t)}{Z \cdot n}$ , comprised between 0 (completely dry) and 1 (completely wet). The Milledge model (Milledge et al.,  
 2014) in Eq. (9) considers not only the friction effects along the sliding surface  $F_{rb}$  expressed in [N], but also the shear  
 280 resistance along the two parallel and vertical side walls  $F_{rl}$  in [N], the passive force of the upstream terrain  $F_{du}$ , in [N], the  
 active force of the valley terrain  $F_{rd}$  in [N], and the mobilizing force due to the terrain weight  $F_{dc}$ , in [N]. In the SLIP model  
 (Montrasio, 2008; Montrasio and Valentino, 2016) shown in Eq. (10) the terms are expressed in [N]:  $N'$  is the normal



component of the weight as a function of porosity  $n$  and soil moisture  $S_m(t)$ ;  $C'$  is the cohesion term;  $W'$  is the slope parallel component of the weight as a function of porosity  $n$  and soil moisture  $S_m(t)$ ;  $F'$  is the term that expresses the seepage forces that are related to the presence of the temporary water table. Since slopes are vegetated, two other factors should be included: the additional cohesion of the root system of trees and the additional weight of plant biomass (Cislaghi et al., 2017; Yu et al., 2018; Rahardjo et al., 2014). In fact, in the absence of root cohesion, several slope areas would be perpetually in conditions of instability with  $FS < 1$ . The addition of root cohesion, varying between 1 – 10 kPa depending on the tree species and the type of land use was included in the stability evaluation (further details in Abbate and Mancusi 2021a).

A debris flow represents movements of mass that are often triggered on steep slopes and travel long distances reaching the fan close to the watershed outlet (Takahashi, 2009). Debris flows are classified as landslides, although they are among the more fluid types of landslides (Iverson et al., 1997). Therefore, solid concentration within the saturated deposit and the presence of superficial water flowing above are the key parameters for assessing the triggering condition. As can be appreciated by Eq. (11) and (12), two criteria are at least to be included. The first one is derived from the theory of infinite slope stability where the solid concentration parameter  $C_*$  is included as the principal triggering factor. The solid concentration  $C_*$  is the grain concentration by volume in the static debris bed and can be expressed by the ratio between the soil amount [ $m^3$ ] to the sum of the soil amount [ $m^3$ ] and soil water volume [ $m^3$ ]. Increasing the local water volume, the solid concentration starts to progressively reduce. The first criterium in Eq. (11) requires the indication of soil density  $\sigma$  [ $kg\ m^{-3}$ ], water density  $\rho$  [ $kg\ m^{-3}$ ], the surface runoff height  $h_{runoff}$  [ $m$ ] and the parameter  $a_{df}$  that can be assumed equal to the representative diameter of the soil deposit, such as  $D_{50}$ , expressed in [ $m$ ]. The second criterium in Eq. (12) considers that specific superficial runoff discharge  $q_l = \frac{F_{kin-dyn}(t)}{\Delta x}$ , in [ $m^2s^{-1}$ ], flowing above the debris deposit, satisfies the threshold condition  $\geq 2$  for the non-dimensional water discharge  $q^*$  [-], where  $g$  is gravity acceleration [ $m\ s^{-2}$ ]. If these criteria are satisfied under a predetermined rainfall condition that basin could be affected by debris flow triggering.

$FS_{debris} = \frac{C_*(\sigma - \rho)}{C_*(\sigma - \rho) + \rho \left(1 + \frac{h_{runoff}(t)}{a_{df}(D_{50})}\right)} \tan \varphi$	(11)
$q_* = q_l / \sqrt{D_{50}^3 * g} \geq 2$	(12)

### 2.2.3.2 Erosion production and bed-load solid transport routing

Gavrilovic's method (summarized in Eq. 13-14-15) is a semi-quantitative method capable of giving an estimation of erosion and sediment production in a basin (Longoni et al., 2016; Milanese et al., 2015; Globevnik et al., 2003; Brambilla et al., 2020). Initially, it was developed in southern ex-Yugoslavia and then successfully applied in Switzerland and Italy. The mean annual volume of eroded material  $G$ , expressed in [ $m^3yr^{-1}$ ], is a product of  $W_s$  and  $R_{EPM}$ , which are respectively the mean annual production of sediment due to surface erosion, expressed in [ $m^3yr^{-1}$ ] Eq. (14), and the retention coefficient, nondimensional [-] in Eq. (15) considers the possible re-sedimentation of the eroded material across the watershed.

$G = W_s R_{EPM}$	(13)
$W_s = \pi \bar{P} \tau_G(\bar{T}) \bar{Z}_{EPM}^{\frac{3}{2}} A_{Basin} \rightarrow W_{s_{downscaled}} = \pi P(t) \tau_G(T(t)) \bar{Z}_{EPM}^{\frac{3}{2}}(x, y) \Delta x \Delta y$	(14)
$R_{EPM} = \frac{\sqrt{OD}(l + l_{lat})}{(l + 10)A_{basin}}$	(15)

The terms that appear in the equations are:  $\tau_G$  temperature coefficient [ $^{\circ}C$ ] in the function of watershed mean annual temperature  $\bar{T}$  in [ $^{\circ}C$ ],  $\bar{P}$  mean annual precipitation value [ $mm\ yr^{-1}$ ],  $\bar{Z}_{EPM}$  mean erosion coefficient [-],  $A_{basin}$  basin area [ $km^2$ ],  $O$  perimeter of the basin [ $km$ ],  $D$  mean elevation of the basin [ $km$ ],  $l$  length of the main watercourse [ $km$ ],  $l_{lat}$  the

total length of the lateral tributaries [km]. The Gavrilovic method was developed to work with annual data of mean precipitation and temperature. Since with CRHyME, we are interested in a continuous simulation, the method has been temporally and spatially downscaled (Eq. 14) by substituting  $\bar{P}$  and  $\bar{T}$  with the time-series of precipitation  $P(t)$  [mm timestep<sup>-1</sup>] and  $T(t)$  temperature [C°] and calculated for each domain cell ( $A_{Basin} \rightarrow \Delta x * \Delta y$ ). The values of  $Z_{EPM}$  are correlated to the land use characteristics and geological maps (Milanesi et al., 2015; Abbate and Mancusi, 2021a) therefore the coefficient was spatially distributed through these parameters using the conversion table proposed by Globovnik et al. (2003). The Gavrilovic method defines  $W_s$  as the source of available sediment routed through the watershed until the outlet. In CRHyME the sediment routing has been modelled considering its strong relation with the liquid discharge. First of all, the theory of incipient motion of Shields that states the starting motion of sediments in the function of  $D_{50}$  quantity, the median diameter of the soil granulometric curve (Chow et al., 1988; Merritt et al., 2003; Vetsch et al., 2018), is implemented (Fig. 4). The solid discharge is evaluated in two ways. A first calculation considers a stream-power formula for bed load transport (Morgan and Nearing, 2011; Shobe et al., 2017; Campforts et al., 2020). Here, the solid discharge  $Q_s$ , expressed in [m<sup>3</sup> s<sup>-1</sup>], is in function of the reach hydraulic and geometrical characteristics (Fig. 4) and it doesn't consider the local availability of the eroded material in the channel that may decrease/increase the amount of sediment delivered. This first implementation of solid transport routing is also defined as Transport Limited (TL) since only the reach transport capacity is determined. A second calculation consists of an adaptation of the kinematic model for clear water to the sediment transport, under the hypothesis that the velocity of sediment transport is assumed similar to the water flow. The application of the kinematic method requires the estimation of stage-discharge relations for the sediment in analogy with the clear water stage-discharge functions. Several authors (Govers, 1989; Govers et al., 1990; Rickenmann, 1999) have considered this hypothesis reasonable when no further additional information about solid transport is available. For this second case, the sediment balance is required and it has been assessed in each cell domain through Eq. (16) considering: the erosion rate  $E_s$  equal to the source term  $W_s$  computed by EPM and the deposition rate  $D_s$ , following Shobe et al. (2017), expressed in [m<sup>3</sup> yr<sup>-1</sup>]; the transport term  $T_s$  considering the kinematic model adapted for sediment routing, expressed in [m<sup>3</sup> s<sup>-1</sup>]; the sediment amount  $h_{solid}(t)$  in [m], converted in volume [m<sup>3</sup>] if multiplied by cell area extension [m<sup>2</sup>]. This second implementation is representative of the Erosion Limited (EL) condition where the sediment availability in the river or across the slopes is limited by their effective availability, as it frequently happens (Shobe et al., 2017; Campforts et al., 2020; Chow et al., 1988; Davy and Lague, 2009).

$\frac{dh_{solid}(t)}{dt} \cong \frac{\Delta h_{solid}(t)}{\Delta t} \pm \frac{T_s(t)}{\Delta x * \Delta y} = D_s(t) - E_s(t)$	(16)
--	------

In CRHyME both TL and EL methods are evaluated for assessing quantitatively sediment transport yield within a physically reasonable range. According to Papini et al. (2017), Ivanov et al. (2020a), Dade and Friend (1998a), Lamb and Venditti (2016), Peirce et al. (2019), Pearson et al. (2017) and Ancy (2020), the sediment transport dynamic is an active research frontier. In this sense, the spatial distribution of  $D_{50}$  is a critical point because is difficult to be reconstructed at the catchment scale (Abeshu et al., 2021). Moreover,  $D_{50}$  distribution influences incipient motion threshold that sensibly modifies the local sediment routing leading to wrong estimations of the watershed sediment yield (Fig. 4). Since a close formulation doesn't exist for indirectly estimating the granulometry in the absence of an on-field survey dataset, empirical approaches have been proposed by Nino (2002), Sambrook Smith and Ferguson (1995), Lamb and Venditti (2016) and Berg (1995). According to these authors, morphological, climatic, hydrological, and geological factors can influence river granulometry in a particular section. Slope-like factors have shown a quite significant correlation with  $D_{50}$  and in some cases  $slope \rightarrow D_{50}$  relations (power-laws in the form like  $D_{50} = a_x slope^{b_x}$ ) were retrieved (Nino, 2002). Namely,  $D_{50}$  tends to increase with slope steepness. These relations mimic the formula proposed by Berg (1995) where the  $D_{50}$  is indirectly determined using a power-law function describing the river morphology evolution. Even though  $slope \rightarrow D_{50}$  represent a crude approximation it has a

physical meaning since in the upper catchment (where slopes are steepness) coarse granulometries are generally prevalent while at the outlet (where slopes are lower) the sediment fine fraction becomes more significant (Tangi et al., 2019). In CRHyME, the  $D_{50}$  is a necessary granulometric data, therefore an ensemble of empirical  $slope \rightarrow D_{50}$  curves have been proposed to assess automatically  $D_{50}$  distribution across the catchment using the slope data. Curve's parameters were calibrated ad hoc in the examined areas comparing simulated sediment yields to the available measurements and with on-site granulometry surveys.

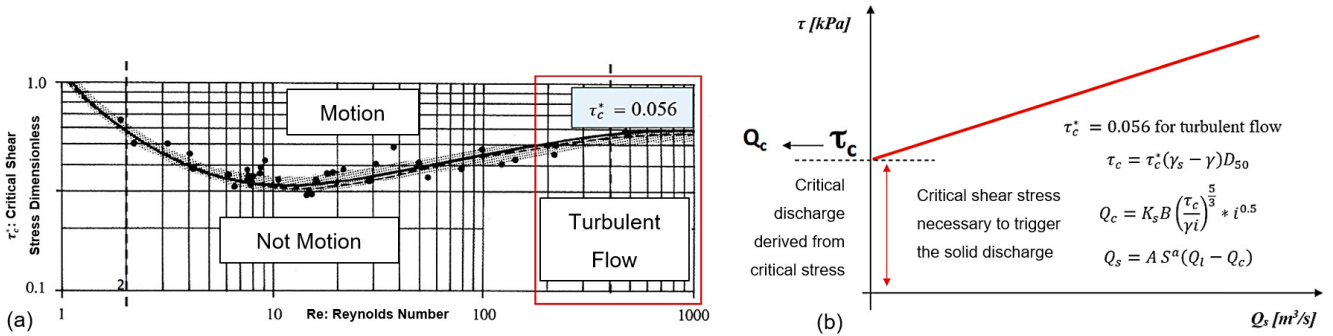


Figure 4: (a) Shield abacus (Chow et al., 1988) for solid transport incipient motion under different conditions of turbulence ( $Re$  Reynolds number). In the red box is defined the typical range of turbulent flow in rivers with a critical dimensionless shear stress  $\tau_c^*$  of 0.056; (b) evaluation of the incipient motion condition for solid transport discharge  $Q_s$  using power-law relation where: the critical shear stress  $\tau_c$  [kPa] and the critical liquid discharge  $Q_c$  [ $m^3 s^{-1}$ ] are a function of saturated grain  $\gamma_s$  and  $\gamma_w$  and water specific weights [ $kN m^{-3}$ ], the local granulometry through the parameter  $D_{50}$  [mm], the roughness  $K_{Strickler}$  [-], the channel width  $B$  [m], the reach slope  $i$  [%] and the two coefficients  $\varepsilon$  [-] (comprised between 1 and 2) and  $c_{pl}$  [-] (comprised 0.94 and 5.8) (Vetsch et al., 2018).

### 2.2.3.3 Connections among simulated geo-hydrological processes

The processes here described may occur simultaneously inside a catchment, especially during heavy rains or after periods of prolonged precipitation (Abbate et al., 2021a). In CRHyME, the erosion and sediment transport are well integrated within the hydrological routines following the state-of-the-art in the literature (Vetsch et al., 2018). Here, both the triggering function (sediment detachment and incipient motion) and the magnitude (amount of sediment eroded and transported) are quantified. On the other side, for shallow landslide and debris flow, only the triggering condition of failure has been analysed while the mass wasting propagation across the catchment has not been included in the code yet. This choice is motivated by the fact that mass wasting failures, especially for debris flows, are characterized by large uncertainties in their volume quantification related mainly to the entrainment processes and their runout strongly depends on DEM accuracy and spatial resolution (i.e. they are spatial scale dependent) (Jakob and Hungr, 2005; Scheidl and Rickenmann, 2011). The entrainment effect is difficult to be modelled in a closed form and it may perturb the volume estimation by orders of magnitude (D'Agostino and Marchi, 2001). Mass wasting processes may have a strong incidence on sediment transport dynamic and compared to widespread erosion, which is a "low intensity" process, landslides may change abruptly the geo-morphological characteristics of the catchment (Iida, 1999; D'Odorico and Fagherazzi, 2003). These issues are not investigated in this present work but are under study.

## 2.3 Model performance

The PCRaster libraries implemented in CRHyME have the advantage of being fully parallelized to work with multicore processors (Karszenberg et al., 2010). This is an important aspect of our code that permits us to decrease sharply the time-consuming of each simulation. The intrinsic parallelization of the PCRaster libraries simplifies and facilitates code maintenance and updating, without any further optimizations. In Table 1 the operating time calculation ranked for the model CRHyME is reported for different numbers of core processors (worker thread).

CPU cores	PCRaster N° Worker Thread	Single Operation on LANDSURFACE Module with a large file (10'000 cells)	Single Cycle (1° to 7° Module) of Model Iteration with a large file (10'000 cells)
2 cores	2	4.07 s	Around 20 – 25 s
4 cores	4	1.48 s	Around 8 – 10 s
8 cores	8	1.05 s	Around 5 – 6 s

**Table 1: Performances of the CRHyME model working on different CPU core sets. By increasing the number of cores available, the computation time for a particular operation can drop significantly.**

### 2.3.1 Hydrological error metrics and sediment transport assessment

Hydrological performance assessment at basin outlets is evaluated through error indexes that compare water discharges recorded by the local hydrometer and the water discharge simulated by the model (Chow et al., 1988; Bancheri et al., 2020). The most common error metrics used in hydrology are the Nash–Sutcliffe Efficiency (NSE), and the Root-Mean-Square Error (RMSE). The Nash–Sutcliffe Efficiency (NSE) in Eq. (17) is a normalized model efficiency coefficient where  $S_i$  and  $M_i$  are respectively the predicted (or simulated) and measured (or observed) values at a given time step  $i$ . The NSE varies from  $-\infty$  to 1, where 1 corresponds to the maximum agreement between predicted and observed values. The Root-Mean-Square Error (RMSE) in Eq. (18) is given by where  $S_i$  and  $M_i$  are respectively the predicted (or simulated) and measured (or observed) time series, and  $N$  is the number of components in the series.

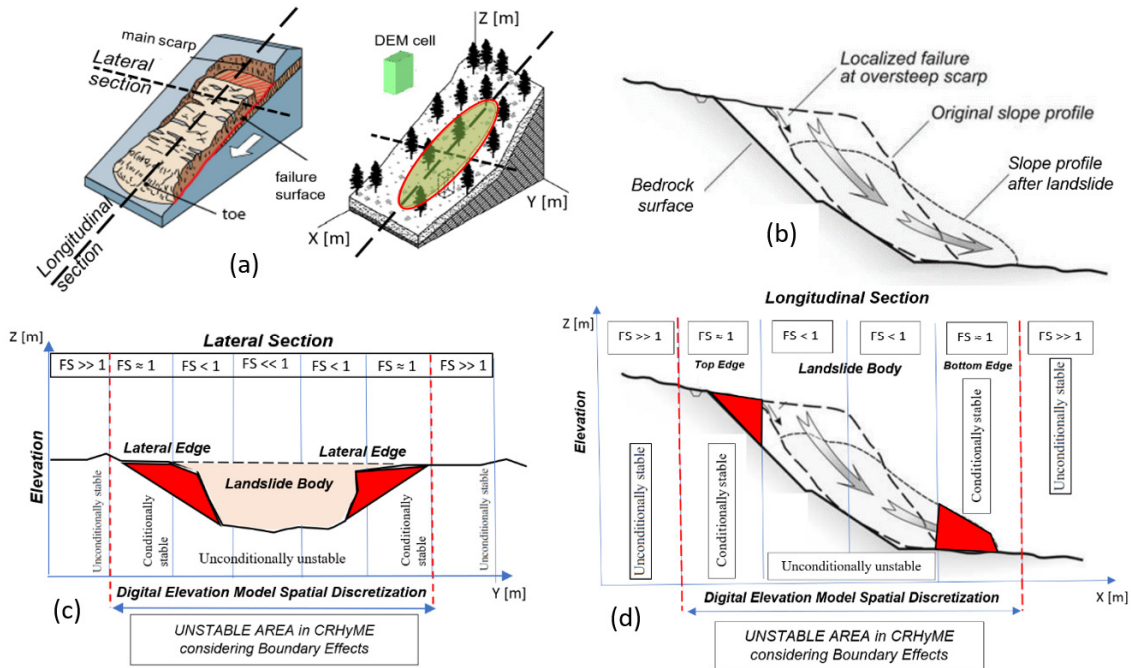
$NSE = 1 - \frac{\sum_{i=1}^n (S_i - M_i)^2}{\sum_{i=1}^n (M_i - \bar{M}_i)^2}$	(17)
$RMSE = \sqrt{\frac{1}{N} \sum_{i=1}^n (M_i - S_i)^2}$	(18)

For the sediment transport assessment, the periodical bathymetry campaigns carried out inside hydropower reservoirs can be considered as a reference for the sediment yield measurement (Pacina et al., 2020; Langland, 2009; Marnezy, 2008). Compared to hydrometric data which can be easily gathered from local environmental agencies (Rete Monitoraggio ARPA Lombardia; Rete Monitoraggio ARPA Emilia), bathymetries are generally not accessible to the public (ITCOLD, 2009, 2016). Therefore, the calibration and validation of erosion and sediment transport models have considered the seasonal volume estimation in hydropower reservoirs and the event-based volume estimation only where available. For the case studies analysed, these data were retrieved also from specific reports (Milanesi et al., 2015; Ballio et al., 2010; Brambilla et al., 2020).

### 2.3.2 ROC curves for local landslide prediction

According to Formetta et al. (2016), Pereira et al. (2016), Vakhshoori and Zare (2018), Gudiyangada Nachappa et al. (2019), Kadavi et al. (2018) and Fawcett (2006), a useful technique to assess the prediction performances of a slope stability model is the Receiver Operating Characteristic (ROC) methodology. The ROC curve illustrates the diagnostic ability of a binary classifier system as its discrimination threshold is varied. In landslide stability assessment, the binary classificatory is the condition of  $FS \geq 1$  (stable) or  $FS < 1$  (unstable) characterizing each pixel of the model domain (Formetta et al., 2016; Vakhshoori and Zare, 2018). In CRHyME, the number of landslide activations is counted. On each timestep, a 0-1 map is produced where the destabilized pixels ( $FS < 1$ ) are signed as 1 while stable pixels ( $FS \geq 1$ ) are signed with 0. This landslide-triggering algorithm is rather simple to be implemented inside a code and other authors have also followed this approach (Harp et al., 2006; Milledge et al., 2014; Formetta et al., 2016). However, the inclusion of a “*pixel range*” in the surrender area of a detected unstable pixel (prone to shallow landslide failure) is necessary to describe the instability activation. Generally speaking, landslide instability areas are not confined to the landslide body but could extend to surrounding boundaries: in the upper part, the landslide crown could experiment with further collapse since other cracks may generate and propagate retrogressively (Ivanov et al., 2020b); in the bottom part, the landslide may evolve into soil slip or earth flow and travel along

the slope following the maximum gradient (Jakob and Hungr, 2005); the lateral boundaries could be also affected by landslide instability due to shear stress perturbation and reduced lateral roots cohesion (Rahardjo et al., 2014) that develops during landslide collapsing. Bearing in mind that a single-pixel slope failure evaluation may be not conservative from a hazard perspective, in CRHyME the unstable area related to the predicted unstable pixel has been extended considering also the surrounded eight adjacent cells, as reported in Fig. 6.a.

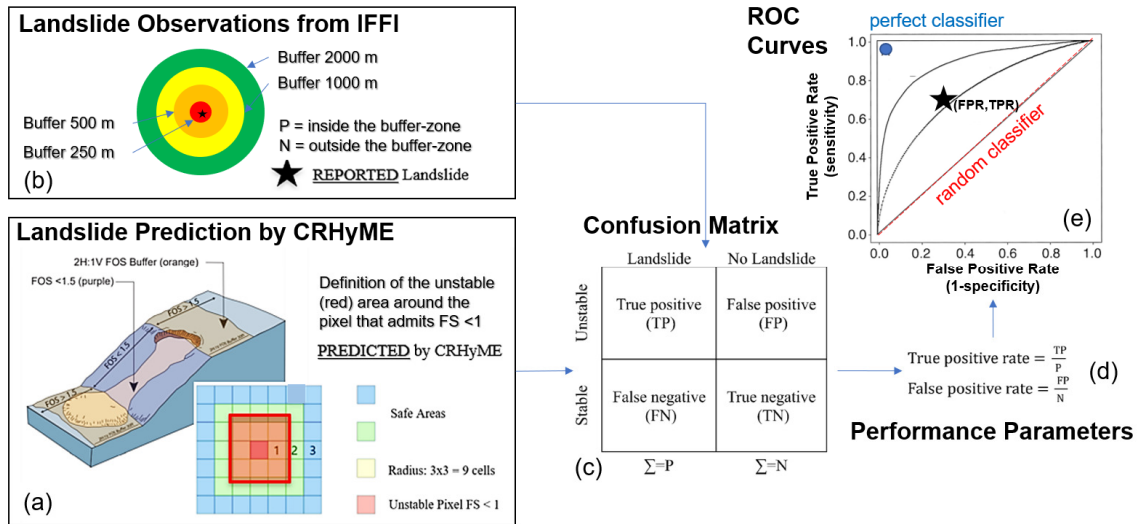


430 **Figure 5: Scale dependence in the infinite slope stability assessment. (a) geometrical sections (longitudinal and lateral) of shallow landslides, (b) landslide kinematics along the longitudinal section, (c) exemplification of the stable and unstable areas in the lateral section, (d) exemplification of the stable and unstable areas in the longitudinal section with respect to DEM resolution. In red the lateral, top and bottom edges of the landslide affected by instabilities are highlighted.**

A 9-pixel counting may overestimate in some cases the extension of the hazardous area because it is also dependent on the DEM resolution. To assure the reasonability of this choice, a survey conducted within the IFFI (Inventario Fenomeni Franosi Italiano) landslide inventory (ISPRA, 2018; Guadagno et al., 2003; Guzzetti and Tonelli, 2004) has shown that typical rainfall-induced shallow landslides have mean and median spatial extension equal to  $\sim 20'000 \text{ m}^2$  and  $\sim 10'000 \text{ m}^2$  respectively, which correspond an indicative pixel size comprised between 150 – 100 m. In our case, the 90 m DEM resolution (sampled at the equator) becomes  $\sim 70 \text{ m}$  at the latitude of the tested case study due to geographical transformation (Lehner et al., 2008). Therefore, a 9-pixel approximation could bring the overall landslide extension equal to  $(70 \cdot 3)^2 \sim 40'000 \text{ m}^2$ , slightly larger compared to the IFFI inventory range but within the same order of magnitude. However, the exact landslide geometry is not definable “a priori” since it has large variability in terms of extension and shape (areas span mostly  $10^3$  to  $10^6 \text{ m}^2$  according to Tanyaş et al (2019)) which could be larger or narrower compared to DEM resolution (Fig. 5.a and 5.c). Moreover, Oguz et al. (2022), Zheng et al. (2020), Legorreta Paulin et al. (2010) and Michel et al. (2014) have shown how DEM resolution and its accuracy may significantly perturb the local stability at the top and bottom edges, extending or reducing the effective unstable slopes (Fig. 5.b and 5.d). According to Legorreta Paulin et al. (2010) a higher DEM resolution could improve the unstable area description by reducing size over/underestimation but it would increase sensibly the computational cost of the hydrological model (Zhang et al., 2016). These topics will be further discussed in section 4.4.

Since the reference data on historical landslides in the IFFI inventory comes from several sources, the localization of the shallow instability could not be geo-localized with high precision, especially for historical events where sometimes only triggering point locations (not the landslide polygon) are reported (ISPRA, 2018). To carry out the ROC methodology and avoid reference data issues, a buffer zone with different radii around each landslide point was created: 250 m, 500 m, 1000 m

and 2000 m (Fig. 6). This radius represents an attempt to cope with the uncertainties about the real position and extension of the triggered landslide.



455 **Figure 6: ROC methodology scheme to assess the CRHyME model performances in detecting landslide failures that occurred after a rainfall event. (a) unstable areas predicted by CRHyME considering the surrounded 8 cells, (b) unstable area reported in IFFI considering buffer-zones due to geo-localization uncertainties, (c) confusion matrix and parameters TP, FN, TN and FP calculation, (d) evaluation of performance parameters TPR and FPR for the graphical representation of the ROC curves (e).**

460 Knowing the observed and the predicted instabilities (retrieved by IFFI and simulated by CRHyME) referring to a specific geo-hydrological event, the ROC assessment was conducted. The ROC curves were built following the scheme presented in Fig. 6. Through a confusion matrix (Fig. 6.c), the False Positive Rate (1-specificity, FPR) Eq. (19) and the True Positive Rate (sensitivity, TPR) Eq. (20) are calculated (Fig. 6.d), and the point (FPR, TPR) is reported in the ROC graph (Fig. 6.e). The upper left corner of the graph (TPR = 1 and FPR = 0) represents the perfect performance (or perfect classifier), and the diagonal line, represents the random classification or “no skill”. As the point (FPR, TPR, the prediction skill) plotted on the ROC graph is closer to the upper left, the prediction capacity of the CRHyME model is better.

$FPR = \frac{FP}{N} = \frac{FP}{FP + TN}$	(19)
$TPR = \frac{TP}{P} = \frac{TP}{TP + FN}$	(20)

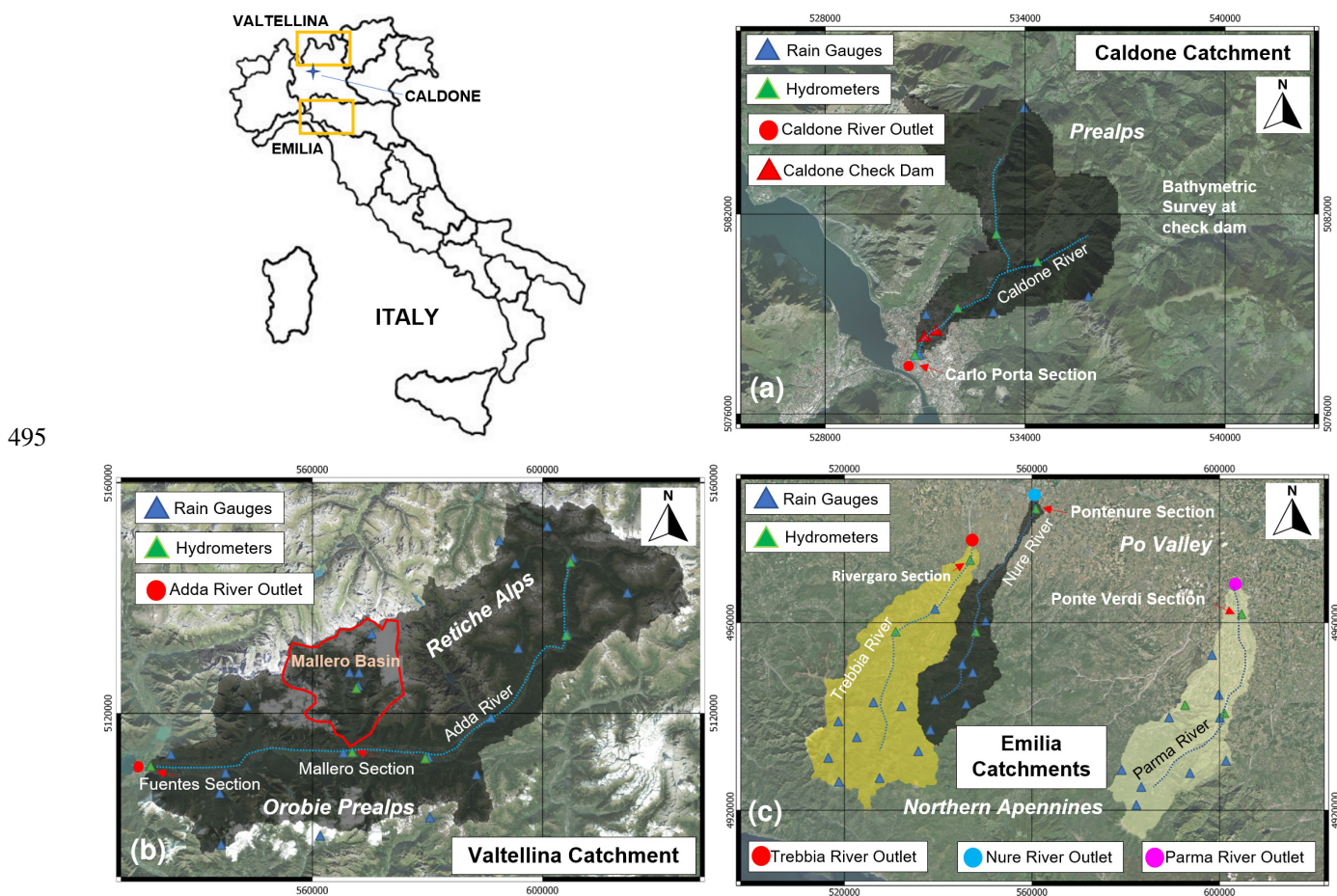
## 2.4 Cases studied

The case studies simulated with CRHyME are located in Northern Italy and presented in Fig. 7. The Caldone basin (Fig. 7.a) represents the on-field laboratory of the University of Politecnico di Milano (Brambilla et al., 2020). The basin is about 27 km<sup>2</sup> situated near the city of Lecco (Lombardy region) across the Prealps and is characterized by intense sediment transport. The catchment is well monitored by 5 rain gauge stations, a hydrometer at the outlet and two sediment check-dams where the sediment yield is constantly monitored with periodic bathymetric surveys. The lithology of the area is constituted by consolidated calcareous rocks with good strength properties but susceptible to rainfall erosivity. Karst is present in the surrounding region but is not relevant in the Caldone catchment (Papini et al., 2017). From a climatic viewpoint, the area has a mean precipitation of 2000 mm yr<sup>-1</sup> (Rete Monitoraggio ARPA Lombardia).

475 The Valtellina catchment (Fig. 7.b) is settled in the northern part of the Lombardy region across the Central Alps and in 1987 experienced a dramatic geo-hydrological episode triggered by intense and prolonged rainfalls (Abbate et al., 2021a). The effects on the territory were severe: shallow landslides, debris flows, and flash floods were recorded causing human injuries and fatalities and extensive damage to infrastructure and buildings (Luino, 2005). The secondary branch of Mallero River also experienced intense sediment transport during the 1987 flood, which affected Sondrio town. Similar events iteratively hit the



area in November 2000 and 2002. The Valtellina valley has E-W topographical development, and its geomorphology is characterized by a strong difference between the opposite slopes. In the southern flank of the valley, the Orobic Prealps are constituted by consolidated metamorphic rocks (Gneiss) while across the Retiche Alps (northern flank), magmatic and sedimentary rocks alternate with metamorphic. The most prevalent type of soil texture is formed by sandy loam and silty loam (Crosta and Frattini, 2003; Longoni et al., 2016). The catchment is characterized by a strong precipitation variability in the range of a minimum of 600 mm yr<sup>-1</sup> in the north-eastern part of the Retiche Alps and a maximum of 3500 mm yr<sup>-1</sup> in the south-western sector of the Orobic Prealps (Rete Monitoraggio ARPA Lombardia). According to this, two different meteorological datasets were examined here to test the ability of CRHyME to deal with different rainfall datasets. The first one considers the meteorological data provided by the Regional Agencies for Environmental Protection (ARPA Lombardia) (Rete Monitoraggio ARPA Lombardia) ground-based weather stations. The second one is MERIDA, the MEteorological Reanalysis Italian Dataset (Bonanno et al., 2019). MERIDA consists of a dynamical downscaling of the new European Centre for Medium-range Weather Forecasts (ECMWF) global reanalysis ERA5 using the Weather Research and Forecasting (WRF) model, which is configured to describe the typical weather conditions of Italy.



**Figure 7: The Caldone (a), the Valtellina (b) and the Emilia (c) catchments studied. Rain gauges, hydrometer stations and river outlets are indicated in (a, b, c). Hydrometric stations considered for assessing the CRHyME performances are located at Carlo Porta section (for Caldone River), Fuentes and Mallero sections (for Adda and Mallerio Rivers) in the Valtellina catchment, and Rivergaro (for Trebbia River), Pontenure (for Nure River) and Ponte Verdi (for Parma River) sections across the Emilia area. Base layer from © Google Maps 2023.**

The Emilia area is situated in the Northern Apennines (Fig. 7.c) and experienced intense geo-hydrological episodes in October 2014 and September 2015 (Ciccarese et al., 2020). Three watersheds were particularly affected: the Trebbia, Nure and Parma catchments. The event of October 2014 hit the Parma catchment while the event of September 2015 hit the Trebbia and Nure catchments. From a geomorphological viewpoint, the Northern Apennines represent a “fold-and-thrust” mountain chain where several landslide instabilities are present due to the post-failure weathering of claystone, sandstone, and limestone rock

fragments. These deposits are in residual strength conditions and can be quite easily mobilised and trigger debris flows during heavy rain episodes (Parenti et al., 2023). The Emilia region is characterized by a rainfall distribution with a south-north gradient where a maximum amount of 2000 mm yr<sup>-1</sup> is recorded in the highest relief of the Apennines (south) while the 700 mm yr<sup>-1</sup> characterizes the floodplain areas of the Po Valley in the northern part (Rete Monitoraggio ARPA Emilia).

During calibration and validation of the CRHyME model, some monitoring points for checking the water discharge and volume were chosen in correspondence with the reference hydrometers located at the catchment outlets (green triangles in Fig. 7.a,b,c). Check dams and hydropower reservoirs were considered for estimating reference sediment yield: a check dam close to the outlet for the Caldone catchment (red triangles in Fig. 7.a), three hydropower reservoirs of Campo Tartano, Valgrosina and Cancano for the Valtellina case study (red triangles in Fig. 10.a) and AdBpo reference data (Autorità di Bacino Distrettuale del Fiume Po, 2022) for the Emilia case study. Regarding shallow landslides and debris flows triggered during the investigated geo-hydrological events, a literature survey has been conducted within the IFFI inventory and scientific literature to find an available inventory of the occurred failures (Fig. 11.a and 13.a).

### 3 Results

In the next paragraphs, the results obtained for the three case studies are presented in this way: description of the simulation settings, reporting of hydrological and sediment transport performances, and showing landslide and debris flow ROC assessment.

#### 3.1 Caldone case study

The Caldone catchment was investigated to verify the numerical conservativity of hydrological and sediment balances calculated by CRHyME, to explore the sensitivity to the variation of spatial resolution of the input data (e.g. DEM) and to calibrate and validate the *slope* → *D*<sub>50</sub> empirical relations. According to Rocha et al. (2020), and Tavares da Costa et al. (2019), a spatially distributed hydrological model is sensitive to input data spatial resolution. The reconstruction of the catchment parameters, such as the flow accumulation and the flow direction, depends on the characteristics of the DEM. As a result, routing methods, which also depend on the flow direction accuracy, may experience differences in results under different cell resolutions. Moreover, increasing the DEM resolution is time-consuming due to the considerable number of cells within the computational domain. To test these aspects in CRHyME, for the Caldone catchment four runs were executed in a short period of 6 months, considering four different DEM resolutions: 90 m, 50 m, 20 m and 5 m. In Table 2 the simulation settings are resumed.

Simulation Settings and Error Analysis	Simulation 1	Simulation 2	Simulation 3	Simulation 4
Spatial Resolution	90 m	50 m	20 m	5 m
Starting Date	01/06/2020	01/06/2020	01/06/2020	01/06/2020
Ending Date	10/10/2020	10/10/2020	10/10/2020	10/10/2020
Initial Soil Moisture	90%	90%	90%	90%
NSE (Volume) [-]	0.765	0.777	0.777	0.656
NSE (Discharge) [-]	0.341	0.650	-1.333	-2.610
RMSE (Discharge) [m <sup>3</sup> s <sup>-1</sup> ]	1.605	0.699	1.804	2.244

**Table 2: Settings adopted for the Caldone River simulations, and hydrological volume and discharge error metrics calculated at Carlo Porta hydrometric station, testing different DEM spatial resolutions.**

As can be appreciated from Table 2, the model's ability into the reproduction of a realistic water discharge tends to degrade progressively using a higher resolution. Looking at NSE scores for the discharge, the best accordance with the reference is



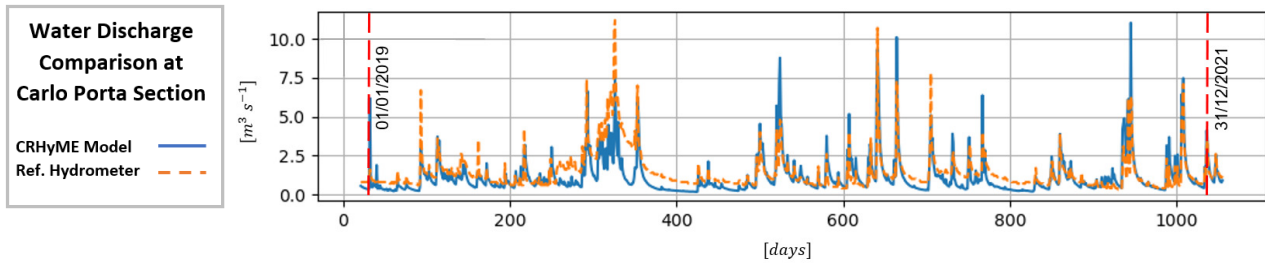
540 reached with a 50 m resolution. RMSE for the discharge is lower for a 50 m simulation. The model is conservative since the NSE for the volume is close to 0.8, verifying that almost all the precipitation volume has arrived at the outlet within the simulated period. The NSE for the volume is a parameter that is rather invariant with respect to the resolution while the NSE for the discharge is spatial scale dependent. The meteorological data series necessary to run the model were gathered from the ARPA Lombardia agency (Rete Monitoraggio ARPA Lombardia) (Fig. 6.a). The hydrometers data and the local stage-discharge relation were retrieved from the Lecco municipality station located at Via Carlo Porta (Fig. 7.a). The rain gauges were spatially interpolated using the IDW technique (Chow et al., 1988) with a temporal resolution of 1 day.

545 The influence of the  $slope \rightarrow D_{50}$  curves parameterization was the second aspect investigated in the Caldone catchment. A long-term simulation has been carried out from 1 January 2019 up to 31 December 2021 (Fig. 8), with a DEM resolution of 50 m and after a “*spin-up*” period of 2 years for raising the model to realistic initial conditions. Considering the limited extension of the watershed, this period has revealed sufficient for assessing the performance of solid discharge. The sediment discharge was computed considering both TL (Transport Limited) and EL (Erosion Limited) options. In Table 3 can be noticed that NSE for water discharge and volume exhibit a high score, about 0.462 and 0.719 respectively. The former states that the reproduction of the hydrological part has been assessed almost correctly by CRHyME. Four  $slope \rightarrow D_{50}$  functions have been tested in the form like  $D_{50} = a_x slope^{b_x}$  (Table 4): set 1, set 2, set 3 and set 4. Results have shown that the choice of  $slope \rightarrow D_{50}$  can sensibly modify the outlet’s sediment yield: the cumulated sediment amount increases with a decrease in the mean diameter. These data were compared with the onsite bathymetric surveys that were carried out four times during the investigated period (Table 5). From the bathymetry measurements, a sediment yield of about  $1000 \text{ m}^3 \text{ yr}^{-1}$  was considered representative of Caldone River. In our sensitivity analysis, this value has matched the reference using set 2:  $2993 \text{ m}^3$  for 1055 days  $\approx 3 \text{ yr}$  correspond to  $\approx 1000 \text{ m}^3 \text{ yr}^{-1}$ . Set n° 2 is rather higher than the functions considered for Valtellina and Emilia

550

555

560 simulations.



565 **Figure 8: Hydrological simulation carried out for sediment transport assessment in the Caldone catchment with 50 m DEM resolution from 01/01/2019 up to 31/12/2021 simulated water discharge (blue line) vs. reference hydrometer at the Carlo Porta section (orange line).**

	NSE (Discharge)	NSE (Volume)	RMSE (Discharge) [ $\text{m}^3 \text{ s}^{-1}$ ]
Error Analysis	0.462	0.719	0.900

**Table 3: NSE and RMSE error metrics of the previous hydrological simulation for the volume and discharge quantities.**

Curve Set	$a_x$ parameter	$b_x$ parameter	Slope $\rightarrow D_{50}$ Equations	Total Sediment Volume [ $\text{m}^3$ ]
Set 1	5604.8	2.38	$D_{50} = 5604.8 \text{ slope}^{2.38}$	2608
Set 2	1786.9	1.79	$D_{50} = 1786.9 \text{ slope}^{1.79}$	2993
Set 3	1453.1	1.61	$D_{50} = 1453.1 \text{ slope}^{1.61}$	5947
Set 4	285.3	0.8	$D_{50} = 285.3 \text{ slope}^{0.80}$	16446

570 **Table 4: Slope  $\rightarrow D_{50}$  functions tested in the Caldone catchment, and the volume of the total sediment simulated by CRHyME at the basin outlet.**

Bathymetry survey	Total Sediment Volume [m <sup>3</sup> ]
20 July 2019 - 20 July 2020	≈ 294 ± 100
20 July 2020 - 13 October 2020	≈ 438 ± 100
13 October 2020 - 15 November 2021	≈ 800 ± 100
Estimated Sediment Yield [m <sup>3</sup> yr <sup>-1</sup> ]	1532 / 2.32 = 660

575 **Table 5: Bathymetric survey and total volume stored at the check dam close to the Caldene catchment outlet. An average sediment yield was calculated around 660 m<sup>3</sup> yr<sup>-1</sup>. Due to possible measurement uncertainties and relatively short time series, a representative sediment yield value of 1000 m<sup>3</sup> yr<sup>-1</sup> was considered in the simulations.**

### 3.2 Valtellina case study

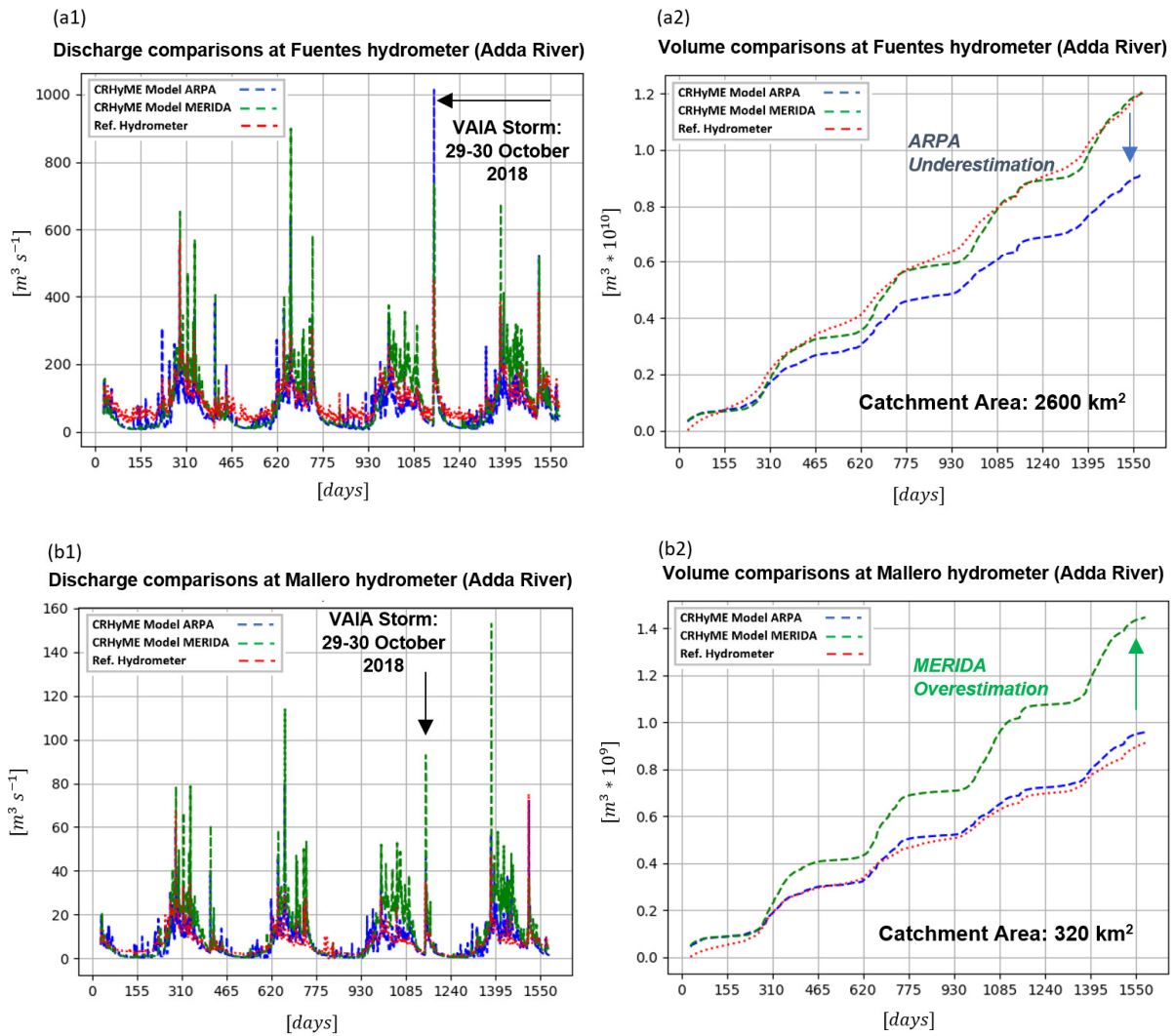
580 The analysis conducted for the Valtellina area has followed the settings reported in Table 6. The CRHyME calibration was carried out for three years between 1 September 2015 and 31 August 2018 after a “*spin-up*” period of 2 years for acquiring realistic initial conditions. Then, a subsequent validation period started on 1 September 2018 up to 31 December 2019. In Fig. 9 the water discharges and the total volumes computed by CRHyME in the two reference sections of Fuentes (basin area = 2600 km<sup>2</sup>) and Mallero (basin area = 320 km<sup>2</sup>) are reported.

Settings for Valtellina catchment	Geo-Hydrological Event Simulated	Starting Date	Ending Date	Rainfall Dataset used
Calibration	-	01/09/2015	31/08/2018	ARPA Lombardia and MERIDA
Validation	October 2018	01/09/2018	31/12/2019	ARPA Lombardia and MERIDA
Validation	July 1987	01/09/1984	31/07/1987	ARPA Lombardia
Validation	November 2000	01/09/1997	30/11/2000	ARPA Lombardia
Validation	November 2002	01/12/2000	31/12/2002	ARPA Lombardia

585 **Table 6: Simulation settings of the Valtellina case study. The calibration and validation of the model have considered more than 4 years of data on a daily basis gathered from ARPA Lombardia (Environmental Agency) weather stations and the MERIDA reanalysis database (Bonanno et al., 2019). These event-based simulations were carried out for significant geo-hydrological events of July 1987, November 2000, November 2002 and October 2018.**

590 Looking at the simulation driven by the ARPA dataset, the total volume transited at the Fuentes section (blue line, Fig. 9.a) is underestimated if compared to the local hydrometric reference (line red), while at the Mallero section (blue line, Fig. 9.b) simulated and recorded volumes are in agreement. NSE scores for volumes also highlight this fact since Mallero’s NSE is ~1 while Fuentes’s NSE is about 0.783 (Table 7). Transited volume is the integral of water discharge that has been better reproduced for the Mallero section (agreement among the blue and red line in Fig. 9.b and NSE = 0.325 in Table 7) rather than Fuentes’s section (disagreement among the blue and red line with underestimation of the mean flow during winter periods in Fig. 9.b and NSE = 0.199 in Table 7) by the model.

600 Opposite results were obtained considering MERIDA’s dataset. There, the Fuentes section has performed well both in discharge and volume computation rather than the Mallero section. The volume NSE at Fuentes is now closer to the perfect agreement while at Mallero station the transited volume is sensibly overestimated. In both cases, NSE scores for discharges are badly represented with values below the ‘0’ threshold. This fact is also well depicted in Fig. 9.a and 9.b where discharge spikes simulated from the ARPA dataset (blue line) are lower compared to the green ones simulated from the MERIDA dataset. The CRHyME model performed numerically conservatively in both cases without code instabilities so that these outcomes are supposed to be perturbed by the different reconstructions of rainfall fields. From these results can be noticed how the influence of rainfall data is determinant in the hydrological assessment. Looking at RMSE scores in Table 7, the simulation with the ARPA dataset was better performed giving lower values of the index, around 4.7 m<sup>3</sup> s<sup>-1</sup> and 45.4 m<sup>3</sup> s<sup>-1</sup> for the Mallero and Fuentes sections respectively. This means that discharge uncertainties propagate proportionally increasing the catchment extension and CRHyME’s performances are sensibly higher for small catchments.



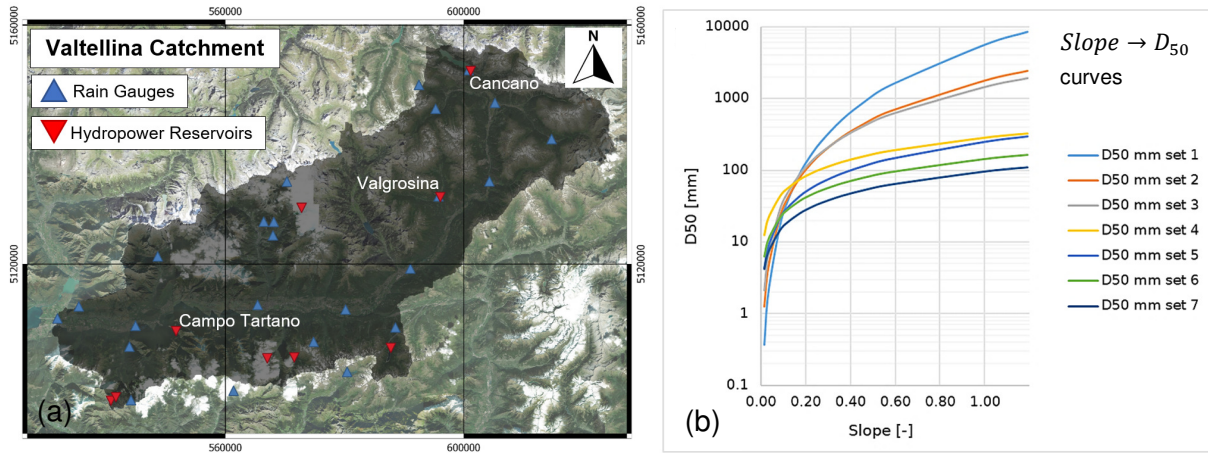
610 **Figure 9: CRHyME model simulation results of water discharges (a1 and b1) and volume (a2 and b2) at the Fuentes (a), and the Mallero (b) hydrometers for the period 2015-2019 and using ARPA weather stations and MERIDA dataset. The geo-hydrological event that occurred in late October 2018 (The Vaia Storm (Davolio et al., 2020)) has been recognized by CRHyME as one of the most intense, especially at the Fuentes section.**

Error Analysis of Hydrological Variables	NSE [-]	RMSE [ $\text{m}^3 \text{s}^{-1}$ ]	NSE_MERIDA [-]	RMSE_MERIDA [ $\text{m}^3 \text{s}^{-1}$ ]
Discharge Fuentes (Adda River)	0.199	45.370	-0.603	64.172
Volume Fuentes (Adda River)	0.783	-	0.993	-
Discharge Mallero (Adda River)	0.325	4.695	-2.369	10.494
Volume Mallero (Adda River)	0.988	-	-0.145	-

615 **Table 7: NSE and RMSE error metrics of the previous hydrological simulation for the volume and discharge quantities. As can be appreciated, the volume performances are better than the discharge: the Valtellina basin is strongly regulated by hydropower plants and dams that operate a consistent lamination of the peak discharge during major rainfall events; the kinematic routing may be not sufficiently accurate for flood propagation across the valley floodplain since dynamic lamination may occur. As a result, green and blue spikes overestimate the peak discharge compared to the reference.**

620 Sediment transport results were checked in correspondence with three hydropower reservoirs of Campo Tartano, Valgrosina and Cancano (Fig. 10.a) considering ARPA dataset simulations. For each reservoir, a literature survey has been conducted to estimate the yearly mean sediment accumulation rate (Ballio et al., 2010; Milanesi et al., 2015; ITCOLD, 2016). The sensitivity parameter for sediment yield is represented by the  $slope \rightarrow D_{50}$  curve that was adjusted during the calibration period (Fig. 10.b and 10.c). Among others, the set n°6 was retained sufficiently representative of the Valtellina area. In Table 9 the results obtained from CRHyME simulations show that the sediment yields evaluated yearly have matched the reference data for the

625 three reservoirs investigated. For the Campo Tartano dam, the difference between the simulated and the reference is around -11.7%, for the Valgrosina dam is about +2.15% while for the Cancano reservoir is around -11.9%.



(c) Curve Set	$a_x$ parameter	$b_x$ parameter	Slope $\rightarrow D_{50}$ Equations $D_{50} = a_x slope^{b_x}$	Literature Reference and Curve Calibration
Set 1	5604.8	2.38	$D_{50} = 5604.8 slope^{2.38}$	From Berg (1995), $b_x = 2.38$
Set 2 (Caldone)	1786.9	1.79	$D_{50} = 1786.9 slope^{1.79}$	Decreasing $a_x$ and $b_x$
Set 3	1453.1	1.61	$D_{50} = 1453.1 slope^{1.61}$	Decreasing $a_x$ and $b_x$
Set 4	285.3	0.8	$D_{50} = 285.3 slope^{0.80}$	Decreasing $a_x$
Set 5	246.7	0.8	$D_{50} = 246.7 slope^{0.80}$	Decreasing $a_x$
Set 6 (Valtellina-Emilia)	142.6	0.8	$D_{50} = 142.6 slope^{0.80}$	From Nino (2002), $b_x = 0.8$
Set 7	95.1	0.8	$D_{50} = 95.1 slope^{0.80}$	Decreasing $a_x$

630 **Figure 10:** (a) Valtellina case study area where hydropower reservoirs of Campo Tartano, Valgrosina and Cancano are indicated; (b) and (c) Slope  $\rightarrow D_{50}$  relations tested and implemented in CRHyME based on the theory of Berg (1995) and Nino (2002) and considering on-site surveys. Base layer from © Google Maps 2023.

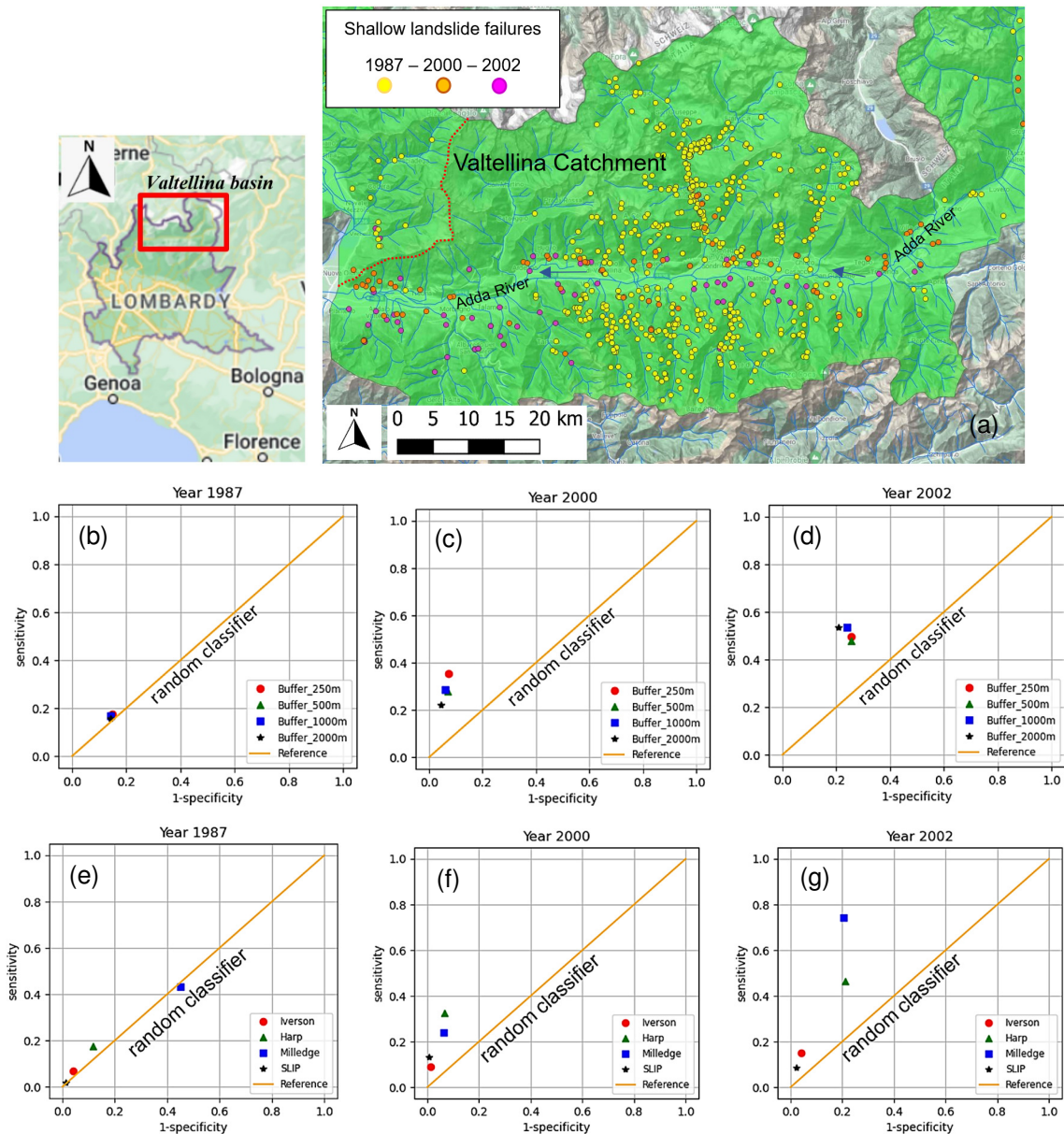
Sediment Yield Error Analysis	Campo Tartano Dam	Valgrosina Dam	Cancano Dam
Literature Reference	38'037 m <sup>3</sup> yr <sup>-1</sup>	33'600 m <sup>3</sup> yr <sup>-1</sup>	21'450 m <sup>3</sup> yr <sup>-1</sup>
Simulated 2015-2019	33'604 m <sup>3</sup> yr <sup>-1</sup>	34'324 m <sup>3</sup> yr <sup>-1</sup>	18'893 m <sup>3</sup> yr <sup>-1</sup>
% difference	-11.7 %	+2.15 %	-11.9 %

635 **Table 9:** Sediment yield estimations for the three dams of Campo Tartano, Valgrosina and Cancano where can be noticed the correct estimation compared to the ITCOLD reference (ITCOLD, 2009, 2016).

The capacity of CRHyME to predict the localization of shallow landslides triggered during the 1987, 2000 and 2002 events was investigated through the ROC scores. Figures 11.b, 11.c and 11.d describe the ROC assessment for the shallow landslides that occurred across the Valtellina valley during the July 1987, November 2000 and November 2002 events. The four shallow landslide instability models included in CRHyME (Iverson, Harp, Milledge, and SLIP) were compared, ranking the Harp model as the most accurate one (Fig. 11e, 11.f and 11.g) and with stable performances. A realistic combination of friction angle values was considered among the broader ranges available in the literature (Abbate and Mancusi, 2021a): 40° for gravels, 35° for sand, 33° for silt and 30° for clay similar to those proposed by Crosta and Frattini (2003). In analogy with root cohesion, the friction angle was spatially distributed by considering the soil composition (%coarse, %sand, %silt, %clay) within the superficial layers (Hengl et al., 2017). Using the Harp model for the three events, the ROC curves have assessed CRHyME's performance above the "random classifier" threshold line. The sensitivity (True Positive Rate) of the model is comprised between 0.2 and 0.6 while the 1-specificity (False Positive Rate) is around 0.2. The distorted distribution of the shallow



landslide inventory related to 1987 may have influenced the performance predictions, lowering the ROC assessment compared to the events that happened in 2000 and 2002. The buffer's choice can influence the redistribution among TP and FP: the performance is slightly lower when large buffers are considered, especially for 1000 m and 2000 m radii, while tends to increase with the radius of 250 m and 500 m close to the actual extension of shallow landslide recorded.



655 **Figure 11:** (a) Triggered shallow landslides during the events of July 1987 (yellow points), November 2000 (orange points) and  
 660 November 2002 (fuchsia points) reported by the IFFI inventory across the Valtellina area; (b, c, d) Representation of the ROC curves  
 for 1987, 2000 and 2002 events considering the Harp model with different landslide position's buffers of 250 m, 500 m, 1000 m and  
 2000 m; (d, e, f) Representation of the ROC curves for 1987, 2000 and 2002 events considering a buffer of 250 m and comparing the  
 four stability models. The Milledge model behaved best for the 2002 event but was the worst for 1987, while the Harps model showed  
 the most stable performances. Base layer from © Google Maps 2023.

### 3.3 Emilia case study

For the Emilia case study, CRHyME simulations were carried out considering 5 years from 01/09/2011 up to 01/09/2016 where the investigated geo-hydrological events of 13/10/2014 and 14/09/2015 have been recorded in the area (Table 10). To raise the model to a realistic initial condition, a spin-up period of 900 days comprised between 01/09/2011 and 28/02/2014 has been carried out. ARPA Emilia meteorological dataset (Rete Monitoraggio ARPA Emilia) was considered for rainfall and temperature variables.

Settings for Emilia's catchments	Geo-Hydrological Event Simulated	Starting/Ending Date for the Spin-Up Period		Starting/Ending Date for Validation Period	Rainfall and Temperature Dataset
River Trebbia	September 2015	01/09/2011	28/02/2014	01/09/2016	ARPA Emilia
River Nure	September 2015	01/09/2011	28/02/2014	01/09/2016	ARPA Emilia
River Parma	October 2014	01/09/2011	28/02/2014	01/09/2015	ARPA Emilia

**Table 10: Simulation settings of the Emilia case study considering the ARPA Emilia (Environmental Agency) rainfall and temperature data.**

670 The hydrology of Trebbia, Nure and Parma rivers has shown scores similar to the Valtellina area (Fig. 12). Looking at NSE in Table 11.a, we can appreciate that higher scores are assessed for the water volume of Nure (0.978), Parma (0.820) and Trebbia (0.773) rivers. For water discharges, NSE scores are better for Trebbia (0.272) and Parma rivers (0.452) while for Nure River are lower (0.102), also confirmed by the RMSE index (Table 11.a).

675 Looking at the solid transport quantification in Table 11.b, the AdBPo (Autorità di Bacino del fiume Po) reports were taken into consideration as reference data for the comparisons (Autorità di Bacino Distrettuale del Fiume Po, 2022). Keeping the same calibration of the  $slope \rightarrow D_{50}$  curve (set n°6) that was adopted for the Valtellina (no granulometry data were found in the examined catchments), the results obtained after the simulations have shown fairly good accordance with the reference. In the three cases, the order of magnitude of the sediment yield delivered each year at the outlet is similar to AdBPo data especially for Trebbia (+12.6%) and Parma (-24.7%) basins while for Nure we have a slightly larger difference (-35.7%). Perhaps, finer  
680 granulometry should have been taken into account for simulating the Parma and Nure rivers, decreasing the  $D_{50}$ . This suggests how the sediment transport dynamics are sensitive to the  $slope \rightarrow D_{50}$  parameterization that strongly depends on the geological and lithological characteristics of the catchment.

(a)

Error Analysis of Hydrological Variables	NSE [-]	RMSE [ $m^3 s^{-1}$ ]
Discharge Rivergaro (Trebbia River)	0.272	27.915
Volume Rivergaro (Trebbia River)	0.773	-
Discharge Pontenure (Nure River)	0.102	33.468
Volume Pontenure (Nure River)	0.978	-
Discharge Ponte Verdi (Parma River)	0.452	14.898
Discharge Ponte Verdi (Parma River)	0.820	-

(b)

Sediment Yield Error Analysis	Trebbia River	Nure River	Parma River
AdbPo Reference	247.2 $10^3 m^3 yr^{-1}$	69.4 $10^3 m^3 yr^{-1}$	101.1 $10^3 m^3 yr^{-1}$
Simulated 2011-2015	278.3 $10^3 m^3 yr^{-1}$	44.6 $10^3 m^3 yr^{-1}$	76.1 $10^3 m^3 yr^{-1}$
% difference	+12.6 %	-35.7 %	-24.7 %

685 **Table 11: CRHyME model error analysis for hydrological discharge and volume (a), sediment yield (b) for the Emilia's catchments at Rivergaro (Trebbia River), Pontenure (Nure River), Ponte Verdi (Parma River) hydrometers.**

The performance of CRHyME in detecting the triggered debris flow during the events of October 2014 and September 2015 (Fig. 13) was assessed again through ROC. A new calibration on the friction angle was carried out since the value provided for the Valtellina was too conservative for stability. This fact could be explained by the Apennines's lithologies which are  
690 characterized by higher percentages of clay compared to the Central Alps, reducing the soil friction resistance (Raj, 1981; Hengl et al., 2017). The highest ROC scores were obtained by slightly decreasing (-20%) the slope friction angles and reducing the soil cohesion to the minimum, supposed to be representative of incoherent deposits. In most cases the model has

outperformed the random classifier, showing a sensitivity (TPR) comprised between 0.1- 0.4 and a higher value of specificity (1-FPR) depending on the chosen buffer extension around the triggering point. In our simulations, debris flow failure has been effectively detected across a small valley impluvium, confirming the onsite observations carried out by Ciccarese et al. (2020, 2021): the highest scores were obtained for the Nure catchment, intermediate rank for Parma basin and poor description of instabilities for Trebbia watershed.

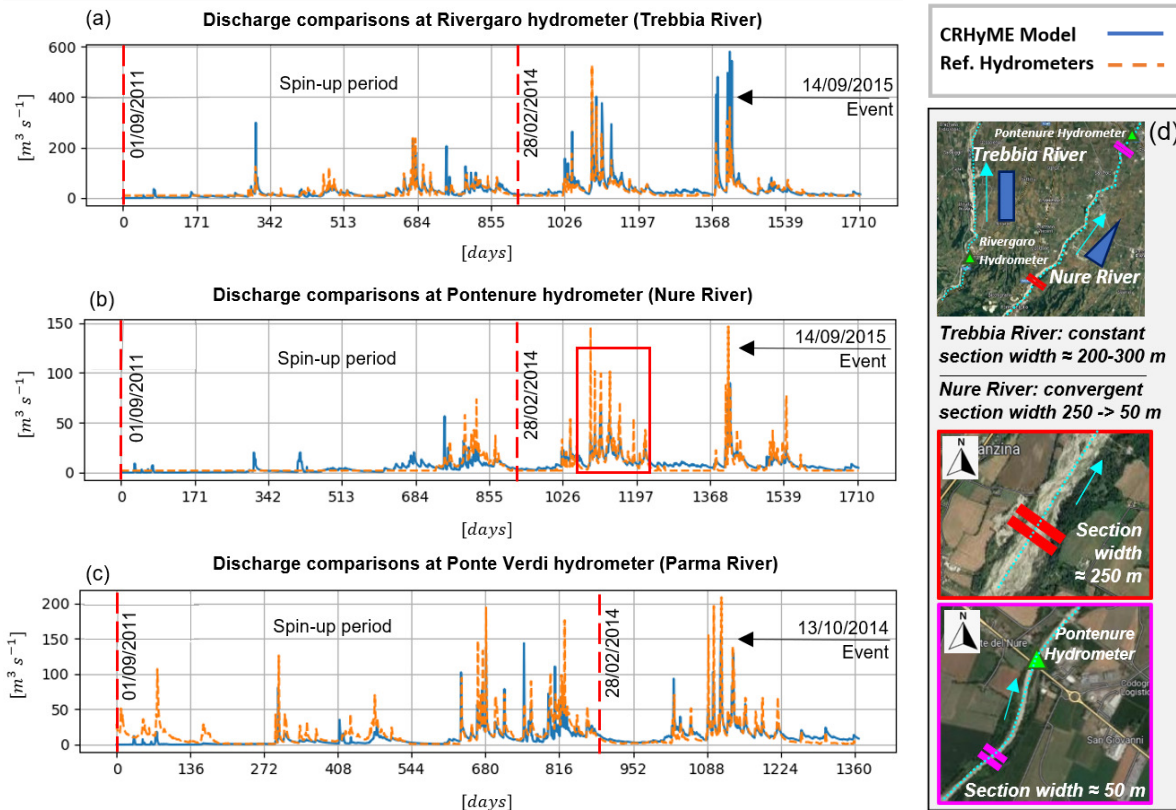
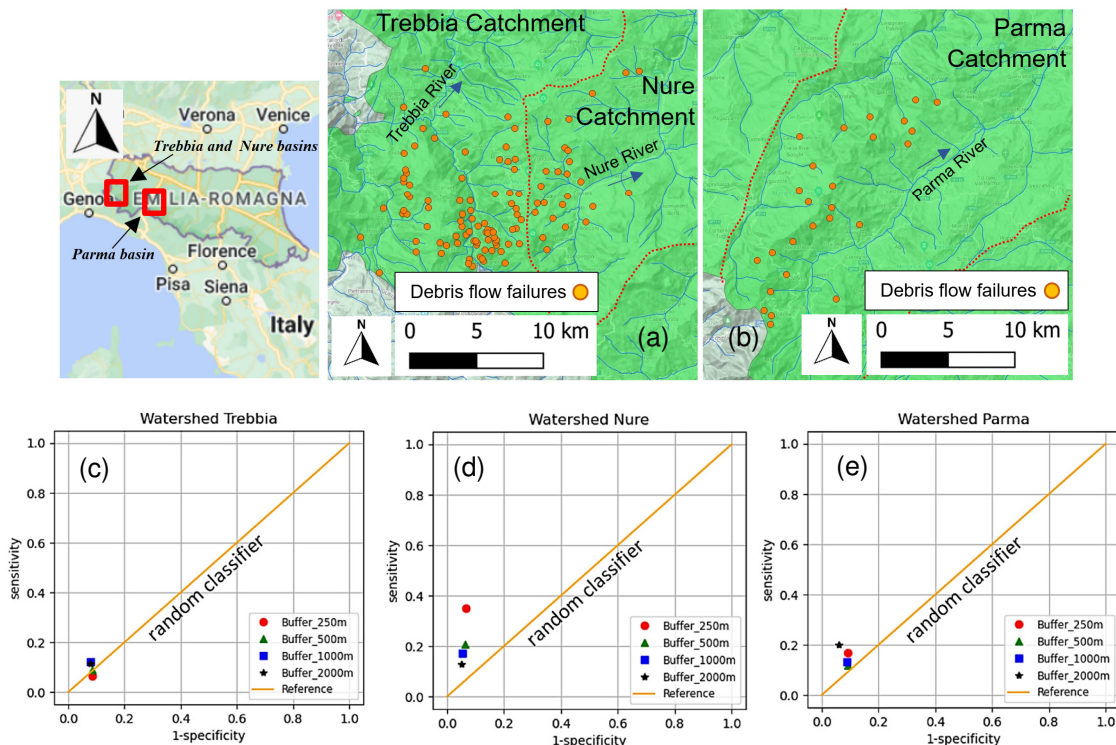


Figure 12: CRHyME water discharges comparisons for the Emilia's catchments at (a) Rivergaro (Trebbia River), (b) Pontenure (Nure River), (c) Ponte Verdi (Parma River) hydrometers for the period 2011-2016. The first 900 days of each simulation are considered for model "spin-up" to a realistic initial condition. In the red box (b) is highlighted the peak discharge underestimation for the Nure River due to section variation along floodplain (d). Base layer from © Google Maps 2023.



705 **Figure 13: (a) Debris flows triggered in Trebbia and Nure basins during the event of September 2015 and (b) debris flows triggered in Parma basin during the event of October 2014. Orange points are the mass wasting starting points reported by Ciccarese et al. (2020, 2021) after the event. Representation of ROC curves for Trebbia (c), Nure (d) and Parma (e) watersheds for the events of September 2015 and October 2014. Base layer from © Google Maps 2023.**

## 4 Discussion

### 710 4.1 CRHyME sensitivity analysis: spatial resolution and sediment diameters

The CRHyME model sensitivity in reproducing hydrological cycle has been tested considering four different spatial resolutions within the Caldane catchment (27 km<sup>2</sup>): 90 m, 50 m, 20 m and 5 m. CRHyME results were obtained with sufficient accuracy and faster computation for cell resolution > 10 m. The computational time was observed to be proportional to the number of domain cells: the 90 m, 50 m and 20 m simulations were concluded in one to two minutes while for the 5 m simulation, the  
715 time was raised to 5 min. However, increasing spatial resolution doesn't mean always increasing the accuracy (Rocha et al., 2020; Zhang et al., 2016) and with CRHyME the best performance was acquired for DEM resolutions of 50 m and 20 m and not for 5 m. The variation of the DEM resolution can change sensibly the flow direction of the rivers ("*ldd.map*") and the basin drainage density, affecting discharge computation. According to the literature (López Vicente et al., 2014; Erskine et al., 2006), the routed runoff could be perturbed by "*numerical diffusion*", a known problem of the spatially distributed models that is  
720 predominant with fine spatial resolution, which depends on the algorithm applied for flow direction computation (Barnes, 2017, 2016). To preserve CRHyME's solution accuracy and to maintain affordable computational times, we suggest applying the HydroSHEDS DEM model at 90 m resolution for quite large basins > 500 km<sup>2</sup> while higher resolutions are advisable for smaller basins.

Within the Caldane catchment, the dependence of the sediment transport processes on the soil granulometry was tested. The  
725 distribution of  $D_{50}$  that increase as a function of the slope is a reasonable representation of the geomorphological processes encountered in mountain catchments (Brambilla et al., 2020; Ivanov et al., 2020a; Ballio et al., 2010). According to Nino (2002), among *slope* and  $D_{50}$  exist a slight correlation, but non-linearities are caused by sediment processes occurring within river granulometry (sorting and armouring). Recently, data-driven approaches were explored in the USA to define a map of the  $D_{50}$  along the river stream (Abeshu et al., 2021). To evaluate the map, these authors have chosen a series of geo-  
730 morphological predictors of  $D_{50}$  (elevation, slope, curvature etc.), verifying results with the available databases at country-based they have retrieved the USA  $D_{50}$  map. Not surprisingly, one of the most important predictors is the basin slope which has the highest correlation coefficient with a  $D_{50}$  but other geomorphological factors (river path length and elevation) have a similar correlation with  $D_{50}$ . It seems clear that a unique formulation of the  $D_{50}$  as a function of morphological and hydrodynamical parameters cannot be assessed straightforwardly. Since  $D_{50}$  is required for incipient motion of bed-load  
735 sediment transport (Chow et al., 1988), and bearing in mind its complexity in spatial evaluation, *slope* →  $D_{50}$  curves implemented in CRHyME represent a crude but efficacious simplification. Moreover, *slope* →  $D_{50}$  have the advantage of being easily calibrated if on-site data are available.

### 4.2 CRHyME's hydrological performances

For the Valtellina case study, CRHyME hydrological performances for the water discharge (NSE ~ 0.2-0.3) were not  
740 comparable to the ones obtained for Caldane River (NSE ~ 0.46). A possible explanation resides within the characteristics of the Valtellina catchment, which is bigger (2600 km<sup>2</sup>) than the Caldane basin (27 km<sup>2</sup>). Bigger computation domains mean increased landscape heterogeneity which implies higher uncertainties in the reproduction of infiltration-runoff-groundwater processes (Morbidelli et al., 2018; Mishra et al., 2003; Chow et al., 1988). Comparing volume and discharge scores for the Valtellina area driven by the ARPA dataset, a general tendency to overestimate the peak discharge during rainfall seasons  
745 (spring, summer, and autumn) can be noticed while an underestimation of the discharges during winter is detected (Fig. 9.a).



This effect is more significant at the Fuentes hydrometer but is less evident at the Mallero station. After analysing these results, three main error components were disentangled into 1) infiltration, 2) losses, and 3) routing parameterizations. They represent key processes that should be paid attention to during the calibration phase (Morbidelli et al., 2018) since if they are wrong-conditioned may also cause numerical instabilities, losing the water-balance conservativity of the code. As reported by Abbate and Mancusi (2021a), infiltration models strongly regulate runoff generation. Their parameterization depends on land surface coverage and terrain composition which are sometimes affected by high uncertainties: onsite measurements are generally not available and coverage layers have low resolution. For CRHyME, this fact may imply cascade effects on landslide processes causing underestimation of the landslides triggered due to the reduced subsurface pore pressure caused by wrong soil moisture balance predictions. Water recirculation inside the groundwater reservoir affects water balance in the long term. In this regard, Alps and Apennines have complex hydrogeology (ISPRA, 2018) which affects the groundwater dynamics that a simple Dupuit model may oversimplify. Unfortunately, the unavailability of local piezometric reference data for calibration has not permitted us to assess model performance for this part. To cope with these uncertainties, several sensitivity and calibration tests (not reported here) were conducted during model construction, varying groundwater parameterization to achieve the best performances. Another source of error is embedded in the runoff-routing algorithm. The kinematic algorithm adopted in CRHyME is sufficiently representative of the small lateral catchment rainfall-runoff processes (as for Caldono or Mallero rivers) but maybe not be suitable for interpreting floodplain flood evolution where dynamic processes are prevalent (Chow et al., 1988). Moreover, across the Valtellina catchment, river discharges are regulated by several hydropower plants (ITCOLD, 2009, 2016). Dams can smooth and shift floods peaks and perturb seasonal water discharges recorded at the outlet's hydrometer lowering the CRHyME performances: in the current version of the model lakes and dams are not considered explicitly. Among others, this fact could explain the best water discharge score (NSE = 0.325) of the Mallero sub-catchment (less regulated, only 2 dams) with respect to the Fuentes outlet (NSE = 0.199) for the whole Valtellina catchment (Fig. 7).

The hydrological performances of Emilia catchments have scores similar to the Mallero River. The water discharge assessment for the tested period shows the best agreement for Trebbia (NSE ~ 0.27) and Parma (NSE ~ 0.45). These basins are less regulated by hydropower reservoirs compared to the Valtellina, and, since they are smaller (about 1/3 of the extension), the kinematic approach for runoff routing is more representative. Nevertheless, the lower scores for the Nure River were caused by an underestimation of the peak discharges (Fig. 12). Several simulations conducted in the Nure basin have shown a systematic bias within the reference data. The latter could be explained by the location of the reference hydrometer, which is settled far away across the flood plain where the river is constricted to flow within a narrow section (~10 m) compared to the upper catchment (~250 m) (Fig. 12.d). Looking at Fig. 7, the Pontenure (Nure River) hydrometer is located across the flood plain ~20 km downstream of the catchment for the Rivergaro (Trebbia River, ~1 km) and the Ponte Verdi (Parma River, ~10 km) stations. Similarly to the Valtellina case, where a flood lamination is likely to occur, to describe river behaviour across a floodplain, the dynamic approach should be preferred instead of kinematic routing. In fact, including section geometry as input could increase the simulation accuracy, improving the model's performances in hydrographs and discharge reconstruction (Lee and Pin Chun, 2012; Chow et al., 1988).

#### 780 4.3 CRHyME's geo-hydrological performances

Geo-hydrological processes have been consistently reproduced by CRHyME. The sediment yields in the Valtellina catchment have matched the available reference data of Campo Tartano, Valgrosina and Cancano dams after a calibration of the  $slope \rightarrow D_{50}$  to distribute grain size parameters across the catchment. The good reproduction of the annual sediment yield (~10% underestimation for Valtellina) has been confirmed also for the Emilia case study where the order of magnitude was comparable to the AdBPo reference (~±20% depending on the basin).

Since the  $D_{50}$  perturbs the threshold that activates the sediment transport (Vetsch et al., 2018), it has revealed the critical parameter to be assessed in the CRHyME model. For the Valtellina and Emilia areas, the optimal  $slope \rightarrow D_{50}$  curve (set  $n^\circ$

6, Fig. 10) was different compared to the one adopted for the Caldone catchment (set n° 2, Fig. 10). From a geological viewpoint, the Caldone catchment is located in the Prealps where calcareous rocks are prevalent while metamorphic bedrock is more diffused across the Valtellina catchment (ISPRA, 2018). Depending on the state of fracture, metamorphic could be less strength than calcareous and more prone to be fragmented into small diameters (D'Agostino and Marchi, 2001). Moreover, the maturity of the watershed influences the granulometry distribution across the landscape (Pérez-Peña et al., 2009; Strahler, 1952). Large basins such as the Valtellina and Emilia catchments are more “mature” than the small Caldone catchment, therefore, a finer granulometry at the outlet is expected. This fact seems to justify why a lower *slope* →  $D_{50}$  curve was optimal for these catchments while a higher one was more suitable for the Caldone basin.

The CRHyME model has identified the localization and the timing of landslide failures during the extreme events that have affected the studied catchment. Looking at ROC scores for the Valtellina area, 1987, 2000 and 2002 events were reproduced consistently. The best scores were acquired for 2000 and 2002 events where a good quality inventory was available for the investigated area. For 1987, the incompleteness of the available inventory (yellow points in Fig. 11.b) affected the model's final score. However, independently from the specific case, the ROC methodology has highlighted how much the choice of the stability parameters (friction angle and cohesion) has a critical influence on the final results. This fact has been confirmed also by the sensitivity analysis carried out for debris flow episodes in the Emilia case study during the events of 2014 and 2015. Here, to reach the best ROC scores against the random classifier, the friction angles calibrated for Valtellina have been slightly reduced by about 20% confirming the dependence of this parameter on soil texture and lithology.

#### 4.4 Model limitations

The sensibility of the CRHyME model to precipitation mapping, initial hydrological conditions, DEM scale-dependency and geo-hydrological cycle parametrization have affected the result accuracy and performance. Since they represent possible limitations on the applicability of the code a brief discussion is here developed suggesting possible solutions.

Correctly assessing the precipitation distribution is mandatory to define a realistic representation of the external forcing that triggers geo-hydrological failures (Abbate et al., 2021b). Especially across mountain regions, the higher local variability of meteorology and the absence of a dense rain gauge network can complicate the reconstruction of a representative rainfall field. This aspect was investigated for the Valtellina case study, where simulations derived by MERIDA (Bonanno et al., 2019) and ARPA (Rete Monitoraggio ARPA Lombardia) datasets were compared. Using MERIDA, we would expect a better performance from CRHyME but this didn't happen in all situations. Looking at water volumes transited across the Fuentes hydrometer during the period 2015-2019, the MERIDA dataset has performed better than ARPA stations. On the other hand, looking at the Mallero hydrometer, the MERIDA dataset has scored worse than ARPA stations. What is the possible explanation for this contradictory fact? MERIDA gives a rainfall map that has a spatial resolution of ~ 4 km while the ARPA station data are interpolated geometrically using the Inverse Distance Weight (IDW) techniques (Daly et al., 1997; Chow et al., 1988). A trade-off exists between the ARPA's rain gauge network density and the spatial resolution of MERIDA. In large catchments, MERIDA is more representative since it can cover ungauged areas while in small catchments, lower spatial resolution may be insufficient for describing local rainfall variability. This is why MERIDA has performed worse than IDW in the Mallero catchment where several ground-based weather stations are uniformly distributed across a limited area of 320 km<sup>2</sup> (Fig. 7) Moreover, reanalysis datasets could sometimes smooth the rainfall peaks leading to a wrong interpretation of the net rainfall that occurred over a limited area (Abbate et al., 2021b; Bonanno et al., 2019; Ly et al., 2013). This is another key issue that generally influences the ability of the slope stability model implemented in CRHyME to detect landslides triggered by rainfalls. In this regard, a better integration within rainfall sources coming from the ground-based station, reanalysis models, radar maps and satellite data is advisable to reduce possible rainfall uncertainties (Abbate et al., 2021b).

The choice of a realistic initial catchment's moisturizing is another common issue in every deterministic spatially-distributed hydrological model (Uber et al., 2018; Trambly et al., 2010; Chow et al., 1988). Historical measures about the superficial soil

830 moisture, groundwater piezometry and superficial runoff are difficult to gather, especially across small mountain basins where  
monitoring systems are not provided (Schoener and Stone, 2020; Chiarelli et al., 2023). Moreover, soil moisture is a quantity  
that can vary abruptly across different terrain types, so it is not common to implement a network that permits the acquisition  
of distributed information across a catchment (Lazzari et al., 2018). In CRHyME, to overcome these difficulties, a “*spin-up*  
835 *period*” was introduced within each simulation. This period represents the minimum time required by the model for reaching  
an automatic adjustment of the initial condition that depending on the extension of the basin, can be comprised within a few  
months up to some years. The spin-up simulation permits a re-distribution of the water across the cells of the domain  
(horizontally) and among each layer of the model (vertically), reducing the “*time lag*” between rapid (runoff) and slow  
(groundwater) catchment dynamics. This “*time lag*” effect was rather evident for the Emilia case study, where a realistic regime  
condition was reached only after three years, rather slower than for the Adda basin (two years). This fact could be explained  
840 by the different soil compositions and lithology that influence hydrogeological parameters. In the Apennines, the presence of  
clay decreases the speed of soil recharge (lower permeability) slowing down the groundwater recharge (Ronchetti et al., 2009;  
Ciccarese et al., 2020; Parenti et al., 2023) compared to the Alps, where coarser terrain granulometry increases soil  
permeabilities. From a practical viewpoint, running the model up to realistic hydrological conditions is time-consuming. In  
CRHyME, PCRaster libraries are already parallelized and can reduce sensibly the computational cost of this operation.  
845 Moreover, CRHyME can set a “*restart*” condition, saving the hydrological storage outputs  
 $h_{snow}(t)$ ,  $h_{soilwater}(t)$ ,  $h_{groundwater}(t)$  and  $h_{runoff}(t)$  computed during the spin-up period which could be reused for  
subsequent running.

For evaluating shallow landslide and debris flow triggering the simple theory of the infinite slope stability model has been  
implemented. According to Harp et al. (2006), Iverson (2000), Takahashi (2009), Oguz et al. (2022) and Milledge et al. (2014),  
850 this methodology is rather affordable for cell-based landslide susceptibility models and mapping thanks to its fast coding.  
However, different results in slope stability assessment are expected to vary depending on DEM resolution. This fact was not  
directly experienced with CRHyME since in the Caldane catchment, where spatial scale dependence was tested, the IFFI  
inventory was not available for landslide investigation. Legorreta Paulin et al. (2010) and Zheng et al. (2020) have pointed out  
how the simple infinite slope theory needs to be applied carefully. First of all the inaccuracy of the infinite slope method is  
855 related to the fact that each pixel is considered independent from the other settled at the boundary (Iverson, 2000). For very  
large DEM pixel size, namely  $> 100$  m this may be an acceptable hypothesis since  $100 \times 100$  m<sup>2</sup> represents the typical order of  
magnitude of a rainfall-induced shallow landslide or a debris spatial extension ( $\sim 10^4$  m<sup>2</sup>). For pixels  $\leq 100$  m this is not  
properly correct since the cell size is lower than the typical dimension: surrounding areas may participate in the landslide  
collapse due to boundary effects, especially at the top and bottom edges (Fig. 5), caused by strength redistribution (Zheng et  
860 al., 2020; Milledge et al., 2014). In CRHyME, having a spatial resolution of about  $\sim 70$  m, we have preferred to include the  
surrounding 8 pixels close to the unstable areas, following a rather conservative choice justified by the physical interpretation  
of landslide kinematics. On the other hand, varying DEM’s resolution causes a modification of the local slope gradients which  
are the main drivers of failures: lower resolutions can operate an unphysical smoothing of the surface reducing cliff and scarp  
that may trigger small landslides. As a result, the best DEM resolution available may lead to the most accurate results but this  
865 choice is generally adopted for static landslide susceptibility mapping while may not be suitable for dynamic routines  
(Legorreta Paulin et al., 2010; Zhang et al., 2016). In CRHyME hydrological balance is computed at each time step and then  
the slope stability calculation is evaluated recursively: increasing the spatial resolution the computational times rise fast so that  
a trade-off between model performance and result accuracy should be acquired. Bearing in mind this, improvements on  
landslide hazards are planned in the future version of the code, making the slope stability routine less scale-dependent and less  
870 conservative.

According to Gariano and Guzzetti (2016), reconstructing the whole geo-hydrological cycle that drives the erosion and mass  
wasting processes through numerical models is a challenge. In this regard, CRHyME is not an exception: the EPM is considered

for erosion; empirical power-law relationships are implemented for sediment routing; only landslide and debris flow triggering conditions are evaluated by stability models, not including runout evolutions. This subdivision was adopted firstly to simplify the phenomena interactions and secondly to guarantee the fast functioning and stability of the CRHyME code. Following this sequential scheme, geo-hydrological processes are computed after the hydrological assessment, but possible feedbacks are not explicitly taken into account. On a long-term timescale, geo-hydrological processes contribute to a landscape modification, e.g. DEM's height changes. The former is not included by CRHyME since the code has been built with a different purpose with respect to landscape-evolutions models (Campforts et al., 2020; Bovy et al., 2020; Salles, 2019). However, all geo-hydrological hazards also play an important role in the short-term period, modifying temporarily or permanently the local soil depth (Sklar et al., 2017): landslide and debris flow runout can redistribute the local terrain changing the soil depth (asportation at the crown and accumulation at the toe) and modifying the DEM height. Therefore, finding a “closure” for the superficial geo-hydrological balance is a non-trivial task from a theoretical and numerical viewpoint (D’Odorico and Fagherazzi, 2003). The CRHyME experience has shown how landslides and debris flow stability assessment cannot be treated deterministically since their triggering strongly depends on the choice of the model type (*FS* equations) and on stability parameters (the friction angle and the cohesion) which are parameters generally measured on the field or in a laboratory. In CRHyME, following some literature studies (Hengl et al., 2017; Yu et al., 2018; Chow et al., 1988; Dade and Friend, 1998), the cohesion was spatially distributed in the function of vegetation coverage bearing in mind the roots contribute to stability while the friction angle was correlated with the soil composition. On the other side, friction angle is in function of the soil texture, granulometry and consolidation, depending also on complex sediment dynamics and geological processes (de Vente and Poesen, 2005; Merritt et al., 2003; Shobe et al., 2017; Ballio et al., 2010; Kondolf, 1997). As a result, the calibration procedure within a sensitivity analysis was necessary case-by-case since these parameters correlate with several geo-morphological predictors. The assessment of the superficial geo-hydrological cycle cannot be evaluated precisely since its monitoring is currently still insufficient on a catchment scale (ISPRA, 2018; Gariano and Guzzetti, 2016). Even though surface mapping and inventory are supposed to increase their accuracy and completeness in the future thanks to remote sensing data (Ciampalini et al., 2016), some doubts remain about the possible improvements for other fundamental data required for slope stability and sediment transport routines. However, the databases adopted in CRHyME (Hengl et al., 2017; Huscroft et al., 2018; Ross et al., 2018) represent the very first attempt to overcome these issues having already made an important homogenization of the essential data required for geo-hydrological modelling.

## 900 **5 Conclusion**

Geo-hydrogeological processes have been conventionally studied separately in many engineering fields (hydrology, geology etc.). Hypothesis and simplifications adopted to make them more tractable have sometimes partially neglected their mutual interactions, possible chain effects and their embedded interdisciplinarity. Therefore, hydrological models that assess jointly the erosion, and sediment transport processes and evaluate shallow landslide instabilities are quite rare. In this sense, the CRHyME model was designed as a tool able to show a complete picture of the most significant geo-hydrological processes that may occur at the catchment scale.

CRHyME model was built ex-novo using Python programming language, implementing faster PCRaster libraries that can simulate hydrological processes in a very efficient way. CRHyME includes some of the common features of the classical spatially distributed hydrological model but its focus is on quantitative reconstruction of geo-hydrological hazards. CRHyME is characterized by six modules that reproduce hydrological balance over terrain and by a brand-new module deputed to simulate erosion, solid transport, shallow landslide and debris flow triggering at the catchment scale. In the field of geo-hydrological risk assessment, the integration of all those processes in a spatially distributed hydrological model represents a novelty.

Since the aim of our study was to build and facilitate the usage of the model indistinctly in any area of the globe, a deep  
915 investigation of the open-source repositories available for initial data has been carried out. The user-defined calibration  
parameters have been reduced to the minimum. Among them, erosion coefficients, average sediment diameters, cohesion and  
friction angle have been tuned following the strategies presented above. A sensitivity analysis has been carried out to simplify  
and accelerate the reconstruction of realistic hydrological initial conditions, adding the possibility to activate the restart option  
after a spin-up period. Moreover, the DEM's resolution scale dependency was investigated and detected by the results.

920 CRHyME was intensively tested to make it as general as possible and reproducible in whatever catchments. Our case studies,  
the Caldane basin, the Valtellina catchment, and the Emilia area, were chosen looking at the availability of historical data  
which are of paramount importance for model validation. The results have shown a fairly good reproduction of the past  
observations: the model is numerically stable (thanks to PCRaster libraries), conservative (no water or solid leakages outside  
the domain) and hydrologically consistent (compared to the reference hydrometers, the routed water volume shows  $NSE \approx$   
925  $0.8-0.9$  while discharges have lower performance,  $NSE \approx 0.2-0.4$ , especially for larger catchments regulated by hydropower  
plants); the solid discharge reproduced with downscaled EPM Gavrilovic's method is consistent with the observations (errors  
around  $\pm 20\%$ ), even though there are some uncertainties on  $D_{50}$  parameter; the triggering of shallow landslides and debris  
flows is comparable in number and spatial localization to the reference inventory (confirmed by ROC assessment). However,  
CRHyME's performances are rather sensitive to the quality of rainfall field data that should be accurate in spatial and temporal  
930 resolution to allow the code to correctly detect landslide triggering.

The efforts conducted with the creation of CRHyME go in the direction of a better investigation of geo-hydrological hazards.  
CRHyME is a multi-hazard model able to address and quantify at catchment scale several geo-hydrological processes that may  
occur simultaneously, are physically coupled and cannot be interpreted separately. With CRHyME is possible to overcome the  
software fragmentation currently present in the geo-hydrological field, answering the recent needs for multi-hazard  
935 quantification required not only in back analysis studies but also for a multi-risk now-casting at the Civil Protection level.

## Appendix A

In this section, an example of the CRHyME ".INF" file is reported. Each module contains the parameters, variables and other  
settings required for the computations. The ".INF" file reports the simulation time settings (e.g., starting date and ending date),  
940 the spatially distributed input data and the meteorological and climatological data series, the options of each computational  
module and the name of the output files. The ".INF" file is read by the "deterministic\_runner.py" file that starts the CRHyME  
model and its internal routines (Fig. 2): in "pre-processing.py", "reporting.py" and "plot.py" modules, variables are  
respectively defined, saved, and plotted following the formats and standards of the PCRaster libraries (Sutanudjaja et al., 2018;  
Karssenberget al., 2010). CRHyME's results are reported in two formats, a ".csv" datasheet or a ".netcdf" map (Jacob et al.,  
945 2014; Sutanudjaja et al., 2018). The first type is used to pick up information of a particular quantity at one location such as in  
the correspondence of a rain gauge or hydrometric station. The datasheet is organized with a first column containing the time  
step value while the subsequent columns contain picked information of one or more monitoring points. The ".netcdf" maps are  
produced to store information about the states and fluxes variables of the model. At each timestep, the quantity at the spatial  
resolution of the DEM model is saved within the ".netcdf" stack. The required variable to be sampled should be specified in  
950 the ".INF" file under the "reporting options": for ".csv" files a ".map" file containing the location of sample points while for  
".netcdf" the name of the variable required should be specified. Using the GDAL libraries for Python (GDAL/OGR  
contributors, 2020), the input/output geographical data has been converted to the PCRaster standard format ".map" for raster  
data (Karssenberget al., 2010; Sutanudjaja et al., 2018), considering WGS84 datum as a reference system for geographical  
projection. In the output's files: the lateral water fluxes  $F_{sub}(t)$ ,  $F_{GW}(t)$  and  $F_{kin-dyn}(t)$  are expressed in  $[m^3s^{-1}]$ ; the vertical  
955 water fluxes  $C_l(t)$ ,  $S_{ml}(t)$ ,  $I(t)$ ,  $ETc(t)$ ,  $R(t)$ ,  $L_{per}(t)$ ,  $Ex(t)$  and  $Ex_{GW}(t)$  are expressed in  $[mm \text{ timestep}^{-1}]$ ; storage  
quantities  $h_{snow}(t)$ ,  $h_{soilwater}(t)$ ,  $h_{groundwater}(t)$  and  $h_{runoff}(t)$  are converted into  $[m^3]$  for volumes simply multiplying

the storage height by the cell area extension of the DEM in [m<sup>2</sup>]. Further description of the sub-modules can be found inside the CRHyME's manual (Abbate and Mancusi, 2021a, b).

## 960 .INI FILE EXAMPLE

```

[globalOptions]
inputDir = ***\ModelCRHyME\CRHyME_Inputs_Trebbia
outputDir = ***\ModelCRHyME\CRHyME_Outputs_R
cloneMap = map\clone.map
965 institution = RSE_Ricerca Sistema Energetico
title = CRHyME project
description = by Andrea Abbate and Leonardo Mancusi, resolution = 90 m
resolution = 90
startSeries = 1985-12-31
970 startTime = 1986-01-01
endTime = 2005-12-30
timestep = 24
stampTimestep = 1
Restart = 1
975 Restart_Snow = \restarts\mod2\Restart_Snow.map
Restart_Surface = \restarts\mod2\Restart_Surface.map
Restart_Soil = \restarts\mod2\Restart_Soil.map
Restart_Ground = \restarts\mod2\Restart_Ground.map
Restart_SoilSed = \restarts\mod2\Restart_SoilSed.map
980
[climaOptions]
CLIMA_Switch = 1
Rain_NC4 = netcdf\euordhi_mod2_pr_day.nc
985
[meteoOptions]
input_tab = tab
mask = map\mask01.map
DEM = map\DEM_clip.map
z0 = tss\mod2\Z0_euordhi_mod2_tas_day.tss
990 TempRatio = tss\mod2\TCcoef_euordhi_mod2_tas_day.tss
z0MAX = tss\mod2\Z0_euordhi_mod2_tasmax_day.tss
TempRatioMAX = tss\mod2\TCcoef_euordhi_mod2_tasmax_day.tss
z0MIN = tss\mod2\Z0_euordhi_mod2_tasmin_day.tss
TempRatioMIN = tss\mod2\TCcoef_euordhi_mod2_tasmin_day.tss
995 infilRain_file = tss\2011_2016\Rain_TREBBIA_Precipitazione_ALL.tss
mayrainstat = map\Rain_Stations_Trebbia.map
LAT = 43
ETC_Switch = 1
Aspect = map\Aspect_Filled.map
1000 Slope = map\Slope_Filled.map
mysoilmap = map\CLC_9Cat.map
Kc_FAO = tbl\Kc_FA0.tbl
Albedo = tbl\Albedo.tbl
1005
[interceptionSnowOptions]
input_tab = tab
LAI_max = tbl\LAI_max.tbl
LAI_min = tbl\LAI_min.tbl

```

(CRHYME'S GENERAL OPTIONS)  
*(input directory)*  
*(output directory)*  
*(clone map for delimiting domain)*  
*(institution name)*  
*(project title)*  
*(project description)*  
*(spatial data resolution)*  
*(starting data of series)<sup>1</sup>*  
*(starting data of simulation)<sup>1</sup>*  
*(ending data of simulation)<sup>1</sup>*  
*(timestep resolution in hours)*  
*(stamp timestep in n° timestep)*  
*(activate restart option after spin-up)*  
*(snow height state for restart)*  
*(runoff height state for restart)*  
*(soil water height state for restart)*  
*(groundwater height state for restart)*  
*(sediment height state for restart)*

(CLIMA MODULE OPTIONS)  
*(enable reanalysis-climatic input data)*  
*(.netcf reanalysis-climatic input data)*

(METEO MODULE OPTIONS)  
*(folder containing .tab (txt) datasheet)*  
*(0-1 mask map, equal to clone.map)*  
*(elevation model DEM.map [m])*  
*(regression temp-elev: intercept)*  
*(regression temp-elev: angular coeff.)*  
*(intercept for MAX temp.)*  
*(angular coeff. for MAX temp.)*  
*(intercept for MIN temp.)*  
*(angular coeff for MIN temp.)*  
*(rain gauges time series .tss (txt))<sup>2</sup>*  
*(rain gauges location .map)<sup>2</sup>*  
*(latitude)*  
*(evapotranspiration calc. enabled)*  
*(aspect file .map [-])*  
*(slope file .map [-])*  
*(use of soil.map)*  
*(kc coefficient for FAO evapotras.)<sup>3</sup>*  
*(albedo coefficient for FAO evapotras.)<sup>3</sup>*

(SNOW AND INTERCEPTION MODULE OPTIONS)  
*(folder containing .tab (txt) datasheet)*  
*(LAI maximum index)<sup>4</sup>*  
*(LAI minimum index)<sup>4</sup>*

	SNOW_Switch = 1	(snow calc. enabled)
1010	[landSurfaceOptions]	(LAND SURFACE MODULE OPTIONS)
	input_tab = tab	(folder containing .tab (txt) datasheet)
	INF_Switch = 2	(infiltration calc. enabled) <sup>5</sup>
	sand_sup = map\Sand_SUP90C.map	(%sand on surface soil at 10cm depth)
1015	silt_sup = map\Silt_SUP90C.map	(%silt on surface soil at 10cm depth)
	clay_sup = map\Clay_SUP90C.map	(%clay on surface soil at 10cm depth)
	CoarseFrc_SUP = map\CoarsFrg_SUP90C.map	(%coarse on surface soil at 10cm depth)
	myrivermap = map\PathRiverSM.map	(river location .map) <sup>6</sup>
	Loss_River = tbl\Loss_RIV.tbl	(reduction coeff. for river losses) <sup>6</sup>
1020	Inf_CLC = tbl\Infiltr_CLC.tbl	(infiltration coeff. f(soil use))
	CN_I = map\CN_I.map	(SCS-CN method CN I .map)
	CN_II = map\CN_II.map	(SCS-CN method CN II .map)
	CN_III = map\CN_III.map	(SCS-CN method CN III .map)
	Initial_SM = 0.9	(initial condition of soil moisture)
1025	SoilDepth = map\BDRICM_M.map	(soil depth .map [cm])
	MaxWatStgTOP = map\TSH1_clip.map	(%max water storage soil 10cm depth)
	MaxWatStgBTM = map\TSH5_clip.map	(%max water storage soil 1m depth)
	sand_btm = map\Sand_BTM90C.map	(%sand on surface soil at 1m depth)
1030	silt_btm = map\Silt_BTM90C.map	(%silt on surface soil at 1m depth)
	clay_btm = map\Clay_BTM90C.map	(%clay on surface soil at 1m depth)
	CoarseFrc_BTM = map\CoarsFrg_BTM90C.map	(%coarse on surface soil at 1m depth)
	[groundwaterOptions]	(GROUNDWATER MODULE OPTIONS)
1035	input_tab = tab	(folder containing .tab (txt) datasheet)
	Sr_Falda = 0.8	(initial condition of groundwater table)
	Idro_Map = map\Idrogeology_Emilias_Trebbia.map	(hydrogeological .map) <sup>7</sup>
	Ks_GLHYMPS_exp = map\GLHYMPS_Emilias_Trebbia.map	(saturated permeability from GLHYMPS) <sup>7</sup>
	Permeability = tbl\IdrogeologyTabs\Permeability.tbl	(saturated permeability .tbl (txt)) <sup>7</sup>
1040	Anisotropy = tbl\IdrogeologyTabs\Anisotropy.tbl	(anisotropy coefficient .tbl (txt)) <sup>7</sup>
	Porosity = tbl\IdrogeologyTabs\Porosity.tbl	(porosity coefficient .tbl (txt)) <sup>7</sup>
	Storativity = tbl\IdrogeologyTabs\Storativity.tbl	(storativity coefficient .tbl (txt)) <sup>7</sup>
	Type_Depth = tbl\IdrogeologyTabs\Type.tbl	(hydrogeological reclassify .tbl(txt)) <sup>7</sup>
1045	[LandSlidesOptions]	(LANDSLIDE MODULE OPTIONS)
	LANDSLIDE_Switch_1 = 2	(landslide trigger calc. enabled) <sup>8</sup>
	C_Veg = tbl\C_Veg.tbl	(cohesion from vegetation .tbl(txt))
	Surcharge = tbl\Sur_Veg.tbl	(cohesion from vegetation .tbl(txt))
	X_Gavrilovic = tbl\X_Gavrilovic.tbl	(EPM X parameter .tbl(txt)) <sup>9</sup>
1050	Y_Gavrilovic = tbl\Y_Gavrilovic.tbl	(EPM Y parameter .tbl(txt)) <sup>9</sup>
	LithoY_Gavrilovic = map\Idrogeology_Emilias_Trebbia.map	(EPM Y parameter Lithology .map) <sup>9</sup>
	FI_Gavrilovic = map\Kst_Emilias_Trebbia.map	(EPM fi parameter .map) <sup>9</sup>
	[routingOptions]	(ROUTING MODULE OPTIONS)
1055	ROUTING_Switch = 1	(enable calc. routing)
	lddMap = map\ldd_clip.map	(ldd.map of flow directions)
	cellAreaMap = map\cellsizeArea.map	(map of cell area extension)
	River_Pit = map\Pit_Point.map	(basin outlet location)
	Strickler = tbl\Ks_Strickler.tbl	(Strickler-Manning coefficient)
1060	SectionTable = tbl\Dynamic\Sections2.tbl	(section type table .map) <sup>10</sup>

	[reportingOptions]	(REPORTING MODULE OPTIONS)
	mysamples_real = map\Idro_Samples_Trebbia.map	(real hydrometers sampling .map)
	mysamples_fake = map\Idro_Samples_F.map	(other hydrometers sampling .map)
1065	mysamples_solid = map\Solid_Samples.map	(reservoir sampling .map)
	outDailyTotNC = CumFails,CumFails_D	(daily counted .netcdf) <sup>11</sup>
	outMonthTotNC = P,Etc	(monthly counted .netcdf)
	outMonthAvgNC = T	(monthly averaged .netcdf)
	outMonthEndNC = CumFails,CumFails_D	(end-monthly counting .netcdf) <sup>11</sup>
1070	outAnnualTotNC = P,Etc	(annual cumulated .netcdf)
	outAnnualAvgNC = T	(monthly averaged .netcdf)
	outAnnuaEndNC = CumFails,CumFails_D	(end-annual cumulated .netcdf) <sup>11</sup>
	formatNetCDF = NETCDF4	(.netcdf specified format)
	zlib = True	(enable .netcdf creation)
1075		

ID	Description	Module	Additional References
1	The starting point of the time series, the starting point of the simulation and the ending point are specified.	[GLOBAL OPTIONS]	-
2	To compute rain gauge simulation, a time series in .tss format and a .map of stations are required. Each station has its IDs (1,2,3,...,n) for the corresponding time series with map.	[METEO OPTIONS]	(Karssenberget al., 2010; Sutanudjajayet al., 2018)
3-4	Fao crop coefficient Kc, albedo coefficient and LAI coefficient within .tbl file (a txt table).	[METEO OPTIONS] - [INTERCEPTION SNOW OPTIONS]	(Allan et al., 1998; Nazari et al., 2019)
5	Infiltration model selector: 1) Horton, 2) SCS-SN	[LANDSURFACE OPTIONS]	(Chow et al., 1988)
6	River map derived from PCR flow accumulation and percolation reduction factor below riverbed path.	[LANDSURFACE OPTIONS]	(Chow et al., 1988)
7	Groundwater parameters (.tbl), lithology map and saturated permeability map retrieved from literature and GHYMPS database.	[GROUNDWATER OPTIONS]	(Huscroft et al., 2018; Anderson, 2005; Hayashi, 2020; de Graaf et al., 2015)
8	Landslide model selector: 1) Iverson, 2) Harp, 3) Milledge and 4) SLIP	[LANDSLIDE OPTIONS]	(Iverson, 2000; Montrasio, 2008; Harp et al., 2006; Milledge et al., 2014)
9	EPM parameters from Gavrilovic's method (.tbl and .map)	[LANDSLIDE OPTIONS]	(Milanesi et al., 2015; Panagos et al., 2015)
10	Section table (.tbl) requires for implementation of dynamic routing (experimental)	[ROUTING OPTIONS]	(Karssenberget al., 2010; Sutanudjajayet al., 2018)
11	Cumulated shallow landslides and debris flow fails are sampled at yearly/monthly/daily bases	[REPORTING OPTIONS]	-

## Appendix B

In this section, main symbols and their measurement units included in CRHyME are reported (Abbate and Mancusi, 2021a, b).

Main symbols	Description	Units of measurement
<i>A</i>	Hydraulic section area	m <sup>2</sup>
<i>Δx and Δy</i>	Cell length and width	m
<i>B</i>	Width of the hydraulic section	m
<i>c</i>	Cohesion of surface soils	kPa
<i>C<sup>rs</sup></i>	Concentration of debris flows	-



$C_i$	Canopy Interception	mm day <sup>-1</sup>
<b>CNI CNII CNIII</b>	Curve Numbers SCS-CN for dry-mild-wet conditions	-
$D_{50}$	Median diameter of soil grain size	mm
$ddf_0$	Degree day factor	mm °C <sup>-1</sup> day <sup>-1</sup>
$E_s$ or $W_s$	Surface erosion (source parameter for EPM)	mm timestep <sup>-1</sup> or m <sup>3</sup> yr <sup>-1</sup>
$Et_0$	Potential evapotranspiration	mm timestep <sup>-1</sup>
$Et_c$	Evapotranspiration	mm timestep <sup>-1</sup>
$E_x$	Exfiltration	mm timestep <sup>-1</sup>
$f_0$	Maximum infiltration rate of Horton	mm h <sup>-1</sup>
$f_c$	Horton's minimum infiltration rate	mm h <sup>-1</sup>
$F_{gw}$	Groundwater flow	m <sup>3</sup> s <sup>-1</sup>
$F_{sub}$	Subsurface flow	m <sup>3</sup> s <sup>-1</sup>
$depth_{GW}$	Groundwater depth	mm or m
$depth_{Soil}$ or $Z$	Surface soil depth	mm or m
$h_{snow}$	Snow height	mm
$h_{runoff}$	Water height at the surface	mm
$h_{soilwater}$	Water height in surface soil	mm
$h_{groundwater}$	Water height in the aquifer	mm
$h_{solid}$	Sediment height at the surface	mm
$\alpha$ , $slope$ or $i$	Terrain slope (degrees and dimensionless)	° or %
$I_a$	Initial imbibition of the SCS-CN method	mm
$k$	Horton decay constant	h <sup>-1</sup>
$K_c$	Crop Coefficient	-
$K_s$	Hydraulic permeability	m s <sup>-1</sup>
$K_{Strickler}$	Strickler roughness coefficient	-
$LAI$	Leaf Area Index	-
$L_{per}$	Percolation	mm timestep <sup>-1</sup>
$n$	Porosity	-
$n_{VG}$	Van Genuchten n parameter	-
$P$	Rainfall	mm timestep <sup>-1</sup>
$P_n$	Net Rainfall	mm timestep <sup>-1</sup>
$F_{kin\_dyn}$ or $Q_l$	Liquid Discharge	m <sup>3</sup> s <sup>-1</sup>
$Q_c$	Critical flow rate of incipient motion for solids	m <sup>3</sup> s <sup>-1</sup>
$Q_s$	Solid flow rate	m <sup>3</sup> s <sup>-1</sup>
$R$	Runoff	mm timestep <sup>-1</sup>
$R_{EPM}$	Routing coefficient for EPM	-
$S$	Snow	mm
$S_{tor}$	SCS-CN Storativity	mm
$S_{ml}$	Snowmelt	mm timestep <sup>-1</sup>
$S_m$	Soil Moisture	%
$T$	Temperature	°C
$T_{max}$ and $T_{min}$	Maximum and minimum temperature	°C

$T_s$	Solid Transport	$m^3 s^{-1}$
$a_x$ and $b_x$	Parameters of <i>Slope</i> $\rightarrow D_{50}$ equations	-
$\alpha_{liquid}$ e $\beta_{liquid}$	Parameters of the uniform (liquid) flow rate curve	-
$\alpha_{solid}$ e $\beta_{solid}$	Parameters of the uniform (solid) flow rate curve	-
$\varphi$	Friction angle of surface soils	$^\circ$
$\theta_s$ e $\theta_r$	Maximum and minimum surface soil water content	mm or %

## 1080 Code and data availability

All the data shown in this paper are freely consultable on Internet websites as reported in the references and within the links we specified through the text. Since the CRHyME code is currently underdeveloped, we suggest you contact the main author at this mail [andrea.abbate@rse-web.it](mailto:andrea.abbate@rse-web.it) to receive the most updated and stable copy of the code. For functioning, the CRHyME code requires a Python environment (we suggest Python 3.7 or 3.8 version) and the installation of PCRaster libraries (see the 1085 references and links). Further details can be found in Abbate and Mancusi (2021a, b).

## Author contributions

AA and LM conceptualized the study. AA carried out the formal analysis and wrote the manuscript with contributions from all co-authors. FA, AF, LL and MP supervised the research, and all the authors reviewed and edited the manuscript.

## Competing interests

1090 The authors declare that they have no conflict of interest.

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