



1 **GEOPHYSICAL AND GEOTECHNICAL FACTORS IN URBAN PLANNING:** 2 BURSA (NİLÜFER, OSMANGAZİ, AND YILDIRIM) CASES 3 Ferhat Özçep^{1*}, Guldane Boyraz², 4 5 Okan Tezel¹, Hakan Alp¹, Nuray Alpaslan³, Savaş Karabulut⁴ 6 7 8 1* Corresponding Author, İstanbul University-Cerrahpasa, Department of 9 Geophysics, Buyukcekmece Istanbul, E mail: ferozcep@istanbul.edu.tr 10 2 Istanbul University, Institute of Graduate Studies, Beyazit Istanbul 11 Batman University, Department of Civil Engineering, Batman Istanbul 3 12 4 TMMOB Jeofizik Mühendisleri Odası, İstanbul Şubesi, İstanbul 13 14 15 ABSTRACT 16 The study area covers the Central of Bursa, Osmangazi, Yildirim, Gürsü, Kestel and Nilüfer 17 18 District boundaries in Bursa. The seismic process deals with the occurrence of an earthquake 19 event and the process of wave propagation from the source to the site. Local amplification 20 caused by surficial soft soils is a significant factor in destructive earthquake motion. In the 21 first phase of this study, it is investigated the ground motion level and soil amplifications for 22 Bursa city. For his aim, probabilistic and deterministic earthquake hazard analysis (including 23 acceleration estimations) will be carried out for the region. Local amplification caused by 24 surficial soft soils is a significant factor in destructive earthquake motion. In the first phase of 25 this study, it is investigated the ground motion level and soil acharacterization for the region. 26 For his aim, probabilistic earthquake hazard analysis (including acceleration estimations) was 27 carried out for the region. Then, soil shear wave velocities were estimated from data obtained by MASW measurements. Soil liquefaction is a natural event in which the strength and 28 29 stiffness of a soil are reduced by earthquake vibrations or other dynamic loadings. As it is known, liquefaction occurs in saturated soils, that is, soils in which the space between 30 31 individual particles is completely filled with water. One of liquefaction evaluation methods is 32 based on the cyclic stress approach. In this method, a safety factor is defined as CRR / CSR. 33 CRR is a cyclic resistance ratio that represents soil liquefaction susceptibility, and CSR is the 34 cyclic stress ratio that represents the earthquake effect. In the second phase of this study, 35 possible soil potential index (PL) and ground induced settlements were estimated by using 36 Isihara ve Yoshimine (1990) approach. All results on liquefaction potential index (P₁), liquefaction induced settlements and soil shear wave velocities in Bursa (Turkey) City were 37





compared with each other. Finally, a seismic microzonation map was prepared by theintegration of geophysical and geotechnical data for urban planning purposes.

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1. INTRODUCTION

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The vast majority of seismic microzonation studies have been carried out in earthquake-prone areas around the world (Marcellini et al., 1982, 1998; Astroza and Monge, 1991; Lasterico and Monge, 1972; Faccioli et al., 1991; Chavez-Garcia and Cuenca, 1998; Lungu and et al., 2000; Faccioli and Pessina, 2001; Fah et al., 1997). In terms of countries, Italy (Marcellini et al., 1998); Spain (Jimenez et al., 2000), Greece (Lachet et al., 1996) and Japan (Abeki et al., 1995) can be given as examples.

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51 The first micro-zoning study in Turkey was conducted by the Construction and Housing 52 Ministry and Earthquake Research Department in 1968. In this study, geological, geophysical 53 and is used as an all geotechnical data and its first land-use planning as reflected in the 54 physical planning of a city or in other words for estimation of local earthquake risk in Turkey. 55 This city is the town of Kuyucak in Aydın province (Gülkan ve Ergünay, 2002, Kozacı ve 56 diğ., 1969). Later, microzonation study was made for the town of Gediz (Kozacı, 1970; 57 Ergünay, 1971; Tabban, 1972, Tokay and Doyuran, 1978). Similar studies were carried out 58 for Adapazarı in 1970, for Izmit in 1974, and cities such as Bolu, Gerede, and Erzincan. Erdik 59 et al. (2000) carried out a microzonation study of İzmir city in the context of earthquake master plan. The first microzonation work with international partners was initiated under the 60 name of "Disaster Prevention / Mitigation Including Istanbul City Seismic Microzonation" 61 62 following the agreement signed between Istanbul Metropolitan Municipality and JICA (Japan 63 International Cooperation Agency). If we take a look at the academic studies carried out 64 within the scope of microzonation in our country, it seems that some articles on this subject 65 were made by Ergünay (1973), Özçep et al. (2003) and Yılmaz et al. (2003).

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The plan is the process to achieve a goal and perform a job with the simplest expression. Planning, on the other hand, is an act of planning and can be defined as a means of rational use of resources, a correct and rational decision-making process for the future to reach the expected goals. The most important goal of today's city/region planning is to provide people





with a healthy and safe life. Regional planning can be defined as multidimensional planning, which is carried out by considering the mutual effects of the natural and artificial physical environment and economic and social activities to achieve the goals adopted by the community in urban areas (Aydemir, 1999, Özçep, 2005; Özçep ve diğ., 2003).

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76 In this study, Multi-Channel Analysis of Surface Wave (MASW) analysis measurements were 77 made at 100 points carried out to obtain shear wave velocities (Vs) in the site-effect 78 investigation. Using MASW, shear wave velocity was obtained at 100 points up to 50 meters 79 deep from the surface. As is known, MASW is derived from the classical seismic search 80 method based on the multi-receiver system lined up along a line. Its main advantage is its 81 ability to recognize different types of seismic waves. This technique is important in obtaining 82 the shear wave velocity (Park et al., 1999; Park et al., 2007). Uniform Building Code, 83 Eurocode, etc). With this method, both active and passive source measurements were made 84 and both measurement results were evaluated together.

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The first task for soil liquefaction analysis is to determine the ground motion level for the selected region. After earthquake hazard analysis for the region is undertaken, in the second stage of the study, soil liquefaction and settlements for the selected region were calculated for various earthquake accelerations and magnitudes.

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2. IMPORTANCE OF GEOTECHNICAL ANF GEOPHYSICAL FACTORS

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One of the first known settlements of the world with its urban character and structure is Çatalhöyük located in Anatolian lands (Shane III and Küçük, 1998). Interestingly, the relationship between this city (where the first urban planning was made) and natural disasters was reflected in the art of the period. A wall painting from 6500 BC in Çatalhöyük can be seen in Figure 1. Here, an active volcano and city plan were drawn together. This picture is probably the oldest known geophysical figure in the world. The active volcano in the picture is thought to be Hasandağı.

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102 The most important goals of today's urban / regional planning and urban transformation103 planning are to provide people with a healthy and safe life. Regional planning can be defined





104 as multi-dimensional planning, which is carried out by considering the mutual effects of the 105 natural and artificial physical environment and economic and social activities to achieve the 106 goals adopted by the community in urban areas. Two different factors play a role in planning 107 studies. The first factor is needed to be taken under control of the big cities emerging as a 108 result of industrialization. The second factor is to develop the underdeveloped regions of a 109 country and to use natural resources that are not evaluated effectively. These are driving force 110 of planning studies (Aydemir, 1999, Özcep, 2005; Özcep et al., 2003; Özcep and Zarif, 2006, Korkmaz and Özçep, 2020, Özçep et al, 2020). 111

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113 114

- Figure 1. A wall painting from 6500 BC in Çatalhöyük (Anatolian Civilizations
 Museum, Ankara)
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119 Vast social changes and dramatic growth of the urban population led to the need for 120 improvement / development of urban areas to meet new demands. In this context, urban 121 transformation and microzonation studies were carried out in many large and small settlements of the country with city or metropolitan status. Urban transformation can be 122 123 defined as multidisciplinary studies created to serve the improvement of unqualified and 124 suburb regions for a legal and healthy life in the border regions of industrial cities receiving 125 immigration (Nalkaya, 2006). Not only regions with historic texture or slum areas but also regions under threat from earthquakes and other natural disasters should be evaluated within 126 127 the scope of these urban transformation studies. It should be noted that while factors such as





- 128 irregular urbanization and natural disasters increase the risk curve (Figure 2), our efforts to
- 129 reduce risk remain insufficient.
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132 Figure 2. Urban Risk-Time Relationship (Wenzel and Bendimerad, 2004)

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135 From a general perspective, risk is defined by Beck (1999) as "human actions caused by the 136 unplanned consequences of radicalized modernization". According to Beck, we are members 137 of the world risk community. Risk is a result of the impact of modern technologies. Risk has globalized and overcomes the power of the national state, and the modern age has entered a 138 new phase called Beck's "second modernity". The "risk society" view is defined as a state of 139 140 radicalized modernity. Beck divides modern society into three periods: pre-industry, industry 141 period and global risk society. The downside of globalization is that economic expansion and 142 technological risks are beyond our control. According to Beck, this should be reacted with a 143 unique form of globalization: Cross-border alliances and global networks should be 144 established to tackle global problems on a global level. Only then can you have the chance to 145 turn "world risk society" into "world citizenship society" as Kant says.

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147 The factors that create the risk society by Beck (1992) are defined as



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149	• After the modern, consumption of nature / society / individual and transform them
150	into risks
151	• As science and technology are out of democratic control
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153	If we take a look at one of the most important natural risks (earthquakes), an average of 700
154	damage-causing earthquakes occur annually around the world. Earthquakes occurring in
155	urban areas are among the most devastating natural disasters.
156	140,000 people died in the 1923 Kanto (Japan) earthquake and 240,000 people in the 1976
157	Tangshan (China) earthquake, and the total losses in the 1995 Kobe (Japan) earthquake
158	exceeded 200 billion dollars. Although it is not yet possible to predict earthquakes, studies in
159	this direction continue using all the possibilities of science and technology. On the other hand,
160	it is possible to reduce the material losses and socio-economic losses that will occur as a result
161	of earthquakes to reasonable levels by taking measures against earthquake damages. In this
162	regard, the city's local administrators have important duties in urban planning, land use and
163	control of buildings.
164	
165	The intense and direct relationships of local administrators with the people of the city make it
166	possible to transfer information about earthquake damage reduction and to raise the awareness
167	of the public against earthquakes in general (Erdik et al., 2000). Düzgün (2006) proposed a
168	model to determine the spatial vulnerability of an urban area to the earthquake disaster in
169	terms of various factors (accessibility to structure, soil, social, economic, administrative and
170	critical services) using a selected city area as a pilot application area (Figure 3).





172 Earthquake Hazard Soil Hazard Structural Vulnerability Socio-Economic Emergency Accessibility Fragility Integrated Risk Model 3D City Model Administration **Spatial Decision Systems** and **Scenario Analyses** 173 174 175 Figure 3. Spatial vulnerability model in terms of various factors (accessibility to structure, 176 earthquake, soils, social, economic, administrative and critical services) to the earthquake disaster of the urban area using a selected urban area as a pilot 177 178 application area (Düzgün, 2006). 179 180 181 Necessary items to estimate the effects of earthquakes in urban areas are historical earthquake 182 information, geological, geophysical and geotechnical data, probabilistic or deterministic 183 evaluation of earthquake hazard, determination of ground motion according to soil conditions 184 and preparation of these studies in the context of microzonation studies. 185 186 Urban planning became prominent as an important study in expanding urban areas as a result

of population growth. The balance between human settlements and the natural environment is disrupted by urbanization (De Mulder, 1996). Urban planning practices must reduce these conflicts / imbalances to improve the general well-being and quality of life of society (Bell,





190 1998; Bell et al., 1987). Such planning must be in a multi-disciplinary approach for various
191 human needs (De Mulder, 1996). However, geological, geophysical and geotechnical data in
192 urban planning have become increasingly important in identifying, controlling and mitigating

193 natural disasters (Bell et al., 1987; Legget, 1987; Hake, 1987).

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195 Many earthquakes in the past and present have shown that local soil conditions (including 196 topography effects) have a major impact on damage distribution. When earthquake hazard is 197 evaluated on a regional or local scale, it is very important to identify and take into account 198 these possible local site effects. Seismic microzonation has been an economically useful 199 element of the earthquake risk reduction process (Roca and Oliviera, 2001). Microzonation 200 against earthquake hazards has been defined as the division of a geographical region into 201 small areas according to the behavior of the soils underground motion or slope stability 202 (Hays, 1980; Sharma and Kovacs, 1980)

Nigg (1982) stated that microzonation aims to divide the risky areas into small pieces in order to implement the right plans and policies that can minimize the damage that may occur after the earthquake. Finn (1991) has defined microzonation as procedures involving the development of measures for seismic hazards for building design, taking into account local soil conditions.

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In 1993, a guideline study describing the principles was made on microzonation for "soil amplification", "slope stability" and "liquefaction" by the Earthquake Geotechnical
Engineering Committee of the International Soil Mechanics and Foundation Engineering"
(later "Geotechnical Engineering") Union (ISSMFE, 1993). (ISSMFE, 1993). This work was
revised in 1999 (ISSMGE / TC4 1999).

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During earthquakes, seismic waves, especially under the effect of shear waves, spread in loose soils that are generally saturated and without drainage, creating shear forces relative to each other, causing soil particles to displace. Under these conditions, saturated and loose soil particles tend to converge on each other. The stress at the contact points of the particles in this case is transmitted to the surrounding water. Since the seismic waves cause sudden and very short movements during the earthquake, it does not allow sufficient time for the water between the particles to drain. Therefore, the pressure of pore water, which cannot move away





- from the environment, increases suddenly. This sudden increase in pore water removes the contact forces that hold the soil particles together and removes the particles from each other. Thus, the soil loses its strength. Under these conditions, where the effective vertical pressure is zero, the soil acts as a liquid instead of a solid. This behaviour, which the soil presents as a result of dynamic loads, is known as liquefaction.
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Both soil amplification and soil liquefaction developing on the soils due to earthquake effectsare critical studies for microzonation studies of urban areas.

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3. GEOLOGY TECTONICS OF THE REGION

234 The study area covers the central districts of Bursa province. These districts are connected to 235 Bursa Metropolitan Municipality; Osmangazi, Kestel, Yıldırım, and Kestel districts. 236 Regarding the historical earthquake records, the district has a remarkable feature as it is located in an area with high earthquake potential. The geology of the study area was prepared 237 238 by Zetas (2000). It is located in an ellipsoidal basin, known as the Bursa basin, which started 239 its formation in the Neogene and continued during the Quaternary period and is in the 240 direction of D-B. This basin is one of Turkey's major tectonic structures, formed in the effect 241 of the North Anatolian Fault Zone. The main faults surrounding the study area are given in 242 Figure 4.







245	Figure 4. Main faults surrounding the study area (Le Pichon et al., 2001, Redrawn)
246 247	
248	Karakaya group rocks outcrop in the south and southeast of the study area, where the old-
249	based (paleozoic) relationship cannot be seen. The conglomerate, micro conglomerate,
250	sandstone, feldspar sandstone, micaceous sandstone, grovak, claystone, basic volcanic and
251	limestone alternation are observed. It also contains a large amount of limestone (Permian)
252	olistoliths in various sizes. It contains radiolarite, diabase and spilitic intermediate (Zetaş,
253	2000). Alluvium: Bursa plain, which is a tectonic depression basin, is completely carried by
254	rivers; blocks with detritic material such as gravel, sand, and silt. Alluvium thickness varies
255	between 80m-200m. In the central part of the Bursa plain, the alluvium thickness varies
256	between 140m-200m. It consists of clay with fine elements, silt, sand, and gravel. It is
257	difficult to separate alluvium and Neogene units (Zetaş, 2000).
258	
259	In order to explain the current features of the Marmara region, it is necessary to mention plate
260	tectonic movement of Turkey and the surrounding area. Turkey is located on a mountain
261	formation system we call the Alpine-Himalayan Belt (Ketin, 1977). Crustal movement occurs
262	in the Turkish plate due to the young and active tectonics as a result of (2.0 - 2.5 cm)
263	northward of the Arabian Peninsula.
264	
265 266 267	4. GEOTECHNICAL AND GEOPHYSICAL ANALYSES FOR REGION
268	For geophysical and geotechnical characterization, the view of the measurement points taken
269	within the boundaries of the district is given in Figure 5a and b.







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Figure 5a. Location of the study area Geophysical (MASW) measurement points at
 Osmangazi ve Yıldırım

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- 280
- 281 4.1. Geophysical Factors

measurement points at Nilüfer District

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There are several geophysical measurements and evaluations used in field or site investigations. In this context, geophysical measurements can be divided into two groups as

Figure 5b. Location of the study area and boreholes and the Geophysical (MASW)





exploration geophysics methods and engineering seismology studies. The most important
exploration geophysics methods are seismic methods that are the main method to estimate Vs
velocity. Engineering eeismology studies include evaluating probalistic and deterministic
seismic hazards to estimate the ground motion level.

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292 4.1.1. Seismic Hazard Analysis of the Region

Turkey is a region where there are interactions of Arab-Africa plates in the south and Eurasian 295 296 plate in the North. Many important fault zones are effective within Turkey. The closest fault 297 zone that will affect the study area is the North Anatolian fault zone. The approximate length 298 of this fault zone is around 1200 km. In recent years, the city of Erzincan in eastern Turkey 299 has seen very severe earthquakes (1939 and 1992). While more than 30.000 people died in the 300 1939 earthquake, 700 people died in 1992. With the collapse of several buildings and 301 infrastructure, a high rate of property loss was observed. On August 17, 1999, an earthquake 302 disaster called Kocaeli earthquake occurred around Izmit and Adapazari, 110 km east of 303 Istanbul. This earthquake, the magnitude of which was estimated to be between 7.4 and 7.6 by 304 various researchers, damaged human life and property at a very high rate in the region. 305 Another magnitude 7.2 earthquake occurred on November 12, 1999, in the northern Anatolian 306 fault zone. Over a thousand people were dead or seriously injured. There are some estimations 307 that the centres of these strong earthquakes are shifting from east to west along the North 308 Anatolian Fault zone and draw attention to the possibility that a major earthquake could shake 309 Istanbul, located at the western end of the North Anatolian Fault.

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311 Probabilistic Earthquake Hazard Analysis for the region

The probabilistic earthquake hazard is defined as the probability that the damaging ground motion will occur in a certain place and within a certain period of time. For the analysis, it was obtained 4.5 and larger earthquakes during the instrumental period within an area of 100 km radius, focusing on the region from a database of Kandilli Observatory and Earthquake Research Institute. Earthquake hazard analysis was carried out using "SoilEngineering" software developed by Ozcep (2010). The results of this analysis are given in Figure 6 and in Table 1.a, 1.b, 1.c.







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7.6

4.7

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30.0

37.9





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334 (%)and design year

0	2	5
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D (year)	Probability of Exceedance (%)	M (magnitude)
50	30	7,4

∆, Epicentral Distance (km)	H, Focal depth (km)
25	15

Researcher	Donavan(197 3c)	Oliviera (1974)	Joyner ve Boore (1981)	Campbell (1997)	Average
Acceleration					
(a)	0.23	0.16	0.39	0.42	0.30

342 343

344 Geophysical Analysis based on Field Studies

Within the coverage of this study, geophysical study (surface wave analysis (MASW) and 345 346 shear wave velocity estimation) was carried out at 100 different points in the districts of Bursa 347 Metropolitan Municipality. The average shear wave velocity (Vs30) map for the 30 meters obtained as a result of the measurement is given in Figure 7 for Osmangazi district and its 348 349 surroundings (Region 1) and Figure 8. (for Nilüfer District (Region 2), soil fundamental 350 period and soil amplification maps were given in Figure 9a, b, c, d, e. Some locations of the 351 study area are not suitable to acquired geophysical data especially central parts of Osmangazi 352 district and its surrounding area.







353

354Figure 7. Bursa (Osmangazi, Yildirim, Gürsü, Kestel) Districts and VS30 map of Soils and355measurement locations (black dots with numbers) acquired on the Osmangazi district.

356 357

357 It was observed there were three different velocity zones from the Vs30 distribution map of Osmangazi district and its surroundings (Fig. 7). On the basis of Vs 30 values of the study 358 area can be classified as high (380m/sn - 450m/sn), medium (260-380 m/sn-) and lower (< 359 360 S wave values related to regional topography. While lower values correspond to the 260). 361 basin, high and medium S wave values related to hill zones. The eastern part of Region 1, located in the part of the Nilüfer District, and in a narrow zone in the south of Region 1 have 362 363 medium and higher shear - wave velocity values within the range 260m/sn - 440m/sn. The Vs 30 values are then decreasing to the west and to the north towards the central of Region 1. 364 The Vs30 map in Region 1 shows very good agreement that low velocities in the centeral part 365 366 correspond to alluvium in Bursa Plain.

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369 I km
370 Figure 8. Bursa (Nilüfer) District and VS30 map of Soils

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The Vs 30 distribution map of Region 2 shows three different velocity zones as Region 1. In contradistinction to, while higher velocity was obtained, ranging from 480-600m/sn , medium velocity values within range 340-480m/sn. In this region, only two local areas have high Vs velocities. While One of these local areas is the northern part of Region 2 that has 580m/sn shear velocity maximum, the other is the center of the region towards the east that has a maximum of 600m/sn higher Vs30 values. The lower Vs 30 velocities are calculated in all other areas. Lower values within range 160-280m/sn.







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383 Figure 9a. Soil Fundamental Period Map in Nilüfer (Bursa) District

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The contour map of the Soil Fundamental Period Map in Nilüfer (Bursa) District is computed from 385 386 MASW and microtremor measurements. The fundamental period values of the sites located within 387 areas with low Vs 30 seismic velocities are generally higher than areas with higher Vs 30 values. The 388 soil fundamental period map in Nilüfer (Bursa) District, especially the lowest fundamental period 389 values zones, coincides with the previous results of the Vs 30 distribution map. In the northern part of the area, the fundamental periods are computed between 0.4 - 0.7 sn, whereas at the sites in the 390 391 allüvial basin, the values increase to 1.0 -1.5sn. In addition, low fundamental periods of 0.4.- 0.7sn are 392 also observed in the eastern part of the area.

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8 Figure 9b. Soil amplification maps accordin to Midorikawa (1987) Approach

399 in Nilüfer (Bursa) District





401 402 Figure 9c. Soil amplification maps according to Joyner and Funal (1984) Approach in Nilüfer

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^{403 (}Bursa) District

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- 410 Figure 9d. Soil amplification maps according to Borcherdt et al (1991) approach for strong



417 Figure 9e. Soil amplification maps according to Borcherdt et al (1991) approach for weak

418 motion in Nilüfer (Bursa) District

motion in Nilüfer (Bursa) District





420	Figure 9d and e show comparisons between the soil amplification map according to various
421	approaches (Joyner and Fumal (1984) ; Midorikawa (1987); Borcherdt et al (1991) for strong
422	and weak motion) . According to the map obtained from Joyner and Furnal's (1984) approach
423	(Fig. 9d), while high soil amplification values are observed at the western and central part of
424	the study area, low values are observed at the northern and eastern part of the area. This
425	approach is very well compatible with the Vs 30 map and the fundamental period map. The
426	high values calculated from other approaches also coincides with previous results of
427	geophysical investigations. If the approaches are compared to each other, maximum soil
428	amplification values are calculated from Borcherdt et al (1991) approach for weak motion
429	with these values ranging from 4.4 -3.0.
430	
431	In other approaches, these maximum values within range 2.0- 3.7. According to Joyner and
432	Furnal's (1984) approaches, soil amplification values are computed within the range of 1.3 -
433	2.35. These values are the minimum values in all approaches.
434	
435 436 437	Soil-Earthquake Interaction Maps
438	Soil-earthquake interactive acceleration maps were made by taking into account Vs30 values obtained
439	by surface wave velocity (MASW) analysis in Nilüfer District. The measurement points on the Vs30
440	map are divided into 3 regions considering that the NAF zone passes through the north. The shortest
441	perpendicular distance to the measurement points in the northern part is 15 km, the measurement
442	points in the central part are 20 km and the measurement points in the south are 25 km and the size
443	chosen for the design is M: 7.6. Boore et al. (1997) spectral acceleration map was made with
444	approach. Besides, mean acceleration map and spectral acceleration maps were made for periods of
445	0.0-0.2 and 1.0 seconds. These are given in Figure 9 f,g, and h.

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448

- 449 Figure 9 f. Spectral Acceleration Map with soil-earthquake interaction for 0.0 sec period in Nilüfer
- 450 (Bursa) District
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453 Figure 9 g. Spectral Acceleration Map with soil-earthquake interaction for 0.2 sec period in Nilüfer

- 454 (Bursa) District
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459 Figure 9 h. Spectral Acceleration Map with soil-earthquake interaction for 1.0 sec period in Nilüfer460 (Bursa) District

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462 **4.2. GEOTECHNICAL FACTORS**

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The shear strength of a soil is the highest shear stress that can be applied to the soil mass. The shear stresses that cause the shear stress along the shearing plane are the highest shear stress that the soil can bear, hence the shear strength. Accordingly, the shear strength is the limit value. The shear strength is the value that corresponds to the plastic equilibrium state, that is, it has no return and is a failure condition. Shear strength on the soil is formed by two properties of the soil:

470 (1) Internal Friction Angle (φ)

471 (2) Cohesion (c)

472 The important shear strength parameter of the soil is φ . Because it is a parameter that covers 473 the behaviour for general and all soils compared to c. Friction is caused by the soil grains that 474 are in contact with each other and the stress transfer between each other. The soil is a three-475 phase environment. So, soils are the three components of water, air, soil particles. In the soil





environment where these three components are mixed, the ratios of these components relativeto each other are effective on soil behavior.

478 As geotechnical data for this study, over 200 boreholes (geotechnical) were provided from 479 Bursa City Municipality. From this data, sand and silty sand units were extracted. Soil liquefaction analysis was performed based on geotechnical data. For this analysis, 480 481 acceleration values (a: 0.40-0.45) and magnitude (M: 7.0-7.5) were selected. As a result, 482 liquefaction analysis and liquefaction settlement analysis were estimated. Liquefaction Potential Index (PL) map is given according to several accelerations and magnitudes for 483 484 Bursa (Osmangazi, Yıldırım, , Gürsü, Kestel) districts in Figure 10a and 10b and a map of 485 settlements related to liquefaction, for Bursa (Osmangazi, Yıldırım, , Gürsü, Kestel) city (a: 0.40-M: 7.0) is given in Figure 10c. For Nilufer district, the same maps were given in Figure 486 487 11a, b,c, d and e. The map of groundwater level for Nilufer district was given in Figure 12.

















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- 503 Figure 10c. Map of settlements related to liquefaction, for Bursa (Osmangazi, Yıldırım,
- 504 Gürsü, Kestel) city (a: 0.40-M: 7.0).

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Figure 11a Liquefaction Potential Index (PL) map according to acceleration and magnitude (a:
 0.45-M: 7.6) in Nilufer (Bursa) District.



Figure 11b Liquefaction Potential Index (PL) map according to acceleration and magnitude (a:
 0.40-M: 7.4) in Nilufer (Bursa) District.







Figure 11c Liquefaction Potential Index (PL) map according to acceleration and magnitude (a:
 0.40-M: 7.2) in Nilufer (Bursa) District.



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Figure 11d Liquefaction Potential Index (PL) map according to acceleration and magnitude (a:
0.35 M: 7.0) in Nilufer (Bursa) District.

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- 520







522 Figure 11f. Map of settlements related to liquefaction, in Nilufer (Bursa) District



524 Figure 12. Map of Ground water level for Nilufer (Bursa) District





- 526 Vs15 values was calculated from SPT Data and comparisons of real (MASW) geophysical
- 527 data. This and comparisons were given in Figure 13a and b.



529 Figure 13 a. Vs15 values, estimated from SPT Data in Nilufer (Bursa) District







Figure 13b. Vs15 values, measured from Geophysical (MASW) Data in Nilufer (Bursa)
District

533 4.4. Preparation of Seismic-Micro-Zoning Map for Urban Planning

As a result of the geophysical and geotechnical studies, a microzonation map was prepared for a basis for urban planning (Figure 14a and b). As a result of the calculations, the hazard level for Bursa city is classified into 3 categories.







538 Figure 14a Microzonation map for a basis for urban planning in Bursa(Nillüfer) District



- 540 Figure 14b Microzonation map for a basis for urban planning in Bursa (Osmangaz, Yıldırım,
- 541 Gürsü, Kestel) Districts





542 543 544 **5. RESULTS** 545 546 Geophysical measurements were made to estimate the shear wave velocity at 150 points in the 547 548 study area by using the MASW method. The shear wave velocity map was prepared for the 549 region. When we look at these maps, it was observed that Vs30, the type C type soils 550 (According to NEERP) were dominant and the other parts were represented by the D-E type 551 floors. 552 553 Earthquake hazard was determined probabilistically by using the Poison approach for the 554 study area. In the probabilistic analysis, the magnitude of the project earthquake has been 555 calculated for a certain time period for a certain exceeding rate. In the second phase of the study, soil liquefaction and possible settlements due to liquefaction were estimated and 556 mapped by using geotechnical data. In the last phase, a final seismic microzonation map has 557 been created. In this study, the main purpose is to reveal the use of geophysical and 558 559 geotechnical data together in the context of microzonation. 560 In Bursa city, the levels of hazard for the problems that may occur interactively with the soils 561 were determined in the event of an earthquake. In new studies to be carried out in the context 562 of urban planning, urban transformation, and urban renewal, these hazard levels must be taken 563 564 into consideration. 565 566 6. Acknowledgement 567 568 This study was supported reseach Unit of Istanbul Universitesi (Research Grant Number: 569 YADOP-6708 and Thesis Project number: 5582) 570 571 572 6. References 573 574 575 576 • Abeki, A., Matsuda, I., Enomoto, T., Shigyo, V., Watanabe, K., Tanzawa, Y., 577 Nakajima, Y., 1995, A study of seismic microzonation based on the dynamic





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