

**Response to Reviewers' comments on "*A model for interpreting the deformation mechanism of reservoir landslides in the Three Gorges Reservoir area, China*" (nhess-2019-432)**

Dear Editor and Reviewers,

Thank you for editor's efforts on dealing our manuscript and reviewer's constructive comments on this manuscript. We have studied these comments carefully and made point-by-point corrections, which have enabled us to improve the manuscript. Now we present point-by-point response to reviewer's comments, followed by the revised manuscript. The revised portions are marked in RED in new manuscript (MS).

Below we list every comment received (in *italics*), followed by our response in regular font.

**Response to Reviewer 1's comments**

**General comment**

1. *As a whole, the manuscript is valuable and presents robust data for publication. However, some parts of the manuscript are completely useless and uncorrect from a theoretical point of view, while some other parts require modifications. Therefore, this reviewer suggests a strong re-structuring of the manuscript as well as an improvement of the parts that need corrections. English is generally fine and no significant typing errors have been detected.*

**Response:** Thanks for reviewer's comments and suggestions.

2. *In the introduction section, the authors should better describe, from a theoretical point of view, the problem of rapid drawdown and rainfall infiltration in the landslide equilibrium, and in particular the role of permeability of the landslide soils and the rate of drawdown. Is this problem related to the type of the soils involved or not?*

**Response:** Yes, the problem is related to the type of the soils; landslides with lower permeability are more susceptible to be affected by the drawdown. We now add content as reviewer suggested to describe the effect of rapid drawdown and landslide permeability on landslide stability (new Lines: 61-65).

3. *In the driving-locking model (Section 2), the authors do not completely account for the general equilibrium of the landslide mass, since they reduce all the equilibrium condition to the single unit vertical slice without considering the inter-slice forces, which do have a role in the equilibrium of the single slice. This is uncorrect, since it affects the location of the locking section. All this section, and the equations here*

*proposed, seems to be a neglect of the slice methods historically proposed in the limit equilibrium approach and in general of the equilibrium theory (the problem being undetermined from a statical point of view and the need of integrative equations to balance unknowns- equations. . .). Moreover, in the limit equilibrium analysis proposed by the authors in the following sections, they use the Morgenstern-Price method, which is a well-known rigorous method and of course takes into account the inter-slice forces. Therefore, the first part of the manuscript is not in agreement with the approach followed in the second part.*

**Response:** Yes, the limit equilibrium method has developed from simplified limit equilibrium methods to rigorous limit equilibrium (LE) methods, and we agree with reviewer that a rigorous LE method would give more precise result about the location of the locking section. But we still choose the simplified limit equilibrium method for analysis here for following reasons. The resisting section is defined as the lower-front part of the slide mass, where each unit vertical slice (Fig. 3) can be self-stabilized under its self-weight. In the unit vertical slice of locking section, the difference between the forces on the two vertical sides is very tiny because the width of the unit vertical slice is very small, and the slide surface underlying the lower-front part of the slide mass is relatively gentle; so the interslice forces were ignored for convenience of analysis. Moreover, this results have been effectively adopted to interpret the deformation process of Shuping landslide. So we still preserve the Section 2.

In the Section 5, the rigorous limit equilibrium method (M-P method) is employed to analyze the Shuping landslide, which is not consistent with that used in the Section 2. Because we want to use rigorous LE to check the results from the simplified LE method used in the Section 2.

To address this comment, we added an explanation to clarify why we choose the simplified LE method in the section 2 (on Lines: 124-127), and we also added a discussion in the discussion section to point out the limitation of simplified method (on Lines: 501-509). We hope we can get you your agreement and permission for dealing with this issue.

**4. *The distinction between driving section and locking section (I would suggest “resisting section” rather than “locking”, if necessary) is not rigorous and can have only a qualitative meaning. Even in the driving section, there is some mobilised strength component along the corresponding portion of the sliding surface, as well as even in the locking section the driving forces, in some circumstances, can prevail over the resisting ones.***

**Response:** We agree with reviewer’s opinion. We now change the term “locking section” into “resisting section” in the whole manuscript as suggested.

**Specific comments:**

1. *In the figures proposed the term “deformations” is used to indicate displacements, which have mm as measurement unit. Please, use the term “displacements”.*

**Response:** Thanks for catching this error. “deformation” was changed to “displacement” in the new MS (see Fig.10, Fig.11, Fig.12).

2. *The comment presented at lines 456-461 is questionable, since a displacement of 5 m is not so large to justify a change in the landslide body geometry, especially for a landslide size as that here examined. Apart from the change in the curve trends, a limit equilibrium analysis with the post-movement landslide geometry should be performed to verify the actual change in the factor of safety.*

**Response:** The accurate calculation of the safety factor of the landslide with the change of the landslide body geometry is unavailable here, because the dynamic movement landslide geometry is difficult to be obtained. To address this comment, we removed this questionable content.

3. *The cohesion value adopted for the sliding surface should be justified more in detail. The landslide is moving and has experienced quite a large displacement; therefore, probably the cohesion value proposed is not operative anymore and, in general, post-failure strength conditions would apply in this situation. A comment from the authors on this choice is necessary.*

**Response:** We agree with reviewer’s opinion. Shuping landslide is a reactivated landslide and had experienced large deformation before the reservoir impoundment; therefore the post-failure strength was applied in the calculation in this study.

4. *A more detailed description of the engineering treatment performed in the slope is necessary. It is mentioned, but not described.*

**Response:** Thanks for reminding. We presented the detailed description of the engineering treatment in the Section 6 (on Lines: 486-492).

5. *Since a transient seepage analysis is carried out, the authors should describe also some more data on the hydraulic properties of the soils used in the seepage calculations, as required by the software code used (retention curves, permeability coefficient variation with suctions).*

**Response:** Thanks for reminding. We added the necessary hydraulic properties in Tab 1.

6. *Line 338: what does it exactly mean “rainfall threshold” as expressed in terms of rainfall intensity? Being clay materials, rainfall data in terms of long-term cumulative rainfalls should be more important than rainfall intensity.*

**Response:** Yes, the “rainfall threshold” is expressed in the terms of the monthly rainfall

here, which represents monthly cumulative rainfall.

**7. Dam impoundment has also an external loading (i.e.stabilizing) function on the landslide equilibrium. The external impoundment load affects the overall equilibrium of the landslide body. This is never mentioned by the authors.**

**Response:** The external impoundment load affect has been considered within the SLOPE/W module of GEOSTUDIO software. To address this comment, we mentioned this factor in the new MS (on Lines: 366-367)

**8. Since the authors explain the change in the equilibrium conditions of the landslide in terms of seepage forces (inward or outward, with respect to the slope), they should plot the output of the seepage analysis in terms of flow vectors (during a drawdown stage and an impoundment stage, for example) in order to corroborate their comments.**

**Response:** Thanks for suggestion. We now add representative flow vectors figures (see added figures: Fig.15B, Fig.16B, Fig.18) for seepage analysis.

**9. How is chosen the location of the section dividing the driving and locking portions based on the results of the analyses proposed?**

**Response:** We analyzed this issue in Section 2.2, and the conclusion is that the boundary between the locking and driving sections can be approximated as the position where the slope angle  $\theta_1$  equals the internal friction angle  $\varphi$  ( on Lines 149-157).

## **Response to Reviewer 2's comments**

### **General comment**

*The subject manuscript, "A model for interpreting the deformation mechanism of reservoir landslides in the Three Gorges Reservoir area, China" is an important case study of a large, deep landslide that has been affected by reservoir impoundment and fluctuations. The manuscript is logically organized, well written and presents a long record of data relating landslide movement, reservoir levels, and precipitation.*

**Response:** Thanks for reviewer's kind comments.

### **Specific comments:**

*My primary criticism of the paper is that the authors seem to be unaware of previous studies that have presented similar, closely related models to that presented in sections 2.2 and 2.3. Although most previous work cited in the following lines does not specifically address reservoir effects on landslides, the relationships between landslide geometry, deformation, dynamics, and stability identified in previous studies is*

*relevant to the case presented in the subject manuscript. The model has concepts in common with the wedge method for analyzing landslides consisting of an active driving wedge and resisting block (Terzaghi & Peck, 1967; Sultan and Seed, 1967). Hutchinson (1984) presented an "influence-line" approach for assessing effectiveness of cuts and fills in stabilizing slopes, which is also similar to the models in sections 2.2 and 2.3. Iverson (1986) described relationships between stress distribution and landslide geometry. Baum and Fleming (1991) described the relationship between displacement patterns and the results of stability analysis, and derived expressions for the boundary between driving and resisting elements of landslides. Interestingly, they concluded that the boundary is near the thickest part of the landslide, consistent with the findings of this manuscript. Drawing on insights gained from these earlier studies, McKean and Roering (2004), Guerriero et al. (2014), Prokesova et al. (2014), and Handwerger et al. (2015) as well as others, have further explored the influence of slip-surface and landslide geometry on slide deformation, force distribution and landslide dynamics.*

*In addition to strengthening the background section/literature review to show the relationship of the authors' model to previous work.*

**References cited:**

- ✓ *Baum, R.L., Fleming, R.W., 1991. Use of longitudinal strain in identifying driving and resisting elements of landslides. Geol. Soc. Am. Bull. 103, 1121–1132.*
- ✓ *Guerriero, L., Coe, J.A., Revellino, P., Grelle, G., Pinto, F., and Guadagno, F.M., 2014, Influence of slip-surface geometry on earth-flow deformation, Montaguto earth flow, southern Italy: Geomorphology, v. 219, p. 285-305. <http://dx.doi.org/10.1016/j.geomorph.2014.04.039>*
- ✓ *Handwerger, A.L., Roering, J., Schmidt, D.A., and Rempel, A.W., 2015, Kinematics of earthflows in the Northern California Coast Ranges using satellite interferometry: Geomorphology v. 246, p.321–333.*
- ✓ *Hutchinson, J.N., 1984, An influence line approach to the stabilization of slopes by cuts and fills: Canadian Geotechnical Journal, v. 21, p. 363-370.*
- ✓ *Iverson, R.M., 1986. Unsteady, nonuniform landslide motion: 2. Linearized theory and the kinematics of transient response. J. Geol. 349–364.*
- ✓ *McKean, J. and Roering, J. 2004, Objective landslide detection and surface morphology mapping using high-resolution airborne laser altimetry: Geomorphology 57 (2004) 331–351*
- ✓ *Prokešová, R., Kardoš, M., Tábork, P., Medvedová, A., Stacke, V., Chudy, F., 2014. Kinematic behaviour of a large earthflow defined by surface*

*displacement monitoring, dem differencing, and ert imaging. Geomorphology 224, 86–101.*

- ✓ *Sultan, H.A., and Seed, H.B., 1967, Stability of sloping core earth dams: American Society of Civil Engineers Proceedings, Journal of the Soil Mechanics and Foundations Division, V. 93, no. SM4, p. 45-68.*
- ✓ *Terzaghi, K. and Peck, R.B., 1967, Soil mechanics in engineering practice (2nd ed.): New York, Wiley, 729 p.*

**Response:** Many thanks for reviewer providing these valuable references. We now add a background section to review these references and address the relationship between our work and the previous work (on Lines: 71-79).

Thanks again for editor's and reviewer's effort on our manuscript!

Best regards,

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