Rapid assessment of abrupt urban mega-gully and landslide events with Structure-from-Motion photogrammetric techniques validates link to water resources infrastructure failures in an urban periphery

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Abstract. Mass movement hazards in the form of mega-gullies and landslides pose significant risks in urbanizing areas, yet are poorly documented. To obtain primary data on the size, frequency and triggers of abrupt mega-gullies and landslides in urban areas, rapid assessment methods based on Structure from Motion (SfM) photogrammetric techniques and watershed models were developed and deployed in Los Laureles Canyon, a rapidly urbanizing watershed in Tijuana, Mexico. Three abrupt earth surface hazards were observed over a five-year study period including two mega-gullies and one landslide, and all were linked to a combination of rainfall and water resources infrastructure failures (WRIFs): (1) water main breaks resulted from rainfall-driven gully erosion that undermined supply lines, and the resulting water jets caused abrupt mega-gully formation; ~~we provide~~ this represents one of the first ~~ever detailed documentation of~~ studies to document this process in an urban environment; (2) antecedent saturation of a hillslope from a leaking water supply pipe contributed to an abrupt landslide during a storm event. The return period of the storms that triggered the WRIF-based earth surface hazards was ~1-2 years, suggesting that such triggering events occur frequently. WRIF-based hazards were also a non-negligible contributor to sediment generation at the watershed scale. While the number of observed events is small, these results suggest that WRIF can, in some cases, be the single most important process generating abrupt and life-threatening earth surface hazards on the poor urban periphery. Future studies of the triggers and mechanisms of abrupt urban mega-gullies and landslides should consider the role of WRIFs in antecedent saturation and erosion by broken water supply lines.

1 Introduction

Mega-gullies and landslides are significant earth surface hazards in urban areas, particularly in marginalized
35 neighborhoods on the periphery of large cities in low- and middle-income countries (Sidle et al., 2011; Anderson et al., 2014;
Makanzu Imwangana et al., 2015; Fu et al., 2020). Whereas landslides refer to the movement of sediment down a slope, gullies
are landforms comparable to small valleys or ditches that are carved by running water, and mega-gullies imply a width of at
least 5 m (Makanzu Imwangana et al., 2015). Mega-gullies and landslides can undermine or damage housing and civil
infrastructure and present life-threatening safety risks (Calvello et al., 2016; Peng et al., 2017; McAdoo et al., 2018). Mega-
40 gully and landslide hazards are increasing at a time of rapid urbanization as a result of limited oversight of planning and
construction as well as socio-economic pressures that force populations to settle in high-hazard areas (Hardoy et al., 2013;
Retief et al., 2016; Miller et al., 2019). For example, in Latin America, urban expansion on the periphery of large cities often
occurs on steep slopes (Sepúlveda and Petley, 2015), and unregulated expansion often results in poorly planned and
unmaintained infrastructure that is vulnerable to erosion and destabilization (Griffin and Ford 1980; Kjekstad and Highland,
45 2009; Biggs et al., 2010; Bianchini et al., 2017; Costa et al., 2018; de Albuquerque et al., 2020).

Earth surface hazards that occur abruptly (within a few hours) are of particular concern from a safety and damage
perspective, because there is little time for warnings and other emergency response measures. The literature mainly
characterizes the formation of mega-gullies as a result of landscape changes such as deforestation, roads, and urban
development (Archibold et al., 2003; Adediji et al., 2013; Makanzu Imwangana et al., 2015; Zolezzi et al., 2018), although
50 there have been studies documenting abrupt mega-gully formation in tropical environments (e.g., Moeyersons et al., 2015). In
both agricultural and urban areas, gully formation is associated with rain-generated runoff (Valentin, et al., 2005). However,
mega-gullies may also form abruptly in the presence of a high velocity water jet from a pressurized pipe, a process similar to
hydraulic mining used in mining operations in California during the 19th century (Gilbert, 1917). Furthermore, under rapid
urbanization with limited oversight of design and construction, water supply systems are vulnerable to breaks that trigger
55 hydraulic mining and the abrupt formation of mega-gullies on time scales of hours to days. In Tijuana, Mexico, local authorities
have observed hazardous mega-gullies from pipe breaks and hydraulic mining (Chief of Civil Protection, Tijuana Mexico,
personal communication, 2016), but the phenomenon has not been documented in the literature. Landslides may also occur
abruptly. Landslides refer to a wide range of phenomena associated with the downslope movement of earthen material (e.g.,
rock or soil) under the influence of gravity. In the Tijuana metropolitan area, rotational soil slides are the most common
60 landslide type and abrupt events have been recognized as a significant threat to public safety (Oliva-González et al., 2014).
Landslides occur when the weight of earth material down a slope exceeds its strength (Highland and Bobrowsky, 2008), a
process known as overloading that typically occurs with high soil moisture content following rainfall (Kuo et al., 2018;
Valenzuela et al., 2018; Zhuo et al., 2019; Monsieurs et al., 2019; Marino et al., 2020) or agricultural intensification (Lacroix
et al., 2020). Recent studies have also shown that leaky pipes and septic tanks contribute to overloading (Demoulin and Hans-
65 Balder, 2021). In summary, there is emerging evidence that both water resources infrastructure failures (WRIFs) and rainfall

can contribute to abrupt earth surface hazards in urbanizing areas. Furthermore, WRIFs have been documented to play a role in other land surface processes such as the generation of sinkholes (Kim et al., 2018), erosion (Guo et al., 2013), and destabilization of soil (Van Zyl, et al., 2013). However, the occurrence of abrupt mega gullies and landslides from WRIFs and the interdependence with rainfall is not well documented or understood, which is important to address given the threat of fatalities posed by abrupt hazards and the global growth of urban areas in the Anthropocene (Criqui, 2015; Ercoli et al., 2020).

Monitoring and analysis of abrupt earth surface hazards in urban areas is challenging. Earthwork typically proceeds quickly after an event to clean up or restore sites impacted by displaced sediment, and within days, the site is often so disturbed that it becomes impossible to perform a detailed investigation including measurement of feature size and identification of triggers. Access for monitoring also raises safety concerns due to the steep and unstable slopes. Structure-from-motion (SfM) photogrammetry presents a promising new approach to address these problems. SfM can safely monitor mass movement features with either on-ground or airborne platforms (Nadal-Romero et al., 2015; Eltner et al., 2016; Kaiser et al., 2018; Fugazza et al., 2018; James et al., 2019; Ma et al., 2020), and can be deployed quickly after an event to scan a site—providing data that can be used to estimate the dimensions and volumes of sediment displaced by erosional features. Furthermore, recent advances in the combination of UAS, SfM and MultiView-Stereo (MVS) algorithms facilitate data acquisition and processing to obtain high resolution point clouds, Digital Surface Models (DSMs) and orthophotos (Zhang et al., 2019).

Herein we present an observational and ~~modeling~~modelling study whereby SfM was deployed in a rapid-response mode to document the frequency and magnitude of abrupt earth surface hazards, to document the relative roles of WRIFs and rainfall in hazard formation, and to quantify the amount of sediment generated by the WRIF hazards compared to other rainfall-runoff processes. The study is conducted in Los Laureles Canyon watershed (LLCW) located in the urban periphery of Tijuana, Mexico, and builds on previous work by the authors to document soil erosion, sediment generation, and flood hazards at the watershed scale (Biggs et al., 2010, Luke et al., 2018, Gudino-Elizondo et al., 2019; Goodrich et al., 2020). To our knowledge, no study has examined the role of WRIFs in abrupt earth surface hazards, a topic of growing importance in the Anthropocene (Vanmaercke et al., 2016 and 2021; Poesen, 2018). The objectives of this paper are three-fold: (1) to provide primary data on size, frequency and triggers of abrupt mega-gullies and landslides that occur in an urban periphery, (2) to demonstrate a SfM based approach suited to the rapid response needs of abrupt earth surface hazards, and (3) to evaluate the significance of WRIF events with respect to mass movement hazards and sediment budgets at neighborhood- and watershed-scales.

The remainder of the paper is organized as follows: Section 2 (Methods) presents a site description, SfM-based observational methods, and watershed ~~modeling~~modelling methods for estimating components of the sediment budget; Section 3 (Results) presents SfM and ~~modeling~~modelling results showing the frequency and magnitude of WRIF-based mass earth surface hazards compared to other mechanisms of sediment generation; Section 4 (Discussion) contemplates the relative contribution of rainfall and WRIFs in the observed earth surface hazards, and the value of SfM in this context; and major findings are reported in Section 5 (Conclusions).

2 Materials and Methods

2.1 Site Description

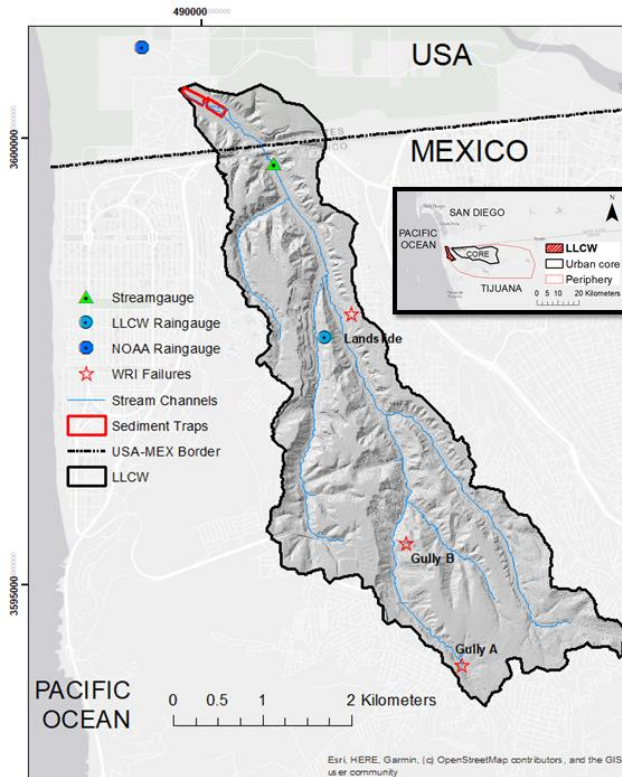
100 The Los Laureles Canyon Watershed (LLCW) is a small (11.6 km^2) urbanizing binational watershed on the urban periphery of Tijuana, Mexico (Fig. 1). LLCW flows from the city of Tijuana, Mexico, into the Tijuana River Estuarine Reserve, USA (Fig. 1). Excess sedimentation into the estuary, which is due to high sediment loads from Tijuana, have buried and impaired the estuarine ecosystem (Weis et al., 2001).

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Figure 1. Los Laureles Canyon Watershed (LLCW), locations of water resources infrastructure failures (WRIFs), and field equipment. Inset shows the regional location of the LLCW within the urban periphery. Base map from © OpenStreetMap contributors, 2020. Distributed under a Creative Commons BY-SA License.

110 The climate in LLCW is Mediterranean, with a wet winter, dry summer and an average annual precipitation of 240 mm. The regional geology includes marine and fluvial deposits of conglomerate, sandy conglomerate, and siltstone of the San Diego formation (Gastil et al., 1975, Minch et al., 1984). Soils are generally sandy with a wide range of cobble fraction, and are dominated by steep slopes (15 degrees, average). Urbanization in LLCW started in 1962, with most urbanization occurring between 1980 and 2002 (Biggs et al., 2017), mostly in the form of unauthorized housing developments ("invasiones").

115 Unauthorized construction of poorly planned housing, water distribution networks and roadways on steep slopes concentrated storm-water runoff and increased soil and gully erosion, slope instabilities, and failures in WRI. The LLCW is on the western periphery of Tijuana; the socioeconomic status of residents in the LLCW is low in the southern part of the watershed and in the areas with infrastructure failure as evidenced by a high marginality index and a low fraction of homes with piped water or drainage as compared with other areas of Tijuana (Biggs et al., 2014).

120 **2.2 Study Design**

Hydrologic conditions, slope instabilities, and sediment generation rates were monitored in the LLCW for a 5-year period beginning in January 2013 and ending in April 2018. A tipping-bucket rain gauge station ("LLCW rain gauge" in Fig. 1) was installed in the watershed, and a pressure transducer (PT) (Solinst, water level logger) was installed in a concrete channel at the watershed outlet and logged water level at 5-minute intervals (Fig. 1). Upon detection of flow at the watershed outlet, field

125 personnel travelled to the site, performed a visual inspection of site conditions, and, upon observation of mega-gullies and landslides, collected two types of data about the WRIF erosional features: (1) photogrammetric surveys (RGB images) were performed using either a ground-based or aerial platform, and (2) ground control points (GCPs) were acquired by differential GPS (Magellan Pro Mart 3) with sub-centimeter to 5 cm accuracy (Magellan Systems Corporation, San Dimas, USA). These primary data were used to create DSMs, and in turn, estimates of sediment volumes and their impacts on sediment budget, as

130 well as to document safety hazards to the people living in the watershed and downstream ecosystems.

A long-term record of rainfall is available from the NOAA Tijuana River Estuary gauging station, located near the outlet of the LLCW, which provides daily rainfall for the period 1980 to 2018, and estimates of daily rainfall back to ~1950 were reconstructed by regression with a nearby gage at Lindbergh airfield in San Diego (Brand et al., 2020). ~~These data are used here to estimate the return period of storm events during the study period. Gudino-Elizondo et al. (2019) used data from~~

135 ~~the tipping~~Tipping-bucket rain gauge (LLCW Raingauge in Fig. 1) were previously used to force a watershed erosion model~~,~~ which was validated with stream gauge data and observed sediment loads at the outlet. ~~Rates~~ (Gudino-Elizondo et al. 2019), and this model was applied here to estimate rates of sediment generation by sheetwash, rill, gully and channel erosion estimated ~~by the model were compared~~for comparison with sediment ~~generation from WRIF features~~ volumes documented by SfM as described next.

 2.3 Image acquisition and processing

Photogrammetric surveys were performed using a modified nonmetric camera (GoPro Hero3+) with a non-distortion lens, 4.14mm focal length, f/3.0 aperture and 12 Megapixel image size (Peau Productions, CA, USA, http://www.peauproductions.com/) mounted either on an Unmanned Aerial System (UAS) (DJI, Phantom2) or a telescoping painter's pole (approximately 2-3 m long). The UAS is advantageous for relatively large and wide erosional features compared with the painter's pole, which can better access relatively small, narrow, and deep erosional features (Gudino-Elizondo et al., 2018a, Taniguchi et al., 2018). Images were acquired once per second using the time-lapse capture mode from different angles to ensure a high overlap between photographs and to reduce the shade in each image (Castillo et al., 2015) and doming deformations (James and Robson, 2014).

The sediment volume mobilized was estimated using a four-step procedure: (1) Imagery were combined with a subset of the GCPs to calibrate the camera and produce Structure from Motion (SfM) point clouds following general workflows (Agisoft LCC, Russia, Version 1.4.4), (2) SfM point clouds were converted to a digital surface model (DSM) (Agisoft LCC, Russia, Version 1.4.4), (3) erosional volumes were computed (ArcGIS 10.6.1, ESRI, Redlands, California) by subtracting the DSM from a reference DSM representative of the pre-event land surface (Wheaton et al., 2010), and (4) the difference of DSMs (DoD) was integrated to calculate the total sediment volume (James et al., 2012). Volumes were converted to mass using a bulk density of 1,600 kg /m^3 corresponding to very fine sand (USDA, 2018).

Pre-event topography was based either on a 2014 aerial LIDAR survey (1 m resolution Digital Surface Model (DSM) with a 0.11 m vertical RMSE, NOAA, 2014), or on UAS-based DSMs generated with imagery collected before the failure event (Table 1). The horizontal and vertical RMSE of the point clouds, or geo-registration error, was estimated using the subset of the GCPs not used to produce the SfM point cloud, called Error Control Points (ECPs). Previous work indicates that 4 to 5 GCPs with a few additional ECPs are adequate for SfM processing (James et al., 2017). The RMSE for the DoD was computed as the square root of the sum of the squared errors for each DSM (Alfonso-Torreño et al., 2019).

Table 1. Structure from Motion survey description and data acquisition

Erosional feature	Landslide	Mega-gully A	Mega-gully B
Acquisition platform	UAS	UAS	Pole
Date of survey	05/22/2015	09/23/2015	02/17/2017
Number of pictures	62	115	899
Altitude (m)	75 m	30 m	4 m
Area covered (m^2)	38,400	1,800	2,800
Ground sample distance (cm/pxl)	10	4	3
Point density (points/m^2)	11	48	261

Numbers of GCPs	8	8	12
Numbers of ECPs	6	6	10
Pre-event topography	LIDAR-DSM	SfM	SfM

165 The dimensions of invariant features (concrete pads, water pipes, etc.) were directly measured in the field and compared to length estimates from the SfM point cloud as described in Gudino-Elizondo et al. (2018a). Additionally, pre- and post-event ground elevations were compared along transects outside the disturbed region where no topographic change was observed to assess co-registration errors of the DoDs calculation.

2.4 Watershed ~~Modeling~~Modelling

170 The Annualized AGricultural Non-Point Source (AnnAGNPS) model (Bingner et al., 2015) was applied to the LLCW to simulate discharge and sediment load during storm events and to develop an inventory of sediment generation rates by mechanism at the watershed scale. The AnnAGNPS model was previously calibrated and validated for runoff and observations of sediment generation in LLCW (Gudino-Elizondo et al., 2018a, 2018b, 2019), and the applications here rely on this calibration. The simulation period was from water year 2012 to 2017 to match the observation period of the mega-gullies and

175 landslide. Sediment excavation rates from sediment traps at the LLCW outlet (Fig. 1) were used for model calibration. The sediment traps were excavated annually from 2007–2012 (N = 7). Uncertainties in the ~~modeled~~modelled sediment yield were previously reported by Gudino-Elizondo et al. (2019) as approximately 10%, with a normalized RMSE of 48%.

Measurements and ~~modeling~~modelling supported an inventory of sediment generation and load from four mechanisms: (1) sheet and rill erosion, (2) gully erosion, (3) channel erosion, and (4) erosion from WRIF. Sediment generation

180 was considered as the total mass of sediment mobilized, while the sediment load was the quantity of sediment observed at the watershed outlet. Sediment load from WRIF was calculated by multiplying the erosion volume per event times the Sediment Delivery Ratio (SDR). For mega-gullies, the SDR was set to 1 based on field observations and ~~modeling~~modelling work described in Gudino-Elizondo et al. (2019). Conversely, the SDR was set to zero for the landslide based on field observations that displaced sediment was intercepted by the road network and mechanically removed or repositioned on the hillslope (Vigiak

185 et al., 2012). Subsequent rainfall events may cause the repositioned sediment to be later mobilized and moved towards the stream network, so our estimates of load correspond only to the period of observation.

2.5 Hazard assessment from water resources infrastructure failure

Reports of the damage caused from mega-gullies and landslide were compiled from residents and local agencies. Primary data from the three events are described, including impacts to transportation, housing, urban infrastructure and downstream

190 ecosystems and communities in the study watershed. The year of urbanization of the neighborhoods where the WRIFs were occurred was determined from an existing dataset (Biggs et al, 2010). The specific soil loss (SSL) of the WRIF mega-gullies was calculated as the total erosion (m^3) normalized by the watershed area (m^2) and was then compared to the observed SSL in

the study watershed and to other studies reported in the literature. A detailed description of safety hazards and the contribution to the total sediment budget of each WRIF event is described in section 3.

195 **3 Results**

A total of 14 storm events was observed during the 5-year study period, based on a flow threshold of 1 m^3 s^{-1} (or ~15 cm of water in the channel) at the gaging station, which corresponds to a depth of rainfall ranging from 6.5 to 13 mm. The total depth of the 14 storms was 322 mm, or 35% of the total rainfall (907 mm, 181 mm y^{-1}) for the 5-year period. Mass movement from WRIFs were observed during three of these events, each characterized by a 1-2-year return period, based on precipitation depth

200 frequency information reported by NOAA Atlas 14 for nearby Imperial Beach, California (NOAA, 2021). WRIFs leading to mass movement were not observed between storm events or during smaller storm events (< 23 mm). Figure 2a shows the reconstructed rainfall time series for the period 1950-2017 which demonstrates variability over roughly seven decades, and Figure 2b shows the timing of WRIF-based mass movement events alongside measured daily rainfall for 2013-2017. Both mega gullies events are associated with rainfall that exceeds the threshold for rainfall-driven gully formation reported by

205 Gudino-Elizondo et al. (2018a) but is not exceptional in magnitude considering the long-term record. The year of urbanization of the neighborhoods was 1980 (landslide), 2010 (mega-gully A), and 2002 (mega-gully B), respectively. Hence, the landslide is associated with a later stage of urbanization (34 years) compared to mega-gully A (4 years) and mega-gully B (15 years).
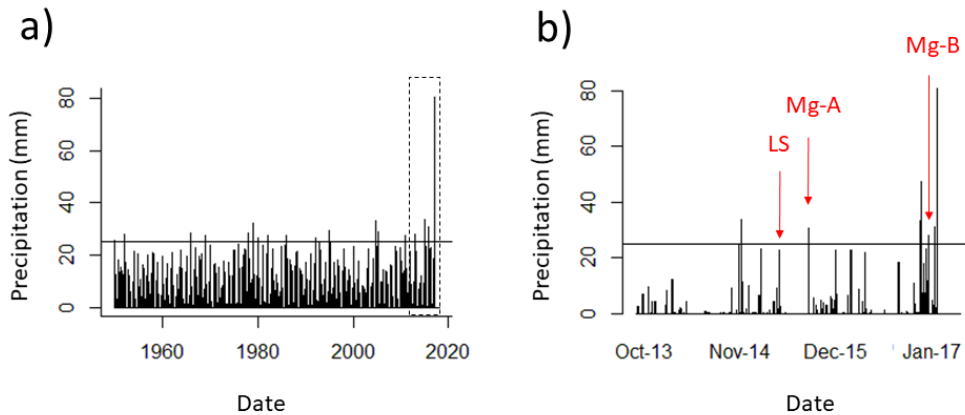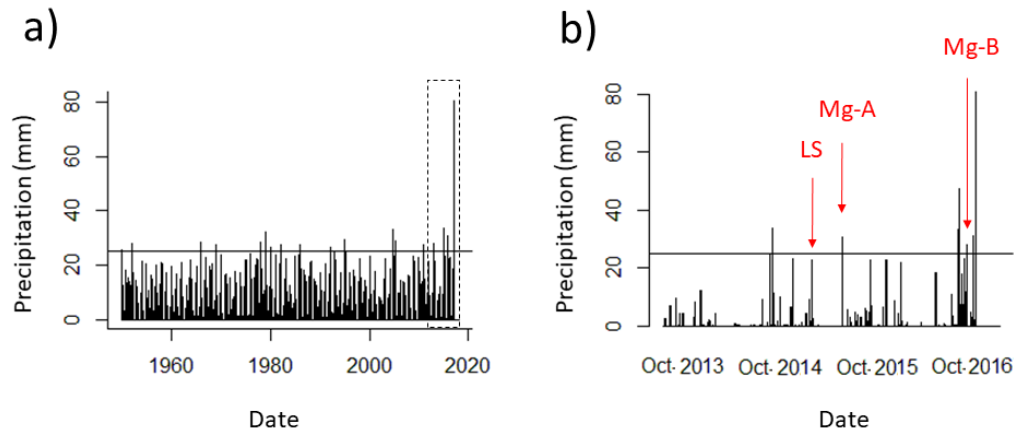
Figure 2. a) Reconstructed daily rainfall time series measured at the watershed outlet for 1950-2017, with horizontal line representing the rainfall threshold for rainfall-runoff gullies formation (Gudino-Elizondo et al., 2018a); and b) measured rainfall for water years 2013-2017 and timing of the observed mega-gullies (Mg) and landslide (LS).

3.1 SfM surveys and social impacts by event

Detailed observations of each feature are described in sections 3.1.1 to 3.1.3.

3.1.1 Landslide

A large rotational landslide occurred during a storm event on 15 May 2015 (Fig. 3a and 3b). More than 20 houses and public transportation were damaged permanently, affecting 100's of people. Based on the daily rainfall total (23 mm) and the long-term rainfall record at the NOAA Tijuana River Estuary Gage, the return period of the storm is 1 year. The landslide was attributed to a WRIF based on resident reports of seepage from the slope several days immediately before the failure incident and prior to rainfall, and of broken water mains observed after the landslide (Fig. 3c, and 3d, red circle). Observation of incipient cracks in the soil prior to failure also triggered the evacuation of the residents. The main scarp was approximately 20 m high and approximately 75 m long, with a maximum width of 40 m measured from the main scarp to the toe of the hill. SfM photogrammetry leads to an estimate for the sediment volume and mass: 19,900 $\pm$ 170 m^3 and 31,900 $\pm$ 280 metric tons. The reported uncertainties ($\pm$ value) are obtained by propagating vertical RMSEs of individual DSMs in the DoD calculation.
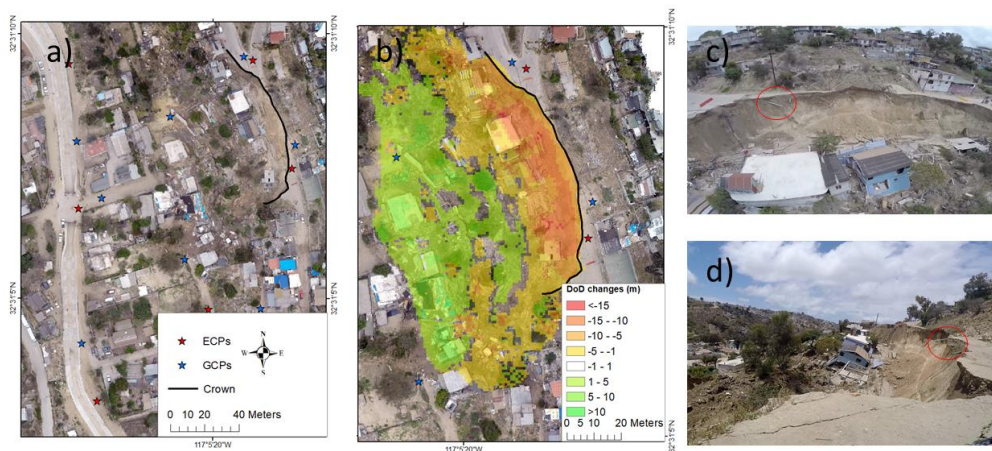
Figure 3. Landslide event on 15 May 2015 triggered by water main leak and rainfall: (a) Orthophoto acquired after the WRIF showing the upper limit of the main scarp (crown) with the black line, (b) the DoD changes from the multi-temporal analysis, (c) oblique aerial and (d) ground-based photographs of the landslide showing broken water mains inside the red circle aligned with the main scarp.

The RMSE (ECPs, n=6) of the DSM obtained from SfM was 3 cm in the horizontal and 7 cm in the vertical coordinates. Elevation differences outside of the disturbed area were <7 cm. Therefore, the co-registration error is assumed to be negligible. The mean difference between measured and ~~modeled~~modelled lengths of objects at the site (e.g., sewer manhole covers) was less than 3 cm. These different methods all suggest the error was less than 7 cm and within the range expected for the observation distance. The DoD map (Fig. 3c) shows the geometry of the landslide as a deep rotational slope failure, which is consistent with the model proposed by Highland and Bobrowsky (2008).

3.1.2 Mega-gully A

Mega-gully A formed along an unpaved road during and immediately following a storm event on 15 September 2015. Based on the daily rainfall total (31 mm) and the long-term rainfall record at the NOAA rain gauge, the return period of the storm was 1-2 years. This mega-gully was attributed to a WRIF based on resident reports that discharge from a broken pipe was observed upstream immediately after the failure event (personal communication, Tijuana Metropolitan Planning Institute). In this case, erosion caused by the storm event undermined the water main, which subsequently broke and enlarged the gully as a result of high velocity water jets from the pressurized water main. The mega-gully was 98 m long, with a maximum width of 8 m and maximum depth of 4 m (Fig. 4). The generated sediment mass was estimated as 1,360 ± 35 tons.
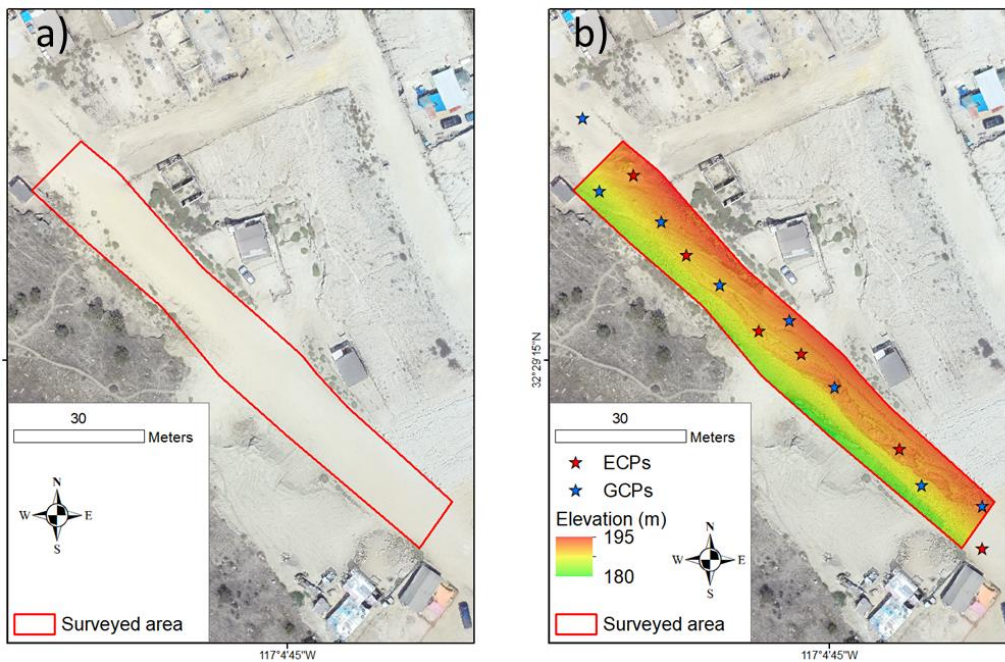
Figure 4. Mega-gully formed on 15 September 15, 2015 by WRIF: (a) Orthophoto acquired before the WRIF showing the extent of the surveyed area centred in the red rectangle, and (b) the resulting DSM showing the spatial distribution of Ground and Error Control Points (GCPs and ECPs, respectively).

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The RMSE (ECPs, n=6) of the DSM obtained from SfM was 3 cm horizontal and 5 cm vertical. Elevation differences outside of the disturbed area were 0-5 cm. Field measurements of the mega-gully width and depth differed from the SfM-derived width and depth by less than 2 cm on average. This WRIF caused the interruption of water supply for 1 month, affecting more than 300 residents (personal communication, Tijuana Metropolitan Planning Institute). The corresponding erosional feature

255   impacted public transportation and life quality to the neighbourhood for 6-9 months (available Google Earth imagery 11 December 2015- 08 August 2016) before the road was repaired.

3.1.3 Mega-gully B

A second mega-gully (B) formed along an unpaved road (Fig. 5a) following a storm event on 16 December 2016. Based on the daily rainfall total (33 mm) and the long-term rainfall record at the NOAA rain gauge, the return period of the storm is 1-

260 2 years. The mega-gully was largest at the upslope position and decreased in cross sectional area with distance from the broken pipeline. The mega-gully was 202 m long, with a maximum width of 10 m and maximum depth of 7 m. Imagery was collected for the SfM processing using a telescoping painter's pole, and sediment generation was estimated to be 4,340 ± 155 tons.
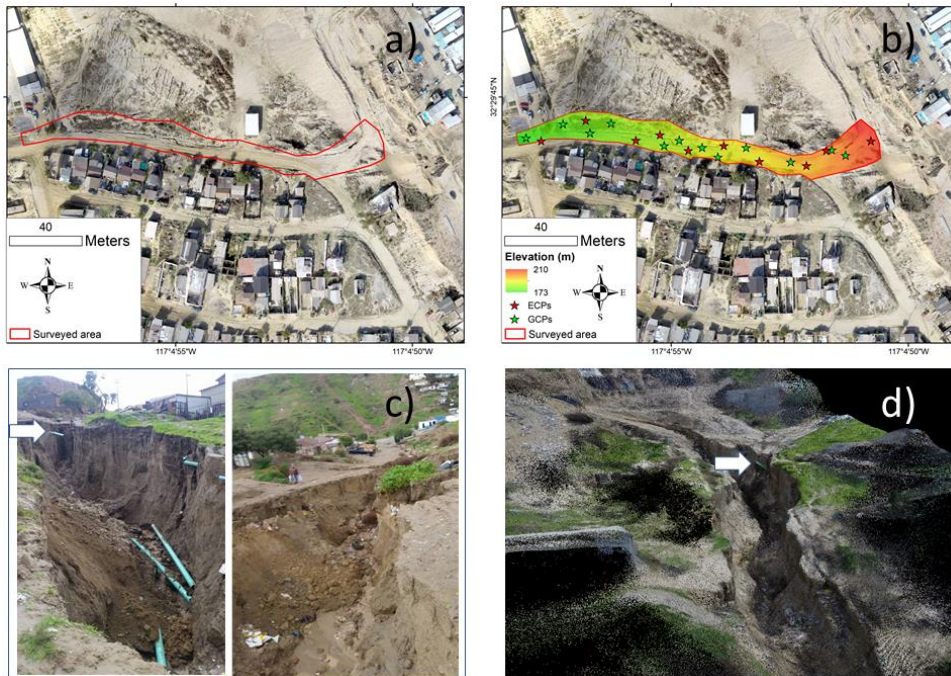
Figure 5. Mega-gully formed on 16 December 2016 triggered by WRIF: (a) Orthophoto acquired before the WRIF
265 **showing the extent of the surveyed area centred in the red polygon, (b) the resulting DSM after the WRIF, (c) Images depicting the mega-gully with view angles looking upslope and downslope of the WRIF, and (d) a screenshot of the resulting point cloud.**

The RMSE (ECPs, n=10) of the DSM (Fig. 5b) obtained from SfM was 3.5 cm in the horizontal and 5 cm in the vertical.
270 Elevation differences outside of the disturbed area were 0-5 cm, which is consistent with the accuracy of the method. For example, differences between measured and ~~modeled~~modelled lengths of non-deforming objects at the site (e.g., water supply pipes shown as white arrows in Fig. 5c and Fig. 5d) were less than 1 cm.

For this second mega-gully event, mass movement was again triggered by erosion that undermined the water main, which subsequently broke and enlarged the gully by discharging piped water directly onto the hillslope. Broken water main pipes

were observed during the rapid-response survey (Fig. 5c). The mega-gully also impacted public transportation and life quality in the neighbourhood for 6 months (based on Google Earth imagery) and interrupted water supply for 1 month, affecting more than 200 people (personal communication, Tijuana Metropolitan Planning Institute).

3.2 Comparison of Sediment Generation Sources

Application of the calibrated AnnAGNPS watershed model to storm events for 2012-2017 yielded daily estimates of rainfall-based sediment generation by sheet and rill erosion, gully erosion and channel erosion. Table 2 presents sediment generation (by mass) on a storm event basis, showing the amount of sediment generation associated with WRIFs measured using SfM, and the simulated total watershed sheet and rill erosion, gully erosion, and channel erosion at the event-scale. Additionally, Fig. 6 shows the relative contribution of WRIF and rainfall-based sediment generation mechanisms.

Table 2. Sediment generation by process during storm events with WRIFs in the Los Laureles Canyon Watershed.

Erosional hazard event	Sediment Generation Mechanism (tons)				
	Measured		~~Modeled~~Modelled		Total
	Water Resources Infrastructure Failures	Channel Erosion	Sheet and Rill	Rainfall-runoff gullies	
Landslide	31,900 ± 280	7,610	5,310	10,500	55,300
Mega-gully A	1,360 ± 35	2,290	4,710	49	8,410
Mega-gully B	4,340 ± 155	5,910	12,100	160	22,500

~~This analysis shows that mass~~Mass movement associated with WRIFs was significant on an event basis. Mega-gully B generated 4,340 tons (Table 2), which is approximately 80 times the area-normalized annual erosion rate for gullies (tons/ha) and 10 times the total sediment generated by other rainfall-generated gullies (Gudino Elizondo et al., 2018a, Gudino Elizondo et al., 2018b). The WRIF-triggered landslide mobilized more sediment than all of the rainfall-based processes combined, while the mega-gullies triggered by pipe failures and hydraulic mining were responsible for 16 and 20% of the total sediment generation across the watershed (Fig. 6).

The proportion of sediment generated by each erosional process differed markedly between the landslide event and the two mega-gully events (Fig. 6); rainfall-generated gullies contributed more sediment during the landslide event because peak discharge, the main control on gully formation, was higher during the landslide storm event (19.5 cms at the outlet) than during the two mega-gully events (~5 cms) (Gudino-Elizondo et al., 2019).
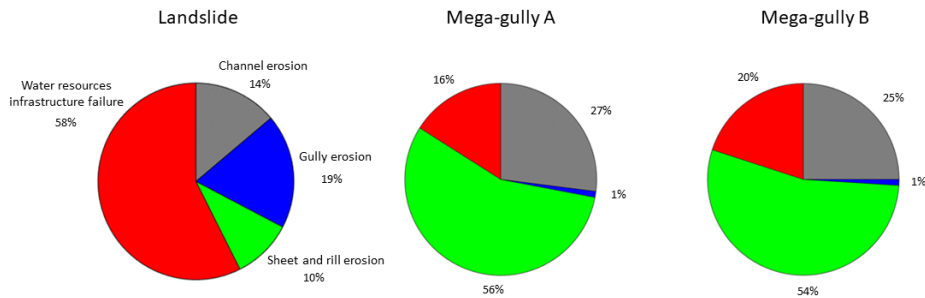
Figure 6. Relative contribution of sheet and rill erosion, gully erosion, channel erosion and water resources infrastructure failure (WRIF) towards sediment generation for three storm events with WRIF.

The total sediment generation and load were computed for the 5-year study period by integrating over all storm events (Table 3). On a five-year basis, WRIFs contributed 5% of the total sediment generation and approximately 2% of the total sediment load at the watershed scale. While the sample size here is small, the frequency of WRIF-based erosional events can be estimated in several ways: three hazard events occurred over a period that had 14 rainfall events (21% of rainfall events), two out of five years had at least one hazard event (40% chance per year), or three events occurred in five years (60% chance per year). The small sample size implies a high degree of uncertainty in all of these estimates; nevertheless, these rates of occurrence are far higher than typical design standards for water resources infrastructure in urban areas. For example, large flood control channels are typically designed with a 0.2-2% annual exceedance probability, and smaller drainage systems in urban areas are often designed for 5-10% annual exceedance probability. Hence, WRIF-based hazards observed during this study are many times more frequent (21-60%) than typical design standards for flood control systems in urban areas (0.2-10%) and thus deserving of greater attention for public safety, infrastructure resilience and environmental protection.

Table 3. Five-year total sediment generation and load rates (by process), fraction of total generation, and fraction of total load for the Los Laureles watershed.

Generation Mechanism	5-years Total Sediment Generation (tons)	5-years Total Sediment Load (tons)	Fraction of Total Generation (%)	Fraction of Total Load (%)
WRIF	37,566	5,696	5	2
Sheet and Rill	258,592	197,538	34	48
Rainfall-runoff gullies	228,207	75,253	30	18
Channel Erosion	234,150	131,212	31	32
Total	758,515	409,699	100	100

3.3 Comparison to Other Erosional Features

320 The mega-gullies observed here are large compared to rainfall-generated gullies surveyed in the study area (Gudino-Elizondo et al., 2018a), which had a mean gully width of 1.5 m and a mean depth of 0.5 m. In terms of size, mega-gully A is up to 8 m wide and 4 m deep, and mega-gully B is up to 10 m wide and 7 m deep. Mega-gullies A and B are also long with lengths of 100 and 200 m, respectively. Mega-gullies A and B were also more developed than rainfall-generated gullies, with greater connectivity to the stream channel, which enhances sediment delivery to the stream network. Figure 7 presents an aerial image
325 which provides a visual comparison of mega-gully B to a rainfall-driven gully network on a neighboring unpaved road.

Figure 7. High-resolution photograph showing the contrast between gullies generated by rainfall-runoff only (road on left) and mega-gully B (road on right).

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Figure 8 shows that the specific soil loss (SSL, the average depth of soil loss in the watershed) from mega-gully B was exceptionally high compared to rainfall-runoff gullies in the LLCW (Gudino-Elizondo et al., 2018a) and compared to sites reported by Castillo and Gómez (2016), which included sites spanning different land uses and precipitation regimes. The SSL from mega-gully A was comparable to other gullies observed in the study watershed, which has higher rates of SSL than the

335 set of sites reported by Castillo and Gómez (2016).
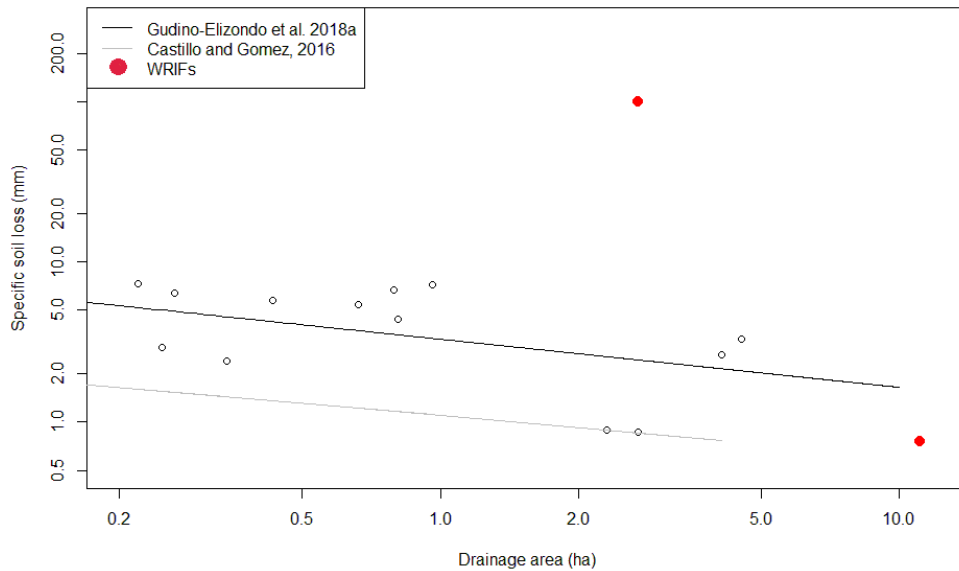
Figure 8. Specific soil loss of mega-gullies caused by WRIF (red dots) compared to previously reported gullies in Tijuana, Mexico (circle points and black line, Gudino-Elizondo et al., 2018a) and trends for ephemeral gullies reported from other sites (gray line, Castillo and Gómez 2016).

The landslide caused by the WRIF was the single largest erosional feature observed in the watershed during the study period. Landslides occur throughout Tijuana, with more than 40 landslides from 1992-2012, including a landslide that damaged 19 buildings (Oliva-González et al., 2014), which is comparable to the LLCW slide (20 buildings damaged). However, data describing the sediment displaced by these 40 landslides and potential connections to WRIFs were not available. More research is needed to establish the links between WRIFs and landslides.

4 Discussion

4.1 Rapid methods for monitoring erosional features

Landslides and mega-gullies have complex topographies and are poorly suited to the application of traditional surveying techniques such as total stations, but are well suited to photogrammetric characterization using SfM. In our study,

photogrammetric data were effectively captured for mega-gullies roughly 5-10 m wide, 5-10 m deep, and >100 m long using a camera mounted on a telescoping painter's pole, and landslides were safely characterized using a UAS-based platform. James and Robson (2012) introduced the relative precision ratio for UAS-SfM applications (i.e., ratio of measurement precision to observation distance), and found that a precision ratio of 1:950 indicates acceptable accuracy over a range of scales. For a flight height of 75 m, the James and Robson (2012) standard gives a desired DSM error of 7.8 cm, which compares well with the horizontal (3 cm) and vertical (7 cm) errors estimated here. The errors in the DSMs were very small (<=5 cm) compared to the size of the features (5-10 m). The presence of urban infrastructure in photographs (e.g., concrete pads, sewer structures) also presented opportunities for ground control points and accuracy checks. Elevation differences outside of the disturbed areas were <7 cm, indicating minimum co-registration errors. Errors in sediment volume estimates were also small (1 to 3%). The DoD also helped to characterize the landslide as a deep rotational slope failure, consistent with other landslides reported in Tijuana, which are linked with unplanned urbanization on hilltops and enhanced pore pressure induced by uncontrolled water leakage (Oliva-Gonzalez et al., 2014). We also note from the DSM analyses that the terrain slope was associated with the depth of incision of WRIF mega-gullies. Mega-gully B was 2-3 times deeper than mega-gully A, which formed on a relatively flat area. Accuracies achieved from these observations, both from individual point clouds and DoD's calculations, are in line with the needs for erosion hazards surveys and sediment budget applications (Dietrich, 2016; Alfonso-Torreño et al., 2019; Ma et al., 2020). The SfM photogrammetric approach is in many ways ideal for surveying WRIFs because: (1) the camera is lightweight and easily mounted on either a ground-based or aerial platform; (2) SfM requires less field personnel reducing significantly human-based errors and is more time efficient than traditional topographic surveying methods (Carrera-Hernández et al., 2020); and (3) hardware costs are relatively low. Laser scanning or LIDAR systems could be advantageous compared to photogrammetry in terms of point density and data accuracy, but they are typically quite heavy compared to cameras, require more sophisticated spatial referencing systems (e.g., inertial navigation units), often present occlusion artifacts, and are expensive (Izumida et al., 2017; Mazzoleni et al., 2020). Our study demonstrates that both ground-based and UAS-based photogrammetry allow for rapid documentation of hazardous erosional features with minimal equipment and low labor requirements. Recent advances in UAS-mounted Real-Time or Post-Processing Kinematic (RTK, PPK) georeferencing systems allow rapid mapping over relatively large areas without GCPs (Zhang et al., 2019), enhancing the potential for rapid response surveys programs, especially in dangerous and inaccessible terrains.

4.2. WRIFs and the sediment budget

Stochasticity in WRIFs and WRIF-based sediment hazards is high. Failures may or may not happen in any given storm (here we observed 3 failures in 14 storm events), and when failures occur, the volume of sediment generated across three events varied by over an order of magnitude. This makes it difficult to generalize and estimate sediment generation by infrastructure failure for other events lacking field observations. However, the data do allow a first-order estimate of annual-average sediment generation from WRIFs which is useful for sizing sediment basins that protect downstream ecosystems from excess sedimentation and for estimating average-annual excavation costs. We found in previous research that rainfall-runoff gully

385 erosion rates are higher on steep sandy soils (Las Flores soil type) (Gudino-Elizondo et al., 2019) and a rainfall threshold to generate rainfall-runoff gullies on those unpaved roads (>25mm) was also observed (Gudino-Elizondo et al., 2018a). Therefore, WRIF mega-gullies in Tijuana are more likely to occur on sandy soils on steep terrain during storm events equal or greater than the threshold precipitation typically required to produce rainfall-runoff gullies on unpaved roads (Figure 2). Such estimates would not likely be applicable outside of the LLCW, but the photogrammetric methods deployed here to monitor

390 sediment generation are easily transferrable to other systems, and data on sediment generation from multiple sites would provide a basis for improved understanding and possibly transferrable models.

4.3 Feedbacks between urbanization processes, erosion and slope instabilities from WRIFs: opportunities for hazard mitigation

Erosion and hazards produced by WRIFs were either exacerbated or triggered by erosion during storm events. The observed

395 landslide was triggered by a storm event, but the event was preceded by the water main leak ~~creating~~that created the preconditions for a landslide. The observed mega-gullies formed after local runoff initially undermined water mains, which then broke and discharged water onto the hillside, triggering more severe gully erosion. This suggests that WRIFs, storm events, and slope instabilities are interdependent. Moreover, this opens the possibility of reducing mass movement hazards through improved design, management and oversight of water resources infrastructure. Whereas rapid urbanization is broadly

400 linked to minimal levels of governance and institutional oversight of urban infrastructure, especially in least-developed countries (Borelli et al., 2018), water resources infrastructure benefit from relatively high levels of planning, design, engineering and oversight (Whittington et al., 2009; Cook, 2011). For example, mass movement hazards could be reduced by aligning water mains away from topographic low spots susceptible to gully formation, and away from hillslopes that may be susceptible to creeping displacements that stress pipes and cause leaks. Pipeline specifications could also be changed to

405 promote greater ductility, or resistance to failure, under hillslope displacement (Honegger et al., 2010; Han et al., 2012). In turn, the water resources infrastructure would benefit from fewer leaks and breaks and higher levels of reliability.

The decadal development of the urban surface is a critical control on the occurrence of WRIFs. While other studies ~~highlighted~~described mega-gullies that develop over years (Archibold et al., 2003; Makanzu Imwangana et al., 2014) or hours (Moeyersons et al., 2015), ~~our~~ mega-gullies in our study site can develop over single storm events due to WRIFs, and pose

410 significant "abrupt" hazards to the population. Any latency between urbanization and WRIFs is due to delay in the installation of water infrastructure, as settlement and road construction commonly precedes installation of sub-surface water infrastructure by several years. The spatial location of the WRIFs is governed by the temporal sequence of urbanization and land cover transformation that occurs over decades (Biggs et al, 2010). In Tijuana, mega-gullies occurred on unpaved roads in relatively recently urbanized areas (< 20 years urban) in the poor periphery, where the water distribution network was buried ~0.5-1m

415 below the surface and easily undermined by rainfall-runoff erosion of the unpaved road. Satellite observations suggest roads remain unpaved for decades following urbanization in Tijuana (Biggs et al, 2010), with consequent chronic exposure of the community to WRIFs. Roads are gradually paved over several decades, starting with the main transit corridors and followed

by smaller roads in residential neighborhoods. As the network is paved, the water distribution network is more protected from road destruction during storm events. We thus anticipate that the occurrence of mega-gullies due to WRIFs will become less common with buildout and road paving but could remain a chronic problem in marginalized neighborhoods on the urban periphery, where socioeconomic status is low (Biggs et al, 2014). The landslide, by contrast, occurred in an area that had been urbanized for longer (~40 years); this kind of hazard could occur in older and wealthier neighborhoods on steep slopes if the water supply network develops leaks (Oliva-González et al., 2014). While other factors such as overloading by heavy construction and water towers may contribute to landslides in some urban contexts, the buildings in our study were single story single family residential units with minimal foundations and likely small impact on landslide risk (Demoulin and Hans-Balder, 2021). Rather, overloading by soil moisture from WRIFs was likely the trigger of the landslide in Tijuana.

4.4 Implications for hazard mitigation in other urban contexts

~~We provide~~ This study represents one of the first ~~ever documentation of~~ instances where abrupt mega gully formation by water infrastructure failure has been documented in an urban environment. Such WRIF mega gullies may be under-reported due to their occurrence in the poor urban periphery, and more research is needed to support sustainable, safe, and equitable urban environments for ~~the~~ poor communities. Landslides and mega-gullies like those observed in Tijuana have been reported across cities in middle and low-income countries where unregulated settlement occurs on steep hillslopes (e.g. landslides by Anderson et al., 2014), but also in developed countries. For example, in the city of San Diego, California (USA), soil erosion caused by a storm on January 5, 2016 undermined a 30-foot section of sewer causing failure and prompting a spill of more than 6.7 million gallons of untreated sewage that severely eroded the riverbank and negatively impacted downstream ecosystems (Garrick, 2020). Nevertheless, what is clear is that even though the sample size of events reported in this analysis is small, the severity of the hazards involving WRIFs is high. Housing, transportation, and utilities that serve hundreds of people living in the watershed are impacted by WRIFs in Tijuana, and such conditions are likely to occur throughout the poor urban periphery, contributing to the vulnerability of marginalized populations to environmental hazards. WRIF-based mass movements also contributed a significant amount of sediment to the total watershed load, which negatively impacted habitat and aquatic ecosystems, and further increased downstream infrastructure maintenance costs (Brand et al., 2020). Acknowledging the challenges of monitoring, as addressed here, what becomes clear is a need for more widespread monitoring of landslides and mega-gullies and documentation regarding the role of WRIFs. It is possible that a substantial fraction of the most hazardous mass movement events in cities are linked to WRIFs, and that significant hazard reduction can be realized by addressing WRIFs.

5 Conclusions

Erosional features within a small (11.6 km^2) watershed on the urban periphery of Tijuana, Mexico were monitored to document the frequency and dimension of mega-gullies and landslides, including volumes of sediment ~~volumes~~ generated. Structure from

Motion (SfM) photogrammetric techniques helped to rapidly and safety assess the volume and shape of mega-gullies and
450 landslides. Using imagery collected by either Unmanned Aerial Systems (UASs) or a camera on a hand-held pole, SfM
techniques registered Digital Surface Models (DSMs) with errors of ~3 cm horizontal RMSE and ~5 cm vertical RMSE which
are in line with the needs for sediment budget applications. The methods presented here have the potential to be applied in
other rapidly urbanizing watersheds throughout the world.

~~Over a five-year period with 14 storm events, two mega-gullies and one landslide were observed, each occurring~~
455 ~~during a rainfall event.~~ Over a five-year period with 14 storm events, two mega-gullies and one landslide were observed
during non-extreme rainfall events corresponding to a 1-2-years return period. The landslide damaged more than 20 homes
and put roughly a hundred people at risk, also affecting public transportation and quality of life permanently. SfM analysis
shows the landslide mobilized more sediment ($31,900 \pm 280$ metric tons) than all of the rainfall-based processes combined
over the time scale of the rainfall event. The two mega-gullies interrupted water supplies for a month, and disrupted public
460 transportation and life quality for roughly 6 months. SfM analysis largest mega-gully generated 4,340 tons of sediment, which
is approximately 80 times the area-normalized annual erosion rate for rainfall gullies (tons/ha). While the link between rainfall
and erosion hazards is well known, observations and interviews with residents indicated that all three events were associated
with a Water Resources Infrastructure Failure (WRIF). Mega-gullies occurred after a break in a water supply pipe, which
unleashed a highly erosive, high-velocity water jet onto an erodible hillslope, destroying an unpaved road and interrupting
465 water supply for weeks. Moreover, pipe breaks occurred after rainfall and runoff formed a small gully that undermined shallow
structural support for the water supply pipe. Hence, we conjecture that the observed WRIF-based landslide formation to the
following two-step process:

(1) A water main leak saturated the hillslope creating the preconditions for a landslide.

(2) Rainfall triggered the landslide.

470 and that the WRIF-based mega-gully formation ~~can be characterized by~~to the following a two-step process:

(1) A water supply pipe breaks after a rainfall-generated gully network undermines the pipe.

(2) A mega-gully is formed from the water released from the broken water supply line.

~~The observed WRIF-based landslide formation can be characterized by the following two-step process:~~

475 ~~(1) A water main leak saturated the hillslope creating the preconditions for a landslide.~~

~~(2) Heavy rainfall triggered the landslide.~~

~~Mega-gullies caused by WRIFs were larger than features generated by local rainfall and runoff, produced a significant amount~~
~~of sediment on an event basis, interrupted water supplies for a month, and disrupted public transportation and life quality for~~
~~roughly 6 months. Furthermore, the WRIF-triggered landslide damaged more than 20 homes and put roughly a hundred people~~
480 ~~at risk, also affecting public transportation and quality of life permanently.~~ The limited data suggest that WRIF-based erosion
events occur with an annual frequency of 40-60%, which is far higher than typical design standards for stormwater
infrastructure (5-10% annual exceedance probability). ~~Modeling~~Modelling shows that WRIFs contribute, on average, 5% of

the total annual sediment generation at the watershed scale, and up to 58% on a storm-event basis. Additional research is needed to improve estimates of the spatial and temporal frequency of erosional features caused by WRIFs, and to understand the significance of WRIF hazards at other spatial and temporal scales and in other geographic contexts. Furthermore, the hazards posed by WRIFs within development on steep terrain calls for greater attention to infrastructure design and maintenance. While the sample size of this study is small, the results suggest that poorly maintained water distribution networks on the marginalized urban periphery can be the single most important process generating earth surface hazards, and this finding calls for further investigation into the prevalence of these mechanisms elsewhere. These results also point to opportunities for hazard reduction within urban peripheries through improved planning, design and maintenance of water distribution infrastructure.

Authors contribution

NG undertook data acquisition, processing and interpretation of the data, and prepared the manuscript with contributions from all co-authors. BS, TB, MB and AG designed the research and co-authored the manuscript; EL and YY co-authored the manuscript and assisted with manuscript preparation, and RB provided valuable guidance on the soil erosion modelingmodelling. AH supervised the landslide survey and contributed to the final revision of the manuscript.

Competing interest

The authors declare that they have no conflicts of interest.

Acknowledgements

This study was funded by National Oceanic and Atmospheric Administration Ecological Effects of Sea Level Rise Program (award NA16NOS4780206), and the US Environmental Protection Agency (EPA) (Interagency Agreement ID # DW-12-92390601-0) in collaboration with the U.S. Department of Agriculture (USDA, Agreement 58-6408-4-015), San Diego State University (USA), and the Centro de Investigación Científica y de Educación Superior de Ensenada (CICESE, México). The statements, findings, conclusions, and recommendations are those of the author(s) and do not necessarily reflect the views of NOAA or USEPA. Special thanks to Alejandro Hinojosa, Sergio Arregui, Kristine Taniguchi-Quan, and Belinda Sandoval whose support on field collection and data analysis is gratefully acknowledged. We thank CICESE and Tijuana Metropolitan Planning Institute (IMPLAN) for data sharing. Thanks also to residents of the Los Laureles Canyon, who provided valuable help for data collection.

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