



## Three-dimensional numerical simulation of mud flow from a tailings dam failure across complex terrain

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**Abstract.** A tailings dam accident can cause serious ecological disaster and property loss.  
10 Simulation of a tailings dam accident in advance is useful for understanding the tailings flow  
characteristics and assessing the possible extension of the impact area. In this paper, a  
three-dimensional (3-D) computational fluid dynamics (CFD) approach was proposed for  
reasonably and quickly predicting the flow routing and impact area of mud flow from a dam  
15 failure across 3-D terrain. The Navier–Stokes equations and the Bingham-Papanastasiou  
rheology model were employed as the governing equations and the constitutive model,  
respectively, and solved numerically in the finite volume method (FVM) scheme. The  
volume of fluid (VOF) method was used to track the interface between the tailings and air.  
20 The accuracy of the CFD model and the chosen numerical algorithm were validated using  
an analytical solution of the channel flow problem and a laboratory experiment on the dam  
break problem reported in the literature. In each issue, the obtained results were very close  
to the analytical solutions or experimental values. The proposed approach was then applied  
to simulate two scenarios of tailings dam failures, one of which was the Feijão tailings dam  
25 that failed on January 25, 2019, and the simulated routing coincided well with the in situ  
investigation. Therefore, the proposed approach does well in simulating the flow  
phenomenon of tailings after a dam break, and the numerical results can be used for early  
warning of disasters and emergency response.

### 1. Introduction

A tailings dam is one of the three basic projects at a mine, and it is a dangerous source of artificial mud flow  
with high potential energy. The stability of tailings dams is highly susceptible to natural and human-induced  
30 adverse factors. According to the International-Commission-on-Large-Dams (ICOLD) (Dams, 2001), the  
accident rate of tailings dams in the past 100 years (1900-2000) was approximately 1.2%, which is two orders of  
magnitude higher than the accident rate of reservoirs (0.01%). Since the beginning of the 20th century, there  
have been hundreds of tailings dam accidents in the world (Rico et al., 2008b). In 2008, the Xiangfen tailings  
dam collapsed in China, causing 381 deaths. In 2015, the Fundão mine tailings dam failure in Brazil released 32  
35 million  $m^3$  of toxic tailings, polluted 650 km of rivers and flowed into the Atlantic Ocean, causing serious  
environmental consequences (Burritt and Christ, 2018). Four years later, more than 232 people died in the

failure of the Feijão tailings dam, which belongs to the same company as the Fundão mine (Santamarina et al., 2019).

40 To date, the number of tailings dams in the world exceeds  $10^5$ , and ensuring the stability of such reservoirs is  
an extremely challenging task for the mining industry. By documenting 147 historical cases of tailings dam  
failure worldwide, Rico et al. (2008b) found that the most common causes of failure are related to unusual  
rain and seismic liquefaction. Furthermore, dozens of factors, such as management operations, slope  
instability, fluvial undermining, and foundation failure, are also likely to cause dam failures. As a  
45 consequence, even if all the tailings dams were constructed correctly, the occurrence of pond failure cannot be  
avoided.

Dam-break simulation and run-out analyses can provide useful information for assessing the risk associated  
with tailings dam collapses, such as run-out path and quantification of potential losses, which have become  
50 more important in geotechnical assessment of tailings dams. Nevertheless, little work has been conducted to  
analyze the dam-break process and investigate the flow of tailings released following dam failure. These  
related dam-break analyses can be divided into empirical models and numerical models. The empirical model  
predicts the volume of released tailings ( $V_F$ ) and the run-out distance ( $D_{max}$ ) by establishing a regression  
equation based on past failure, dam height, and the impounded volume of tailings (Rico et al., 2008a; Larrauri  
55 and Lall, 2018). However, owing to the difference in the actual state of the tailings reservoir and the  
surrounding environment, and without considering the rheological properties of the tailings such as the  
viscosity coefficient and yield stress (Martin et al., 2015), the predicted results may deviate greatly from the  
actual dam-break situation. Additionally, the flow pattern and movement regularity of tailings fluid cannot be  
accurately described.

60 Although numerical modeling based on computational fluid dynamics (CFD) has the advantages of  
sophisticated theory and low cost, it is considered to be a more appropriate method for analyzing geographic  
hazards such as water reservoir failures, landslides and debris flows (Dai et al., 2014; Han et al.,  
2019; Issakhov and Imanberdiyeva, 2019). Using CFD to study and analyze a tailings run-out path is rare.  
65 Unlike water, which is a Newtonian fluid, the tailings fluid formed by mixing tailings and water is in nature a  
non-Newtonian fluid (Henriquez and Simms, 2009) with complex rheological properties, and it is hard to  
define the boundary of large-scale complex downstream terrains. All of these lead to the fact that tailings  
run-out routing is considered difficult to analyze and predict by numerical modeling. Some initial work, such  
as laboratory experiments and relatively simple deposition simulations of tailings fluid, has been attempted  
70 (Mizani et al., 2013; Zhang et al., 2015; Babaoglu and Simms, 2017; Gao and Fourie, 2019; Luo et al., 2017).  
These studies discussed the flow and rheological properties of tailings fluids in detail but did not take into  
account the influence of real terrain on the movement distance of tailings fluids. Wang (Wang et al., 2018)  
integrated the smooth particle hydrodynamics (SPH) and the 30-meter resolution digital elevation model  
(DEM) of the ALOS satellite to simulate the dam-break process of a tailings reservoir, while the complex  
75 rheological properties of the tailings fluids were not mentioned at all.

In this paper, a three-dimensional (3-D) model (based on OpenFOAM, which is an open-source CFD toolkit) for simulating mud flow from a tailings dam break across 3-D terrain was presented to provide quantitative information for risk assessment and disaster prevention, and the accuracy of the model was tested using  
 80 small-scale laboratory experiments of a dam-break type flow. Aiming at the difficulty of accurately reproducing large-scale downstream terrain, unmanned aerial vehicle (UAV) aerial photogrammetry technology was used to obtain fine DEM data. Furthermore, the computations of the model were efficiently executed on the supercomputer cluster of the Fujian high-performance computing (HPC) center. Finally, we simulated two scenarios of tailings dam failure.

## 85 2. Materials and methods

### 2.1. Mathematical and numerical models

In this study, the 3-D continuity and Navier-Stokes (N-S) equations are employed as governing equations, which follow the physics laws of conservation of mass and momentum. The governing equations (Versteeg and Malalasekera, 2007) can be written in the tensor form as:

$$\frac{\partial \rho}{\partial t} + \frac{\partial}{\partial x_i} (\rho u_i) = 0 \quad (1)$$

$$\frac{\partial}{\partial t} (\rho u_i) + \frac{\partial}{\partial x_j} (\rho u_i u_j) = -\frac{\partial p}{\partial x_i} + \frac{\partial}{\partial x_j} \left( \mu \frac{\partial u_i}{\partial x_j} \right) \quad (2)$$

90 where  $\rho$  is the density;  $t$  is the time;  $u_i$  is the velocity;  $p$  is the pressure; and  $\mu$  is the viscosity.

Most natural or engineering fluids are in a turbulent state that causes fluctuations in velocity, pressure, temperature and other transport scalar in space and time. Tailings reservoirs are mostly located in mountainous areas, with a large terrain gradient. Under the influence of gravity, tailings fluids flow out of the  
 95 break at high speed and will be in a turbulent state (Blight, 1997). The standard k- $\epsilon$  Reynolds-Averaged Navier–Stokes (RANS) equations (Jones and Launder, 1972) are used to address the turbulence effect. This method introduces the kinetic energy  $k$  and the turbulent dissipation rate  $\epsilon$  with good convergence, which is the most widely used method in the engineering field. The RANS equations can be written as:

$$\frac{\partial}{\partial t} (\rho \overline{u_i}) + \frac{\partial}{\partial x_j} (\rho \overline{u_i u_j}) = -\frac{\partial \overline{p}}{\partial x_i} + \frac{\partial}{\partial x_j} \left( \mu \frac{\partial \overline{u_i}}{\partial x_j} - \rho \overline{u'_i u'_j} \right) \quad (3)$$

where  $\overline{\quad}$  denotes the average value; and  $'$  denotes the pulsation value.

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In the present model, the tailings fluid and air are assumed to be two incompressible phases, and the surface of the tailings phase movement is captured using the volume-of-fluid (VOF) method (Hirt and Nichols, 1981) that uses a step function  $F$  whose value is unity at any control volume occupied by tailings fluid and zero otherwise. The time dependence of  $F$  is governed by the eq. (4):

$$\frac{\partial F}{\partial t} + \nabla \cdot (u_i F) = 0 \quad (4)$$

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The above governing equations are solved based on the FVM, which is the most commonly used method in the field of fluid engineering (Schraml et al., 2015; Yoon et al., 2014). We split the computational region into a

number of control volumes, which is represented by a mesh of hexahedral cells in the Eulerian framework according to the FVM. The governing equations are discretized into a set of algebraic expressions involving the unknown values of the physical quantities at the center of each control volume by integrating the N-S equation over the control volume. The following high-order discretization schemes (Table 1) are used to interpolate the physical quantities of the centers of the control volumes onto the faces, and this setup is second-order accurate and fully bounded.

Preconditioning conjugate gradients (PCG) and smooth linear algebraic solvers are selected to calculate symmetric and asymmetric matrices separately in the work. Additionally, in terms of pressure-velocity coupling, we use the pressure implicit with the splitting of operators (PISO) (Issa et al., 1991) algorithm to achieve it. The PISO algorithm uses prediction-correction-recorrection steps, speeding up the convergence speed in the iterative process and improving the computational efficiency. To guarantee model stability, the Courant-Friedrichs-Lewy (CFL) condition must be satisfied. For the  $n$ -dimensional case, the CFL number is defined in the following general form:

$$CFL = \Delta t \sum_{i=1}^n \frac{u_i}{\Delta x_i} \quad (5)$$

The CFL number is a measure of how much information ( $u_i$ ) traverses a control volume ( $\Delta x_i$ ) in a given time-step ( $\Delta t$ ). The CFL condition can be restricted by a maximum CFL coefficient between 0 and 1, which is chosen to be 0.5 in the present work.

## 2.2. Rheological model

Tailings fluid is similar to a debris flow and is generally considered an incompressible and viscous fluid. Currently, most existing studies on debris flows and landslides assume that the mud-water mixture is a homogeneous non-Newtonian fluid with a constant density (Dai et al., 2014; Wang et al., 2016; Han et al., 2019). Several authors concluded that fluids with solid concentrations above 10% are non-Newtonian fluids (Komatina and Jovanovic, 1997; Sloff, 1993), and their rheological properties can be described by the Bingham model (Pastor et al., 2014; Komatina and Jovanovic, 1997; Han et al., 2019) that can be represented by the following constitutive equation:

$$\tau_{ij} = \left( \frac{\tau_b}{\dot{\gamma}} + \mu_b \right) \dot{\gamma}_{ij} \quad (6)$$

where  $\tau_b$  is the yield stress;  $\mu_b$  is the viscosity coefficient;  $\tau_{ij}$  is the shear stress tensor; and  $\dot{\gamma}_{ij}$  is the rate-of-deformation tensor:

$$\dot{\gamma}_{ij} \equiv \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \quad (7)$$

The symbol  $\dot{\gamma}$  is the second invariant of the  $\dot{\gamma}_{ij}$ , which is given by:

$$\dot{\gamma} = \sqrt{\frac{1}{2} \dot{\gamma}_{ij} \dot{\gamma}_{ij}} \quad (8)$$

Bingham fluid is a kind of plastic fluid; as long as the shear stress exceeds the threshold  $\tau_b$  specific to the material, the material flows like a Newtonian fluid. Where the threshold is not exceeded, the plastic fluid behaves as a solid. The average diameter of the tailings is generally less than 0.1 mm. At present, several laboratory rheological tests have shown that the rheological properties of a water-tailings mixture conform to those of a Bingham fluid (Mizani et al., 2013;Henriquez and Simms, 2009;Liao and Zhou, 2015;Gao and Fourie, 2015); hence, the Bingham model is adopted as the constitutive model to describe the fluidization characteristics of flow-like tailings in this work. In the Bingham model, the unknown parameters are  $\tau_b$  and  $\mu_b$ , which can be determined by laboratory measurements. The apparent viscosity in a Bingham model can be represented as:

$$\mu_{eff} = \frac{\tau_b}{\dot{\gamma}} + \mu_b \quad (9)$$

It is understandable that the effective viscosity will become unbound when  $\dot{\gamma}$  tends to be infinitely small, such as in the core region of a Bingham flow, which may cause numerical divergence or even crash in the solution procedure (Shao and Lo, 2003). To overcome this inherent discontinuity issue, several regularized approaches have been proposed, where the infinite viscosity presented in the rigid zone can be approximated by a highly viscous fluid (Papanastasiou, 1987;Hannani et al., 2007;Frigaard and Nouar, 2005). The most popular regularization is the Bingham-Papanastasiou model(Papanastasiou, 1987), which is chosen as the constitutive equation of tailings fluid in the present work. Hence, the apparent viscosity is expressed as:

$$\mu_{eff} = \frac{\tau_b}{\dot{\gamma}} [1 - e^{(-q\dot{\gamma})}] + \mu_b \quad (10)$$

where  $q$  denotes the stress growth parameter, such that exceeding the yield stress  $\tau_b$  a finite shear stress is allowed for small shear rates, while the stress grows linearly with shear stress beyond the yield stress. Moreover, the limit of  $m \rightarrow \infty$  is fully equivalent to an ideal Bingham fluid.

In this study, mathematical and rheological models are implemented using the release of version 6 (<https://github.com/OpenFOAM/OpenFOAM-6>) of the open-source Field Operation and Manipulation (OpenFOAM) C++ libraries under a Linux environment. OpenFOAM is a framework for developing application executables that use packaged functionality contained within a collection of approximately 100 C++ libraries (Greenshields, 2018).

### 2.3. Experimental verification

To investigate the performance of the model in predicting the behavior of a Bingham fluid, we compared the simulation results with the analytical solutions and experimental results for the flow of a Bingham fluid between two parallel plates and dam-break type flow. The purpose of the flow test of a Bingham fluid between two parallel plates is to check whether the Bingham-Papanastasiou regularization method can correctly describe the flow phenomena of a Bingham fluid, and the 3-D dam-break experiment is used to test the ability of the above scheme to capture the free surface of a fluid and calculate pressure and velocity.

#### 2.3.1. Flow between two parallel plates

The test of flow between two parallel plates was used for the validation of the implementation of the rheological model in OpenFOAM since the analytical solution can be easily acquired from:

$$u(y) = \frac{\Delta PD}{2\mu_b L} \left( y - \frac{y^2}{D} \right) - \frac{\tau_b y}{\mu_b} \quad \text{for } 0 \leq y < S \quad (11)$$

Equation (11) gives the expression for velocity for  $\tau > \tau_b$  (Gopala et al., 2011; Waarde, 2007), and the velocity at the plug is given by Eq. (12):

$$u_{plug} = u(S) \quad \text{for } 0 \leq y < S \quad (12)$$

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For the simulation of flow between parallel plates, a 2-D computational domain was constructed, which consisted of a rectangular channel with length  $L = 2 \text{ m}$  and height  $D = 0.1 \text{ m}$ . The test configuration is schematically shown in Fig. 1. The domain was discretized with  $400 \times 40$  cells limited by three types of boundary faces: inlet, outlet, and no-slip walls, and the Bingham fluid flowed from the inlet to the outlet at a constant velocity of  $0.069 \text{ m/s}$ . Two types of Bingham fluids were simulated, namely, a self-compacting cement (SCC) mixture and grout, and their rheological parameters are shown in Table 2 with reference to Gopala (Gopala et al., 2011). In Fig. 2, the velocity profiles were compared between the numerical simulation and the analytical solution for the SCC and the grout. The simulated results and analytical solutions appear to be in very close agreement, with the implemented results of the Bingham-Papanastasiou model fitting the analytical velocity curves very well. For the plug flow region, the yield stress of the SCC is two orders of magnitude higher than that of the grout, so there is an obvious plug region in the flow of the SCC, as shown in Fig. 2.

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### 2.3.2. 3-D dam-break experiment

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The second experiment presented here is one of the dam-break type problems to validate the model. The dam-break problem is a classical validation case for the assessment of free surface modeling methods. As shown in Fig. 3, the experiment is carried out in a large tank of  $3.22 \text{ m} \times 1 \text{ m} \times 1 \text{ m}$  with an open roof, and the right part of the tank is a rectangular box of water between a fixed wall and a temporary door. At time  $t = 0 \text{ s}$ , the temporary door is removed instantaneously, allowing the fluid to collapse under the influence of gravity  $g = 9.81 \text{ m/s}^2$ . During the collapse, the fluid impacts an obstacle at the bottom of the tank and creates a complicated flow structure. The geometry and the probe positions are briefly described in Fig. 3; points P1 to P8 are pressure sensors on the obstacle box, while points H1 to H4 are used to observe the fluid heights. In this simulation, mesh are used in a structural pattern with an initial spacing of  $0.015 \text{ m}$ , so a set of 971,498 cells is employed, and the boundary consists of the no-slip walls and the top boundary that is free to the atmosphere so that both outflow and inflow are permitted according to the internal flow. For the rheology, the typical values used in the simulations are  $\mu = 0.001 \text{ Pa s}$ ,  $\tau_b = 0 \text{ Pa}$  and  $\rho = 1,000 \text{ kg/m}^3$  for water-based suspensions at  $20^\circ \text{C}$ .

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The experiment was performed by the Maritime Research Institute Netherlands (MARIN), and all detailed experimental data can be downloaded free from this website (<https://app.spheric-sph.org/sites/spheric/tests/test-2>). In this case, we compared the simulation results against the experimental data obtained by MARIN. The free surface and velocity field of the simulation against different physical times  $t = 0.4 \text{ s}$  and  $t = 0.56 \text{ s}$  are shown in Fig. 4, which is similar to the results in Kleefsman (Kleefsman et al., 2005) and Lee (Lee et al., 2010). The small pictures on the top right of the

screenshots represent the fluid behind the gate. As this figure suggests, the simulation results are in good agreement with the experimental data. In Fig. 5, characteristic curves of pressure and vertical water heights varying with time at P1 and H4, respectively, are depicted. Similarly, the global behaviors of the simulation and experiment are fairly consistent, as shown in Fig. 5. Consequently, the ability of the above scheme to capture the free surface of a liquid and calculate the pressure and flow rate are indicated.

### 3. Simulation of tailings dam failures – Feijão and A'xi tailings dams

#### 3.1. Feijão tailings dam

The Córrego do Feijão iron mine complex, with two tailings dams, a cargo terminal, an administrative office and a small railway network for the transport of iron ore (Fig. 6.a), is located in Brumadinho, Minas Gerais state, Brazil, in the upper valley of the Paraopeba River. The tailings dam I, measuring 86 m high and 720 m long, was used for disposal of approximately 12.7 million  $m^3$  of iron tailings. On January 25, 2019, Dam I suffered structural damage and eventually collapsed catastrophically, causing at least 232 deaths (Santamarina et al., 2019). According to a field survey (Porsani et al., 2019) that was conducted after the collapse, approximately 11.7 million  $m^3$  of tailings were spilled from the dam, destroying office buildings, a railway bridge, and a small community. Under the action of gravity, the tailings fluids travelled 8.5 km and eventually flowed into the Paraopeba River, the region's main river. The downstream area submerged by tailings fluids was more than 2.54 million  $m^2$  based on measurements from remote sensing images from Sentinel Satellite S2 (see Fig. 6.b). According to surveillance cameras around the accident area, the velocity of the tailings fluid when the tailings dam failed was more than 10 m/s. Fig. 6 shows the area impacted by the Dam I collapse event.

#### 3.2. A'xi gold tailings dam

The A'xi gold mine tailings reservoir (Fig. 7) is 58 m high and located 46 km from the Ili River, Xinjiang, China, in the upper valley of the A'xi River, a tributary of the Ili River. Designed for disposal of approximately 3.6 million  $m^3$  of gold tailings, this tailings dam measures 210 m long and occupies 170 000  $m^2$  and is the largest tailings dam in Xinjiang. The tailings storage facility of the A'xi gold mine was constructed using the 'paddock' system of construction, where the subsequent dams raised were built on the tailings. In addition, it is located on the south slope of the main ridge of Kogurqin Mountain in the Borokunu Mountains of the Western Tianshan Mountains with an elevation of 1513 m and a large downstream topographic gradient. The mining area belongs to the continental climate of the north temperate zone, where the overall rainfall is low but unusual rainfall events occur frequently. At present, the tailings dam has entered the tail period of its service life with a storage capacity of approximately 2 million  $m^3$  of tailings. Additionally, the area is in an earthquake-prone zone and is prone to abnormal rainfall. Therefore, the possibility of tailings dam failure cannot be ruled out. If a tailings accident occurs, it will cause environmental disasters and large property losses and even pollute the large reservoirs downstream. Considering the serious consequences of the A'xi gold mine tailings dam if it encounters an accident, we chose it as a disaster assessment case study to simulate and analyze its dam-break process and run-out path.

#### 3.3. Key factors and model setup

The presented and validated CFD model was used to simulate the run-out path and range of tailings fluid from the Feijão tailings dam failure and to simulate the breaking of the A'xi tailings dam. The features of run-out are

greatly affected by three significant factors: the fineness of the downstream topography, the rheological characteristics of the tailings fluid and the initial state of the tailings fluid.

250 First, the terrain has a great influence on the tailings fluid's velocity and run-out path due to the gradient  
variations in the terrain. The fine terrain boundary, which is closer to the real landform, has more features and  
obvious topographic fluctuations than the coarse boundary, making the numerical simulation results more in  
line with the realistic phenomenon. Conversely, the coarse terrain boundary has a greater loss of detailed  
topographic characteristics. If the resolution of the terrain boundary is too low, it may cause a large difference  
255 between the simulation results and the real event. The topographical boundaries of a large area possibly  
affected by the tailings dam failure were constructed using a DEM. For the A'xi tailings dam, we went to the  
field to collect a group of aerial photographs with a certain overlapping range using the UAV. Based on these  
overlapped images, a DEM was reconstructed with a resolution of approximately  $0.5\text{ m} \times 0.5\text{ m}$ , which can be  
further generated into a 3-D terrain model. However, due to the large altitude drop and the limited endurance of  
260 the drone used in this area, images of some areas that may be affected by tailings fluids have not been collected.  
For these areas, we chose the ALOS PALSAR RTC (radiometrically terrain corrected) DEM with a  $12.5\text{ m} \times$   
 $12.5\text{ m}$  resolution released by the Alaska Satellite Facility  
(<https://www.asf.alaska.edu/sar-data/palsar/terrain-corrected-rtc/>) to replace them, and all of the DEM was  
georeferenced, cropped, and reconstructed to form the final 3-D terrain geometry. Because the Feijão tailings  
265 dam has collapsed, the DEM data made by drone before the accident could not be obtained; therefore, we also  
used the ALOS PALSAR RTC DEM to make the 3-D terrain geometry. Fig. 8 shows the DEM of the A'xi  
tailings reservoir and the reconstructed three-dimensional model computational grid. The 3-D model of the  
terrain was built to make the meshes of the computational domain, which were the data that must be used in  
numerical calculations based on the FVM. For mesh generation, an OpenFOAM utility, snappyHexMesh, was  
270 employed to generate 3-D meshes containing hexahedra and split-hexahedra automatically from a 3-D file  
(STL or OBJ format).

Furthermore, the travel distance and extent of a tailings outflow are affected by the properties of the tailings,  
such as viscosity, field stress and density. In general, the greater the viscosity, yield stress and density of the  
275 tailings are, the smaller the range that the tailings reach under the same volumes released from the dam. For the  
value of viscosity and yield stress used in the modeling, we referred to the rheological test reported by Liao  
(Liao and Zhou, 2015). Liao (Liao and Zhou, 2015) collected tailings samples from three different tailings  
ponds and mixed these tailings with water to form different concentrations of tailings fluid for rheological tests.  
A total of 15 rheological tests were carried out, with a range of dams(#1-#3), and volume concentrations of  
280 mixtures of 60%-80%. The #1 tailings dam and the Feijão tailings dam are both iron ore tailings ponds, and  
their tailings have a smaller particle diameter belonging to silty soil, similar to that of the A'xi tailings dam.  
Therefore, the 80% volume concentration of the #1 tailings dam is selected in this investigation,  
corresponding to a density  $1,830\text{ kg/m}^3$ , a viscosity of  $0.741\text{ Pa s}$  and a yield stress of  $59.81\text{ Pa}$ .

285 Undoubtedly, the initial conditions of the released tailings fluid can have an important effect on the velocity  
and out-flow distance of the tailings, especially at the time of the dam break, which is mainly caused by the  
initial geometry and volume of the tailings. For the Feijão tailings dam, according to the monitoring video of



the accident area, the structure of the dam body completely collapsed instantaneously, and the same hypothesis was also used for the A'xi tailings pond. The volume of tailings released by the Feijão tailings  
290 accident was set to 11.7 million  $m^3$  based on subsequent reports. While the Feijão tailings dam almost  
discharged all the tailings it had stored, tailings dam accidents will generally only release part of the tailings  
stored, according to many historical accidents. The volume of tailings released after the dam break at the A'xi  
Gold Mine tailings reservoir is estimated to be approximately 756,000  $m^3$  based on the regression equation  
proposed by Concha (Larrauri and Lall, 2018):

$$\log(V_F) = -0.477 + 0.954 \times \log(V_T) \quad (13)$$

295 where  $V_T$  is the total impounded volume and  $V_F$  is the volume of tailings that could potentially be released.  
For the initial 3-D model of the tailings dam, its volume is equal to the total volume of tailings released,  
which is constructed based on the dam height and geometric shape of the tailings pond. Formerly, the 3-D  
topographic model was modified to suit the initial geometry of the tailings.

300 Generally, a smaller cell spacing provides a better simulation result, but it will dramatically increase the time  
for calculation. In our simulation, the cell spacings of the Feijão tailings dam and the A'xi tailings dam are  
fixed to  $10\text{ m} \times 10\text{ m} \times 3\text{ m}$  and  $3\text{ m} \times 3\text{ m} \times 3\text{ m}$ , generating 3,241,998 cells and 6,657,418 cells,  
respectively. The terrain elevation within each cell is evenly distributed, so it is necessary to supplement  
relatively accurate surface information such as terrain roughness. Owing to the difference in the surface cover,  
305 the roughness length of the downstream terrain of the Feijão tailings dam and the A'xi tailings dam are set to  
 $1\text{ m}$  and  $0.1\text{ m}$ , respectively, based on a criterion given by Henderson-Sellers (Henderson-Sellers et al.,  
1993). Finally, the geometry and associated fields are decomposed into pieces using the DecomposePar utility,  
and each separate part of the decomposed domain was run on the cluster of the Fujian HPC by using the  
IntelMPI (an implementation of the standard message passing interface) and SLURM workload manager.

### 310 3.4. Results and discussion

The proposed CFD approach was used to simulate the dynamic behavior of tailings flow over the 3-D  
topography based on the above settings, and the discussion of the propagation features of the tailings outflow is  
presented as follows.

315 For the Feijão tailings reservoir, we simulated the travel of tailings for 2,500  $s$  at the time of the disaster.  
The typical simulation snapshots of the dynamic behavior of the 2019 Feijão dam-break event are presented in  
Fig. 9. A fairly good agreement with the historical run-out area in situ can be observed. However, there is a  
significant difference in the location of the railway network (see Fig. 6.a.), which is mainly due to the  
acquisition time of the DEM used in 2011 when the railway network had not been built. After dam failure, the  
320 gravitational potential energy of the tailings fluid was converted into kinetic energy, causing the velocity to  
increase suddenly, even more than  $20\text{ m/s}$ . Subsequently, it takes  $150\text{ s}$  for the tailings fluid to travel  
approximately  $1,300\text{ m}$  and reach the office center and small community, with a velocity of  $15\text{ m/s}$ . After  
 $150\text{ s}$ , the front flow smashed the railway bridge at approximately  $3,150\text{ m}$  downstream, with a decreasing  
average velocity less than  $5\text{ m/s}$ . Finally, it flowed into the Paraopeba River at approximately  $2,400\text{ s}$ , with  
325 a small average velocity of less than  $2\text{ m/s}$ , and the total travel distance was approximately  $8,560\text{ m}$ . In  
general, the numerical results of the tailings flow directions and impact area were similar to the historical

results. However, the collapse of the tailings pond is a very complicated geographical phenomenon. It is impossible to fully satisfy the historical situation in the simulation because unknown or unclear factors are not considered, so it might not be closely matched in terms of time.

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According to the relevant settings in Section 3.3, if the A'xi gold mine tailings pond experiences a dam failure, the flow path and submerged range of the tailings were predicted within 800 s. The simulated propagation of the tailings fluid released from the A'xi tailings dam failure is shown in Fig. 10. After the tailings fluid is released from the breach, it flows downward along the terrain under the action of gravity. At 20 s, it first submerges the environmental protection depot and power distribution room (see Fig. 10) approximately 250 m downstream of the tailings pond, and the speed exceeds 15 m/s. Immediately afterwards, the tailings fluid enters the Ahi Valley, approximately 320 m from the dam site, and flows downstream along the terrain in the valley. Although the elevation difference in the valley is more than 155 m (in the calculation domain), the valley is so tortuous that the flow rate of the tailings fluid gradually decreases. Eventually, at 760 s, the front reaches the simulated boundary with an average velocity of less than 2 m/s and a flow distance of approximately 3.5 km. It can be seen from the numerical results that almost 75,600 m<sup>3</sup> of tailings released after the break of the A'xi gold mine tailings dam are almost poured into the A'xi Valley except that some of the tailings are blocked by the environmental protection depot. The normal runoff volume of the A'xi River is 0.5-2 m<sup>3</sup>/s, and the runoff volume during the dry season is 0.002 m<sup>3</sup>/s. Overall, the runoff volume is relatively small, so the influence of water flow is not considered in the simulation. However, the region is prone to abnormal rainfall events, which lead to an increase in river runoff. Under this situation, the toxic tailings that flow into the river valley would be transported by the river to the large reservoir downstream, or even eventually to the Ili River, causing serious ecological pollution.

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From the simulated results shown above, it can reasonably be concluded that the proposed method can accurately represent the whole dynamic processes of tailings movement after dam failure. In addition, parallel technology was used to speed up computation. We tested the parallel speedup for different numbers of processors on the supercomputing cluster of the Fujian HPC, with a computational load of more than 3 million cells, and the speedup was nearly 50 times. When the number of processors was more than 50, the calculation time was no longer reduced because the acceleration ratio also depends on the amount of computation. If the number of cells increases, the speedup will be greater. Fig. 11 shows the parallel speedup of different numbers of processors.

#### 4. Conclusions

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The damage caused by tailings dam failures occurring frequently around the globe can be costly and catastrophic. The topic of disaster prevention and mitigation is being discussed more in government departments and the mining industry. The debris flow from a tailings dam failure is a very complex phenomenon involving physical mechanisms and the evolution of fluid properties. In this paper, a 3-D CFD model based on the FVM was proposed to predict the routing and impact area of tailings across complex topography during tailings dam failure. The Navier–Stokes equations and the Bingham rheology model are introduced into the FVM framework as the governing equations and the constitutive model, respectively. To construct a wide range of detailed downstream terrain, UAV photogrammetry and satellite imagery are used

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to sample surface information. Moreover, parallel technology was used to simulate the complex problem of the supercomputing cluster to achieve rapid and reasonable predictions for disaster prevention and mitigation.

370 To verify the performance of the reported scheme, the numerical model is first tested against an analytical  
solution in the literature for the flow of a Bingham fluid between two parallel plates, demonstrating the correct  
implementation of the Bingham-Papanastasiou constitutive model in OpenFOAM. Next, the numerical model  
is validated against the experimental value for the 3-D dam-break experiment, thereby ensuring a good  
prediction of the free surface profile. To further illustrate the performance of the presented method, the 2019  
375 Feijão dam-break event in Brazil was selected as a case study, and the numerical simulation results coincide  
well with the in situ investigation. Finally, considering the serious consequences of the A'xi gold mine tailings  
dam if it encounters an accident, we chose it as a disaster assessment case study to simulate and analyze its  
dam-break process and run-out path. Therefore, CFD numerical modeling can provide an effective means for  
mapping hazardous areas, estimating hazard intensity, and designing appropriate protective measures.

380 **Code availability:** Code can be made available by the authors upon request.

**Data availability:** Data can be made available by the authors upon request.

**Author contributions:** LT, DY and CC conceived and designed the method; DY carried out the simulations, produced the results, and wrote the original manuscript under the supervision of LT. LT and CC writing-review and editing.

**Competing interest:** The authors declare that they have no conflicts of interest.

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open-source CFD code and released it to the public.

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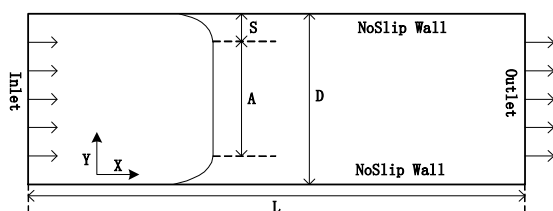
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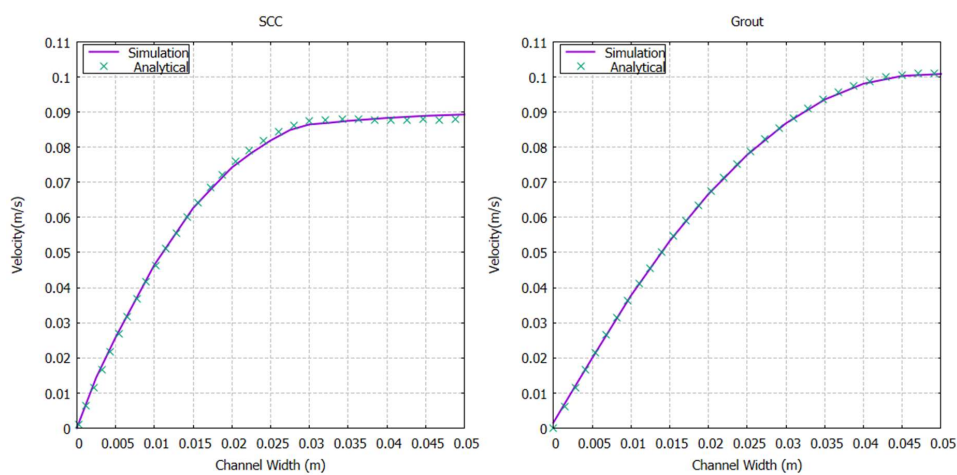
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495 **Figures**

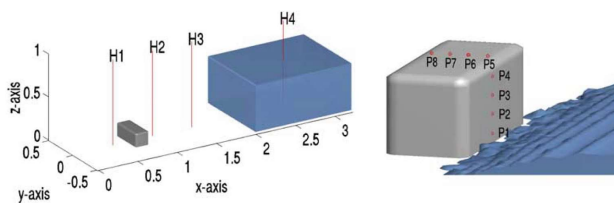


**Figure 1: Schematic of the flow between parallel plates.**



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**Figure 2: Velocity comparison between analytical solutions (Gopala et al., 2011) and simulated results for SCC and Grout fluids.**



505 **Figure 3: Geometry and measurement positions of the 3-D dam-break experiment (from Kleefsman (Kleefsman et al., 2005)).**

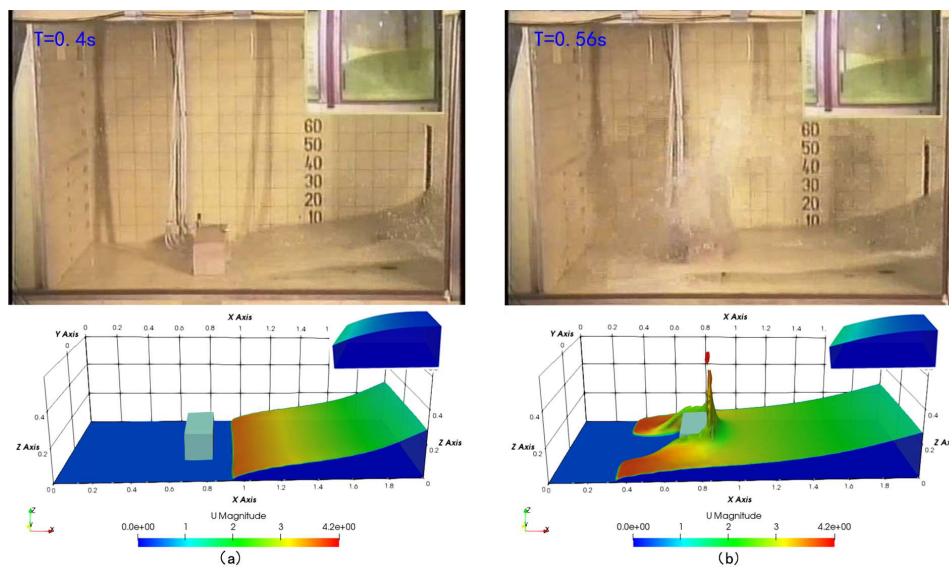


Figure 4: Screenshots of the simulation compared with the experiment (Kleefsman et al., 2005) at  $t = 0.4\text{ s}$  (a) and  $t = 0.56\text{ s}$  (b). The color map of the simulation results is mapped by velocity characteristics.

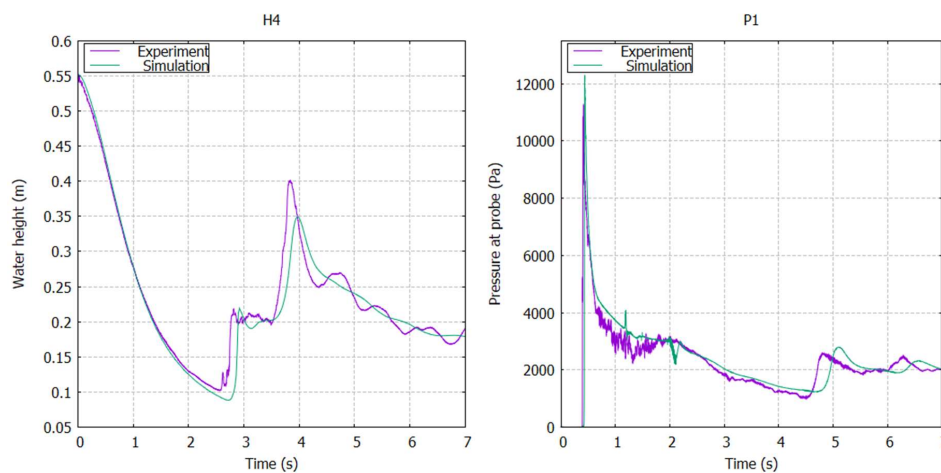
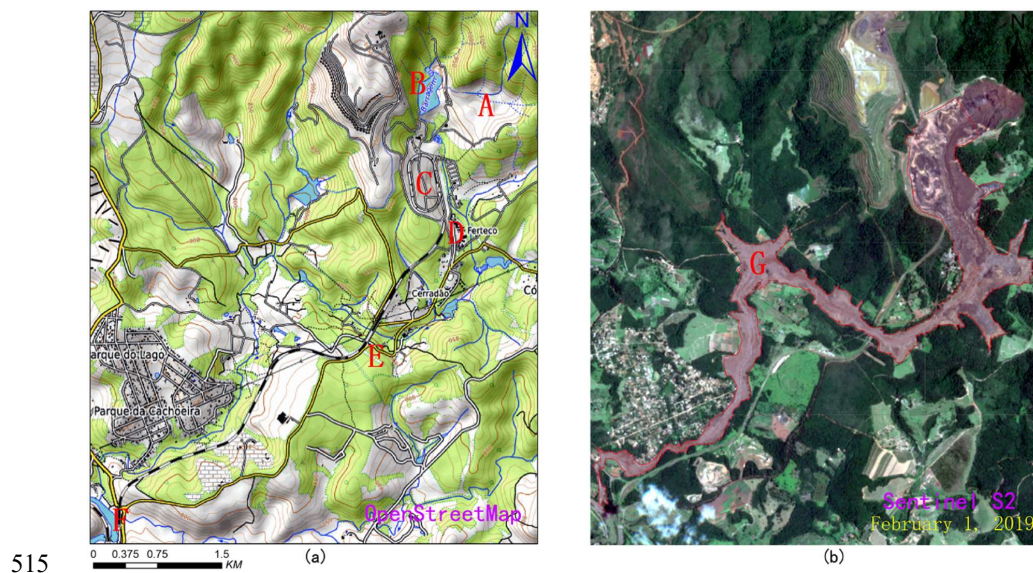


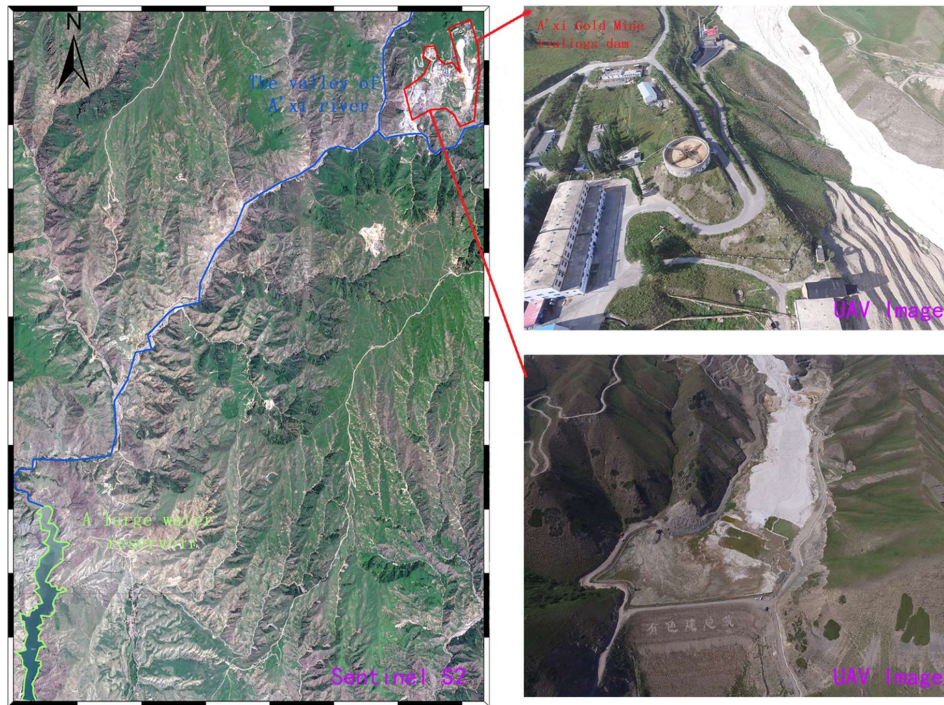
Figure 5: Vertical water heights in reservoir H4 (left) and characteristic curve of pressure changing with time at P1 (right).



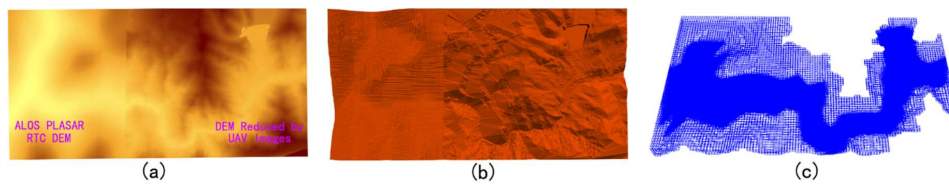


**Figure 6: Location of the Feijão iron mine complex with reconstructions of the area impacted by the 2019 Dam I failure event: (a) Location and downstream area of the Feijão tailings dam. A Location of Dam I. B Location of Dam VI with destabilized dam due to the event. C&E Locations of the rail network and bridge. D Location of the office center and a small community. F Location of the Paraopeba River; (b) G The destroyed downstream area. Map data: OpenStreetMap and Sentinel Satellite S2 imagery.**

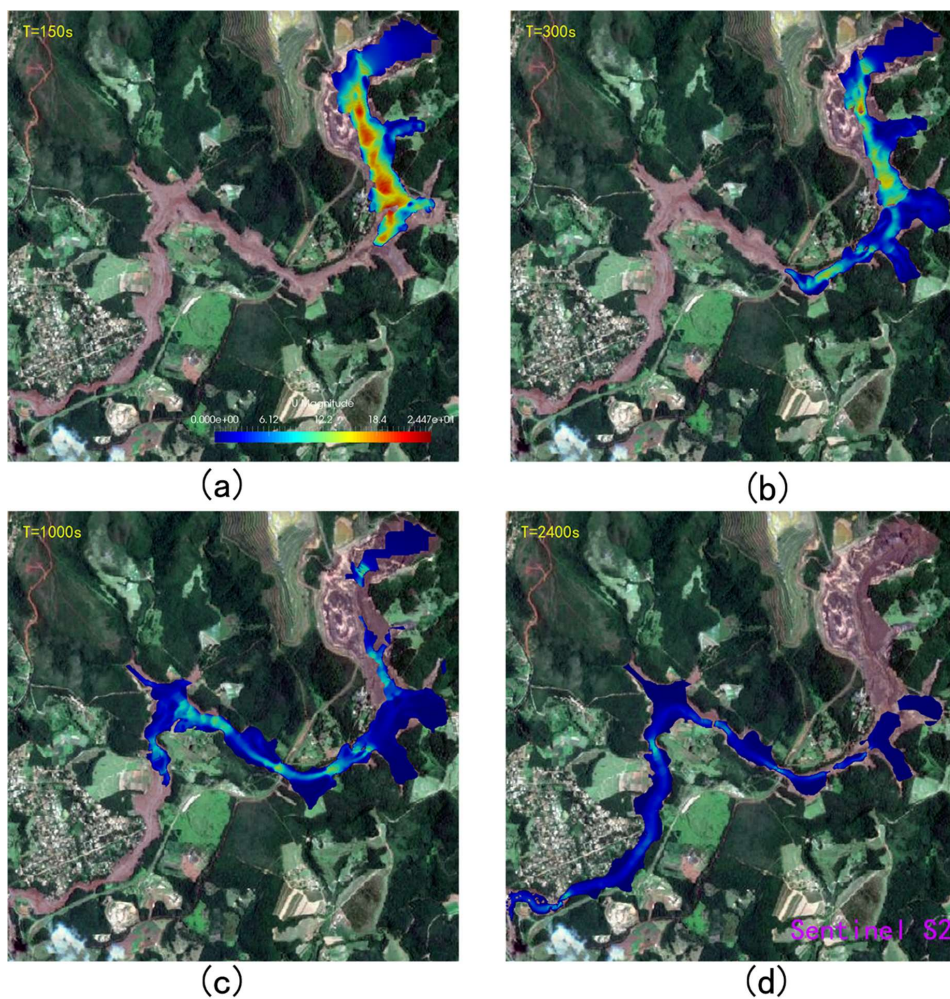




525 **Figure 7:** Location of A'xi gold mine tailings dam: The red area is the A'xi gold mine complex; the blue line represents the valley of the A'xi River; and the green area represents a large water reservoir. Image data: Sentinel Satellite S2 imagery and UAV imagery.

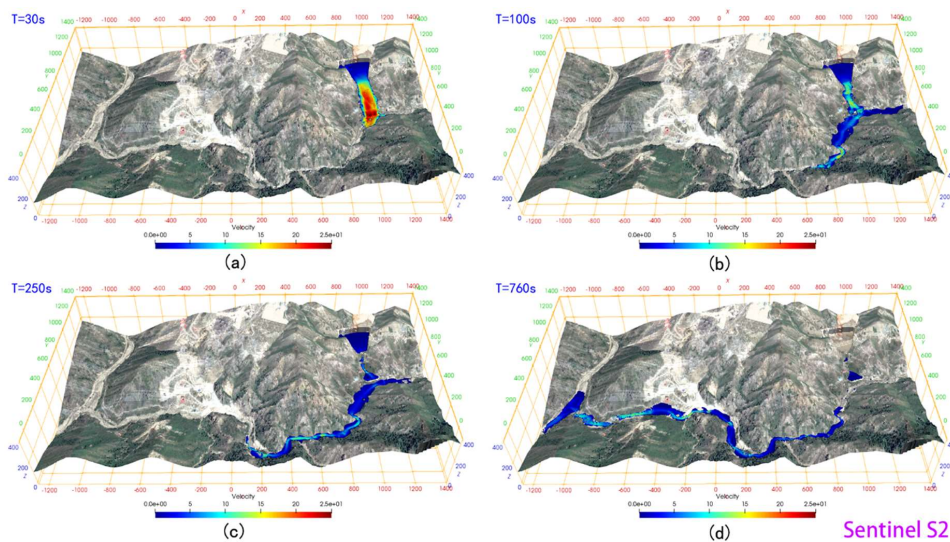


530 **Figure 8:** The fine DEM generated by UAV (right) and ALOS satellite (left), reconstructed 3-D terrain model and the computational meshes

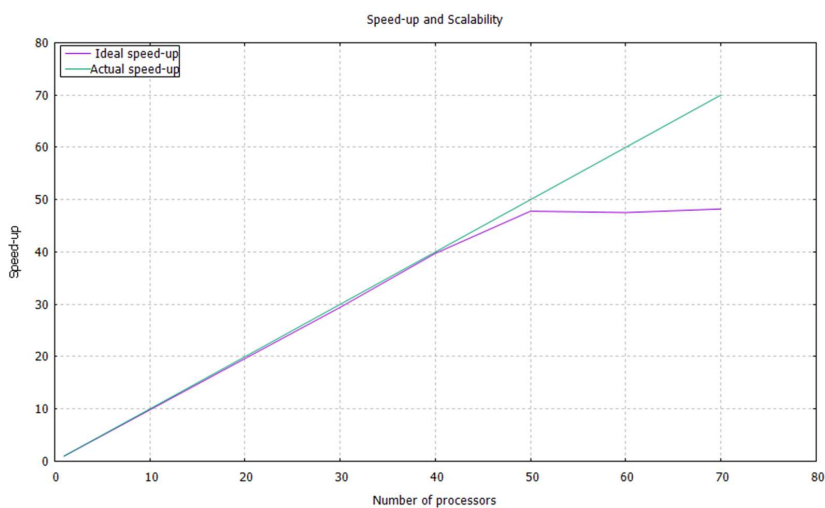


535 **Figure 9:** The time sequence of the numerically computed event up to the tailings reaching the Paraopeba River. The base map is the satellite image (Sentinel S2) after the dam break. The color map of the simulation results is mapped by velocity characteristics.





540 **Figure 10:** The time sequence of the numerically computed event up to the tailings reaching the left boundary of the  
computational domain. The base map is the satellite image (Sentinel S2). The color map of the simulation results is  
mapped by velocity characteristics.



545 **Figure 11:** Parallel speedup with different numbers of processors. The green line is the ideal speedup, and the  
purple line is the actual speedup.



Tables

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**Table 1.** Discretization schemes of the model

$t$	$\nabla$	$\nabla \cdot$	$\nabla^2$
Euler	Gauss linear	Gauss VanLeer/ Gauss linear	Gauss linear corrected

where  $\nabla$  is the Gradient operator;  $\nabla \cdot$  is the Nabla operator; and  $\nabla^2$  is the Lapacian operator.

**Table 2.** Rheological parameters of a simulated Bingham fluid.

	SCC	Grout
Density ( $\text{kg}/\text{m}^3$ )	2200	1900
Yield stress (Pa)	131	2
Viscosity ( $\text{Pa} \cdot \text{s}$ )	44.9	11