Referee #4

Q1. The paper " Numerical and remote techniques for operational beach management under storm group forcing" attempts to describe the morphodynamic response of a microtidal beach under a storm group. The methodology incorporates a wide range of complementary methods to access beach morphodynamics and includes both crossshore and alongshore components.

The paper is interesting and deals with a topic of great importance and clearly within the scope of Nat. Hazards Earth Syst. Sci. Nevertheless, I have some critical comments that should be addressed: (1) definition of manuscript objectives needs to be improved; (2) model application should be more clearly explained; (3) validation methodology should be greatly developed, and (4) the work novelty needs to be highlighted.

> *Reply #1. We deeply thank Referee's comments and the effort that she/he made in reviewing carefully our work. We have introduced in the new version of the Manuscript (hereinafter Ms) all points raised in the review. We sincerely think that, thanks to this discussion, the new version of the Ms has been improved.*
>
> *Regarding the improvement of the objectives, the last part of the Introduction section has been rewritten and the lines #3-5 (page 3) of the new version of the Ms, read,*
>
>> *"The goal of this contribution is to study the effect of a storm group on the morphology of a beach system and to advance a multiplatform methodology for an effective decision-making regarding beach erosion management according to the available data and numerical models."*
>
> *We hope that with the new version of Ms. the model application, the validation methodology and the work novelty are much better enhanced.*

Q2. The second objective stated in the paper "secondly to present a multiplatform approach to help beach managers to make an insight-driven decisions concerning beach erosion management through the use of the data available in the beach and the operational run of numerical models" is unintelligible. Moreover, in the present state of model/platform? development, its application to coastal management is still very unlikely. In this sense, sentences like "This allows a correct management of the beach avoiding unnecessary engineering works between touristic seasons." seems out of scope.

> *Reply #2. As suggested, we have rewritten the main objective of this contribution in lines #3-5 (page 3 new Ms.) as,*
>
>> *"The goal of this contribution is to study the effect of a storm group on the morphology of a beach system and to advance a multiplatform methodology for an effective decision-making regarding beach erosion management according to the available data and numerical models."*
>
> *The showed sentence has been rewritten in lines #6-7 (page 9 new Ms.) of the new version of Ms, as*
>
>> *"This would allow a correct management of the beach avoiding unnecessary engineering works between touristic seasons."*
>
> *Also in the discussion lines #19-23 (page 10) we stated:*
>
>> *"The proposed approach aims to be a tool to assist to the beach management specially during adverse conditions, when field surveys are not possible. The combination of numerical models, video-monitoring and in situ data provide alternatives for the lack of data especially during adverse conditions. This approach follows the change in the paradigm in ocean studies where multiplatform approaches are being developed abroad the globe in order to fill spatial and temporal gaps in the measured time-series."*

Q3. 2) Description of model application should be sufficient clear to allow other researches to replicate the work and several methodological decisions require demonstration. For example:

- how were waves propagated from the AWAC to the offshore boundary of Xbeach? With 10 s waves it is expect that alongshore forcing was not uniform along the Xbeach offshore boundary – how was this accounted for?

> *Reply #3. We agree with you. We consider the wave conditions of the AWAC located at 25 m depth in the seaward boundary of the XBeach simulation, although our grid only goes to 15 m depth, we consider that waves have not been greatly affected between these two positions.*

Q4. - Figure 1 (source and date of the image is missing) and other images at the internet suggests the existence of a rocky platform. Is this considered in the modelling approach? The figure should include bathymetric contours and the nature of the bottom (is case of the existence of a rocky bottom).

> *Reply #4. We thank the referee comment. Figure 1 has been edited in the new version of the Ms. showing all the required information.*

> *Cala Millor is a sand beach where there is a reduced area with rocky outcrops. This zone was neglected because of its narrow dimensions.*

Q5. - How was the closure depth computed? How far from the shore is it located?

> *Reply #5. The depth of closure in Cala Millor Beach is computed according to the Hallermeier (1981) formulation. This is the following one:*
> $$h^* = 1.75 \cdot H_{s,12} - 57.9 \cdot \left(\frac{H_{s,12}^2}{g \cdot T_{s,12}^2} \right);$$
> *where $H_{s,12}$ is the significant wave height that exceeded 12 hours per year; $T_{s,12}$ is the wave period associated with this value of wave height; and g is the gravity.*

> *This depth is found between 300 and 400 m from Cala Millor beach-boulevard.*

Q6. - Bathymetry inversion of the timestack used linear shallow water approximation in the breaker zone? Most works that have dealt with this problem showed that this approach does not give good results.

> *Reply #6. The cutting of the zone where the inversion bathymetry is carried out depends on the image. We remove the breaking zone and aerial beach, since there is a lot of noise in the images.*

Q7. - Why the authors did not use intermediate depth approximation at larger depths? This would avoid the problem stated by the authors that "The largest differences tend to be located at deep profile positions where the model is known to perform worse since the accepted assumption on Eq. (1) only is valid for shallow waters".

> *Reply #7. The maximum errors are in deeper depths because the cameras' resolution is lower and the further is the pixel, more it will be distorted. We have not found great differences in this configuration using the shallow water or the intermediate water approximation but the shallow water reduces considerable the cost of the inversion.*

Q8. - What is the line represented in figure 8? In this figure, the higher erosion values onshore (red pixels) seem to be connected with areas with no erosion (white pixels) suggestion therefore the generation of a beach scarp? Is this true? This feature was also observed on video images?

> *Reply #8. The black line in Fig. 8 is the shoreline on March 17th. We used it as a reference inside the beach, only for evaluating better the sediment movement.*

> *In this figure the value of erosion is progressive, we can see red areas followed by yellow zones and then white zones (no erosion). This is consistent with the obtained erosion/accretion thanks to the video-monitoring approach, in Fig. 9.*

Q9. 3) In my opinion, the validation methodology presented is the weakest element of the work. In fact, the presented information is very scarce and the approximation unconventional. The use of standard error

parameters is strongly suggested (see for example Roelvink et al. 2009). At least one figure with the comparison of estimated and measured data across a profile is also needed. Without that the reader cannot properly access results reliability and therefore can be skeptical about paper conclusions.

Reply #9. Following Reviewer comment, we include the statistical values for the comparison of model and measurements following Roelvink et al., 2009. In this sense, we show the correlation coefficient, the scatter index and the relative bias for each profile (see Fig. 1 of the Ms.) in the new table 1. It is,

Table 1. Error statistics for the simulated profiles by XBeach compared with the measured profiles during Riskbeach.

Profile #	$R^2(\%)$	SCI	Relative bias
01	99.79 ± 0.08	0.07 ± 0.03	0.02 ± 0.04
03	99.77 ± 0.09	0.07 ± 0.03	-0.02 ± 0.03
05	99.48 ± 0.13	0.08 ± 0.01	0.01 ± 0.01
07	99.53 ± 0.08	0.09 ± 0.01	0.00 ± 0.03
09	99.31 ± 0.21	0.11 ± 0.02	0.03 ± 0.01
11	99.73 ± 0.18	0.06 ± 0.02	0.00 ± 0.01
13	99.72 ± 0.03	0.07 ± 0.01	0.02 ± 0.03
15	99.59 ± 0.49	0.08 ± 0.03	-0.03 ± 0.02
17	99.90 ± 0.04	0.04 ± 0.01	0.03 ± 0.02

Q10. 4) Introduction is too large, and novelty of the work is not properly stressed. The introduction should also be more focused on paper objectives.

Reply #10. We have substantially changed the introduction stating more specifically the objectives followed in the paper.

Q11. Paper would also benefit from extensive editing work as a number of inaccuracies were detected that should be corrected by the authors (some examples are given below).

Reply #11. We careful review the new version.

Q12. -Abstract is very confusing and only describes the results and some conclusions; the abstract should be a shorth summary of the entire work: motivation, objectives, methods, results and conclusions.

Reply #12. We edited the abstract.

Q13. -Authors should make correct use of significant figures.

Reply #13. Typos have been addressed.

Q14. -Storm group should be clearly defined – the storm wave threshold is relative to offshore conditions and is independent of wave direction?

Reply #14. To clarify the concept of storm group, the lines #21-26 (page 5) have been rewritten in the new version of the Ms, as,

"We define here storm as sustained wave conditions during at least 6 hours with $H_s > 1$ m. Gómez-Pujol et al. (2011), suggested this threshold as the conditions required to generate a significant impact along beach morphology and sediment properties. When such an event is not isolated but becomes a succession of events, we are referring as a group of storms. These episodes can cause stronger damages in the beach with smaller wave heights, since the beach does not have enough time for recovering its initial morphodynamic state."

The imposed threshold is independent of the main wave direction, although we can see that the most energetic event is perpendicular to the coastline, as the bibliography shows.

Q15. -Pg 1 - l14 – "all temporal scales" – specify – this includes thousands of years?

Reply #15. Following Reviewer comment, in the new version of the Ms., the line 16 in page 1 has been rewritten as,

"Evolution of sandy coasts at temporal scales (from minutes to years) has been a topic of wide interest".

Q16. -Pg 2 – l2 – "0.01 % of land surface? How was this figure computed? What is beach definition? It includes the submarine domain? the gross domestic product is produced by beaches?

Reply #16. We took the information from Yepes and Medina., 2005.

Q17. -Pg 2 – l6 what is "RTK and echosounding surveys"?
Reply #17. They are bathymetric surveys with GPS signal technology, where a single reference station provides corrections in real time, obtaining a submetric accuracy. In RiskBeach campaign, the horizontal accuracy was around 8 mm and the vertical one around 15 mm. The acronyms RTK are Real Time Kinematic.

To avoid confusions, we have rewritten the lines #5-9 (page 3) of the page 2 in the new version of Ms. as,

"Here, we present the explanation of temporal patterns of beach accretion and erosion under consecutive storm events at an intermediate microtidal carbonate beach by using the dataset available on the studied beach, high-frequency data on shoreline positions and crossshore profiles extracted from coastal videomonitoring techniques, Real Time Kinematic (RTK) and echosounding surveys, concurrent hydrodynamic measurements, and the use of numerical models widely validated in order to fill gaps in the dataset."

Q18. -Pg. 2 – l12 – how was the beach area computed? Emersed? Above some datum? Up to the close depth? When the authors say "bottom colonized by the endemic Posidonia oceanica meadow at depths from 6 to 35 m", this is also referring to the beach? The beach does not end at closure depth?

Reply #18. The beach area has been calculated by approximation through geometric shapes. We take into account all the beach, emerged and submerged (according to Álvarez-Ellacuria et al., 2011) to the closure depth (from +2 m depth to -7 m depth, approximately).

Regarding the Posidonia Oceanica meadow, it extends from 6 to 35 m depth. This only is a date, our beach ends at closure depth, as Reviewer mentions.

Q19. -Pg 7 –l9 - the AWAC measured the "JONSWAP spectra"?

Reply #19. The JONSWAP spectra are generated with the measured AWAC data. For this reason, the lines #27-28 (page 7) in the new version of Ms., has been rewritten as,

"Hourly JONSWAP spectra, generated through the measured data with the AWAC."

Q20. -Modelling domain should be display in figure 1.

Reply #20. It is shown in the new version of Ms., figure 1.

Q21. -A bathymetric map is missing.

Reply #21. It is shown in the new version of Ms., figure 1.

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Álvarez-Ellacuria, A., Orfila, A., Gómez-Pujol, L., Simarro, G., & Obregon, N. (2011). Decoupling spatial and temporal patterns in short-term beach shoreline response to wave climate. *Geomorphology*, *128* (3-4), 199-208.

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Numerical and remote techniques for operational beach management under storm group forcing

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Abstract. The morphodynamic response of a microtidal beach under a storm group is analyzed, and the effects of each individual event inferred from a numerical model, in situ measurements and video imaging. The combination of these approaches represent a multiplatform tool for beach management especially during adverse conditions. Here, the morphodynamic response is examined during a group of three storms period. The first storm, with moderate conditions ($H_s \sim 1\,\mathrm{m}$ during 6 hours), erode the aerial beach and generate a submerged sandbar in the breaking zone. The bar is further directed offshore during the more energetic second event ($H_s = 3.5\,\mathrm{m}$ and 53 hours). The third storm, similar to the first one, hardly affect the beach morphology, which stresses the importance of the beach configuration previous to a storm. The volume of sand mobilized during the storm group is around 17.65 m^3/m. During the following months, which are characterized by mild wave conditions, the aerial beach recovered half of the volume of sand that is transported offshore during the storm group (~ 9.27 m^3/m). The analysis of beach evolution shows two different characteristic time scales for the erosion and the recovery processes associated with storm and mild conditions respectively. Besides, the response depends largely on the previous beach morphological state. The work also stresses the importance of using different tools (video-monitoring, modeling and field campaign) to analyze beach morphodynamics.

1 Introduction

Evolution of sandy coasts at temporal scales (from minutes to years) has been a topic of wide interest over the past decades since sandy beaches and dune systems are the first natural lines of coastal defense against flooding and erosion hazards (Callaghan

and Roshanka, 2009; Hallegatte et al., 2013), being at the same time attractive environments in terms of leisure activities and tourism economy (e.g., Jiménez et al., 2011; Bosello et al., 2012; Luijendijk et al., 2018). The maintenance of these areas is crucial for the coastal defense and, at the same time, the coastal tourism seems to be one main target for beach erosion management (Semeoshenkova and Newton, 2015). For instance, in Spain, beaches represent only the 0.01% of the land surface, producing up to 10% of its gross domestic product (Yepes and Medina., 2005). Beach management tends to be reactive rather than proactive, solving the problems as they appear and without a long term planning.

Mitigation of coastal erosion and preservation of coastal areas represent essential aspects of the Protocol on Integrated Coastal Zone Management in the Mediterranean and is included into the objectives of most countries' national regulations and policies in Europe (Semeoshenkova and Newton, 2015). It is already known that decisions concerning coastal management actions should be based using the best available science, and developing new tools that take into account physical, natural and socio-economic characteristics of beaches (Ariza, 2010; Tintoré et al., 2009). This makes it necessary to transfer the knowledge from scientists to managers in an effective way, which is nowadays an challenging matter.

Regarding the management associated to coastal erosion issues, several purposes have recently arisen to improve the managers decisions or, at least, to provide them quality data of the areas of study (Ferreira et al., 2017). One of the main issues in coastal erosion is the response of coastlines to both individual storms and storm groups since the behaviors are quite different (Loureiro et al., 2012; Vousdoukas et al., 2012; Houser, 2013; Coco et al., 2014; Masselink and van Heteren, 2014; Senechal et al., 2015; Masselink et al., 2016, i.e.). Single storms can result in significant beach erosion within a few hours, whereas a sequence of storms can have a large and complex impact on beach morphology whose final effects remain difficult to quantify and to predict (Ferreira, 2005; Frazer et al., 2009).

Storm waves and their associated water-level conditions are key drivers in the shoreline dynamics. Shoreline response to successive storms can be dependent on storm energy thresholds as well as on the feedback mechanisms associated with the beach morphology and the presence or absence of former impacts (Ciavola and Stive, 2012). There are many examples that have shown that shorelines can recover relatively well from erosion triggered by storms and that this recovery can be quick, from few days or weeks (Birkemeier, 1979; Vousdoukas et al., 2012) to a couple of months (Wang et al., 2006). Therefore, the resilience of beaches, understood as their capacity to recover from a major storm, is related to the combination of sediment reservoirs, arrangement of three-dimensional beach morphology (i.e., sand bar type and location, beach slope, etc.) and to the beach memory (Jara et al., 2015).

Recent works, as the one by Vousdoukas et al. (2012), have shown that the observed morphological change during consecutive storms has a strong dependence on the initial beach morphology. These authors, departing from field experiments in southern Portugal, stated that beach recovery did not maintain the pace with storms frequency and that storms can have a dramatic impact on the erosion if they occur grouped. In addition, other works dealing with storm impact in shoreline dynamics in the Bay of Biscay (SE France) have suggested that energetic events are probably not the only drivers of erosion processes, since significant beach erosion has been characterized under very calm conditions following energetic events (Senechal et al., 2015). In a similar way, observations from a detailed field campaign involving daily beach surveys at Truc Vert beach (Bordeaux, France) during a sequence of storms demonstrated that a sequence of extreme storms does not necessarily result in cumulative

erosion, possibly because of the interplay between water levels, the angle of wave approach and the pre-existing beachface conditions (Coco et al., 2014).

The goal of this contribution is to study the effect of a storm group on the morphology of a beach system and to advance a multiplatform methodology for an effective decision-making regarding beach erosion management according to the available data and numerical models. Here, we present the explanation of temporal patterns of beach accretion and erosion under consecutive storm events at an intermediate microtidal carbonate beach by using the dataset available on the studied beach, high-frequency data on shoreline positions and crossshore profiles extracted from coastal videomonitoring techniques, Real Time Kinematic (RTK) and echosounding surveys, concurrent hydrodynamic measurements, and the use of numerical models widely validated in order to fill gaps in the dataset.

2   Study area

Cala Millor is a semi embayed beach $1.7\,\mathrm{km}$ in length and ranging between 15 and $30\,\mathrm{m}$ in beach width. It is located in the Northeastern coast of Mallorca Island (Western Mediterranean Sea, Fig. 1). Sediments are mainly composed of well-sorted medium to coarse biogenic carbonate sand with a grain diameter D_{50} between 0.3 and $0.6\,\mathrm{mm}$ changing along the cross-shore distance, according to the depth (Gómez-Pujol et al., 2011). The beach area is around $1.4\,\mathrm{km}^2$ with a bottom colonized by the endemic *Posidonia oceanica* meadow at depths from 6 to $35\,\mathrm{m}$ (Infantes et al., 2009). This meadow increases bottom roughness reducing near bed velocity, modifying the sediment transport (Koch et al., 2007; Infantes et al., 2009, 2012) and increasing wave attenuation (Luhar et al., 2013).

From a morphodynamic point of view, Cala Millor is an intermediate beach with a highly dynamic configuration of longitudinal sinuous-parallel bars and troughs, presenting intense variations in the bathymetry related to sandbar movement (Álvarez-Ellacuría et al., 2011; Gómez-Pujol et al., 2011).

Tides are negligible (the tidal amplitude is less than $0.25\,\mathrm{m}$) although other surge components such as those induced by wind or atmospheric pressure can increase the sea level up to $1\,\mathrm{m}$ (Orfila et al., 2005). The beach is open to the East and, due to the semi-enclosed configuration, is well exposed to waves from the NNE to the SE (Enríquez et al., 2017). Significant wave height (H_s) at deep waters is usually bellow $0.9\,\mathrm{m}$ with a peak period (T_p) between $4\,\mathrm{s}$ and $7\,\mathrm{s}$, although frequent storms accounting 2% of time increase H_s up to $5\,\mathrm{m}$ with a T_p higher than $10\,\mathrm{s}$, with a return period of 1.5 years (Tintoré et al., 2009).

Cala Millor is one of the most important tourist resorts created in the Eastern coast of Mallorca –more than $60,000$ visitors during the summer period– and with a long history of sand nourishment and coastal management approaches (Tintoré et al., 2009).

Since November 2010 the Balearic Islands Coastal Observing and Forecasting System (SOCIB) is monitoring Cala Millor by means of coastal video monitoring, moored instruments and a periodic program of beach profile and sediment characterization (Tintoré et al., 2013). Along Cala Millor beach, over short temporal scales, shoreline position changes are not always homogeneous (Fig. 2, a) and it is possible to appreciate some different behaviors and responses to the wave climate. Cala Millor has experienced at least 19 events with significant wave height at $25\,\mathrm{m}$ depth over $2\,\mathrm{m}$ between November 2010 and

January 2017 (Fig. 2, b). Some of these events are isolated storms (e.g., April 2013) while others act in groups (e.g., January 2015). Fig. 2,a shows the alongshore anomaly of shoreline distances for the period between November 2010 to January 2017. The correlation between beach face response and sea conditions is not clear: there are storms that, even though Cala Millor is not a pocket beach, are giving rise to apparent temporary rotation, whereas others appear as a general shoreline advance or retreat. Nevertheless, from the averaged alongshore shoreline width anomaly (Fig. 2, c) it can be inferred a clear change in beach behavior since April 2014, just after a group of storm events that will be analyzed below. Despite the beach eventually recovers the former alongshore width, it is observed a net shoreline recession.

In March 2014, just few days before the storm group event, a field experiment was carried out in Cala Millor in order to characterize the beach morphology. This experiment produced detailed bathymetries and beach profiles were measured before the storms and also wave recorders were installed at different depths. Later, in June 2014, it was carried out another detailed beach survey and bathymetry belonging to the SOCIB's periodic beach monitoring program (Tintoré et al., 2013). Unfortunately, even though the April 2014 storm group seems to be critical for the beach width evolution, there are no bathymetric data available immediately after the storms. Nevertheless, the amount of available data before and after the storm group impacts makes this an opportunity to validate and generate numerical proxies that contribute to unravel the beach response to the storm group.

3 Data and methods

This paper partially deals with datasets produced during the Riskbeach experiment, performed by the SOCIB, the Mediterranean Institute for Advanced Studies (IMEDEA) and the Institute of Marine Sciences (ICM-CSIC) in Cala Millor from March 17th to March 26th 2014. This experiment was designed to study the response and recovery of an intermediate beach to usual (one year return period) storm conditions and the related sediment transport processes and morphological changes. During the experiment some instruments, detailed in Fig. 1, were installed in a central section of the beach to obtain high resolution sediment and hydrodynamical data. In this paper we employ the wave and currents recorder data (Acoustic Wave And Current Meter, AWAC) moored at $25\,m$ depth. Measurements are completed with bathymetric surveys, sediment samples and videomonitoring products. After the experiment (just from March 26th) large waves resulted in a significant morphological change of the beach, once the field survey was finished and the echosounding equipment was dismantled. To assess the effects of these storms we combine numerical modelling with videomonitoring techniques to infer the beach profiles that help us to understand the changes in the beach morphology before and after the storm group.

Fig. 1 and Fig. 3 summarize the approach developed in this study showing which data are from different instrumental approaches (i.e., direct measurements from bathymetric and DGPS-RTK surveys) and which ones inferred from numerical modelling and video images (indirect measurements). According to Fig. 3, field wave, sediment and beach morphology data, before storm event, are required in order to start up numerical model tools. The obtained results when field campaign data are available, have to be validated with field bathymetric data. The numerical model validation ensures that the results obtained during the storm period are bearable as accurate. In addition, the acquired product by video-monitoring, once the cameras have

been calibrated with field bathymetric data, will provide the "proxy" of the measured data. Results will be organized in two sections: first, profiles obtained by direct methods and, second, the results related to the use of these data sources for unraveling the beach erosion and recovery time scales.

we have wave mooring data that we use, through statistical analyses, in order to describe the wave climate and the storms that occurred in Cala Millor. We also have bathymetric data, obtained with DGPS-RTK and echosounding beach surveys. With the wave climate parameters, the bathimetric initial data of the beach and the grain size distribution (taken with sediment sampling), we can simulate the situation of the Cala Millor beach in the XBeach model. The obtained results must be validated with field bathimetric data during the period of time that we can recollect them. When the field campaign will be impossible, we will be able to know the situation of the beach thanks to the simulation of the XBeach (once it had been validated). In addition, we can have another source of data, as the video-monitoring. Through image analysis we can obtain the beach profile. Once this tool will be calibrated and validated with the other ones, it will act as an independent technique in order to know the state of the beach.

In this way, we can obtain an approximation of the sediment mass balance and the erosion and recovery time-scales of the beach.

3.1 Wave conditions

Offshore wave conditions (significant wave height, H_s, peak period T_p and wave direction at 50 m depth every three hours) are obtained from a reanalysis of a 60 years wave model output produced by the Spanish Harbor Authority (http://www.puertos.es/es-es/oceanografia/Paginas/portus.aspx). The mean H_s for the period of study is $0.9\,\mathrm{m}$ with a mean peak period (T_p) of $6\,\mathrm{s}$. During the experiment (March 17th to March 26th 2014), wave conditions were measured with an AWAC-system moored at deep waters ($25\,\mathrm{m}$ depth) in the central part of the beach.

Deep waters wave conditions show three storms during the period of study (Fig. 4). We define here storm as sustained wave conditions during at least 6 hours with $H_s > 1\,\mathrm{m}$. Gómez-Pujol et al. (2011) suggested this threshold as the condition required to generate a significant impact along beach morphology and sediment properties. When such an event is not isolated but becomes a succession of events, we are referring as a group of storms. These episodes can cause stronger damages in the beach with smaller wave heights, since the beach does not have enough time for recovering its initial morphodynamic state. The experiment started on March 17th after a period of moderate conditions with H_s close to 1 m that did not result in significant morphological changes. The first storm, S1 (see Fig. 4,a), occurred on March 26th, just after the instruments were moved away, with a maximum significant wave height $H_s = 1.5\,\mathrm{m}$ and $T_p = 9.9\,\mathrm{s}$ from the SE (Fig. 4,c) and a duration of 7 hours. The second storm, S2, beginning on March 28th, lasted 53 hours and peaked during the evening of March 29th with a maximum H_s of 3.4 m and T_p of 10.4 s. The estimated return period for S2 storm is around 1.2 years. Nevertheless, the return period just refers to the significant wave height threshold, despite the storm duration and persistence of wave height was 38 hours with $H_s > 2\,\mathrm{m}$ which is unusual. Wave conditions started to build up again on April 2nd 2014 after a short period of relatively small waves ($H_s < 1\,\mathrm{m}$). The third storm, S3, April 2nd to 3rd, peaked 4 days after the former storm with maximum

H_s of 1.3 m m and T_p of 7.8 s (Fig. 4,a and Fig. 4,b) during 48 hours. The following two months were characterized by mild conditions wich will be used to study the beach recovery after the storm groups.

3.2 Beach morphology

The topographic surveys were performed from March 17th to March 26th using a DGPS-RTK with submetrical resolution (having a horizontal accuracy around 8 mm and a vertical accuracy around 15 mm) for both the aerial (the area located over the mean sea level) and the submerged beach (from deep waters up to 1 m depth). Additionally, for submerged beach, bathymetric data was obtained using a Biosonics DE-4000 echosounder with a DGPS which allowed dense mapping from 0.5 to 10. m in depth. On March 17th, an initial bathymetry was acquired. Besides, 9 cross-shore profiles were taken daily between March 18th and March 26th (see Fig. 1). An additional bathymetry was performed on June 12th for control purposes. Elevations were referenced to the Balearic Islands Ordinance Survey mean sea level and the horizontal position referenced to UTM coordinates systems (Gómez-Pujol et al., 2011). These data covers the area between the boulevard sea wall and the lower shoreface (ca. 8 m in depth).

3.3 Sediment characteristics

Sediment samples were collected from aerial beach (+2 m) to 6 m depth at one of the central cross-shore transects (profile #07, Fig. 1). Sediments in the aerial beach and up to 1 m depth were collected by dragging on the bottom a plastic bag inserted in an oval metallic frame being their vertical penetration about $2 - 4$ cm and for greater depths, throwing a clamshell bucket from a boat. The weight of samples ranged from 200 to 500 g. After collection, samples were soaked in fresh water for 4 hours and drained before being dried for 24 hours. Sediment were analyzed using a laser granulometer and grain size obtained through the method described by Folk and Ward (1957) using Gradistat software (Blott and Pye, 2001).

3.4 Video monitoring

Coastal monitoring using video images is a practical and widely used technique since the advent of Argus (Holland et al., 1997). Since then, several systems (Cam-era, Horus, Cosmos, Beachkeeper, Ulises, etc.) mimic the Argus philosophy with the objective of providing continuous measurements of coastal processes in a non-supervised and autonomous procedure. Here, we use one of such approaches, SIRENA/Ulises (Nieto et al., 2010; Simarro et al., 2017), which has been operating since 2009 in Cala Millor. The system is composed of five charge-coupled device (CCD) cameras connected to a server acquiring daily images (Gómez-Pujol et al., 2013). The five cameras encompass an alongshore distance of around 1.7 km, largely including the monitored area. We use the timestacks, consisting of pseudo-images built with all pixel observation taken at 7.5 Hz at a predefined cross shore transect during the first 10 minutes of each hour, to infer the beach profile with the inversion of the wave dispersion relationship. The underlying idea in the inversion method is that the wave speed for progressive waves can be measured from its visible signature at consecutive snapshots to estimate the bathymetry using linear wave theory at the observed cross-shore transect (Stockdon and Holman, 2000).

Adopting the linear wave theory, the wave celerity c for shallow water waves ($kh < \pi/10$ where k is the wave number and h the local water depth) is

$$c^2 = g \cdot h, \tag{1}$$

where g is the gravitational acceleration.

Timestack images (Fig. 5,a) are pre-processed to convert the RGB data to a tractable intensity matrix. First, original times-tacks, with spatial and temporal dimensions $(n_x, n_t) = (650, 4500)$, are re-sampled by removing pixels at the aerial beach as well as at the outer domain (intermediate waters) where each pixel corresponds to large distances being not useful to measure hydrodynamic processes. Final images have spatial and temporal dimensions of $(\hat{n}_x, n_t) = (460, 4500)$. A quadratic filter with a time window of $3\,\mathrm{s}$ is applied to smooth the intensity timewise, and for each cross-shore position the temporal mean sub-tracted. From the intensity matrix $I(x,t)$, the wave frequency is obtained as the main component of the FFT in the time domain which is constant along the cross-shore dimension. A FFT is performed for each of the 460 cross-shore time series and the wave frequency, f, found as the mode of all resulting peaks (Fig. 5b).

Once f is known, the spatial component of the wave phase function (Fig. 5b), is evaluated following (Stockdon and Holman, 2000) as,

$$\phi = \arctan\left\{ \frac{\mathrm{Im}(\mathrm{I(x,\omega)})}{\mathrm{Re}(\mathrm{I(x,\omega)})} \right\}, \tag{2}$$

and the wave celerity obtained as,

$$c = \frac{2\pi f}{\partial \phi / \partial x}. \tag{3}$$

The beach profile is finally obtained from Eq. (1).

3.5 Numerical modelling

Morphological evolution is assessed using the XBeach (eXtreme Beach behavior) model (Roelvink et al., 2009), which re-solves the hydrodynamic processes of both the short waves (refraction, shoaling and breaking) and the long waves (generation, propagation and dissipation).The used version is the 4920 for 64 bits supplying mpi and netcdf. The model has been exten-sively validated with laboratory data as well as with field observations to study the morphological response of beach and sandy dunes, mostly under storm conditions. Here, we apply the model to analyze the storm group period with the surfbeat mode that resolves the 2D averaged equations.

The initial bathymetry (of March 17th) is discretized in an orthogonal rectangular grid evenly spaced with a resolution of $\Delta x = 7.44\,\mathrm{m}$ in the cross-shore direction and with $\Delta y = 15.86\,\mathrm{m}$ in the along-shore direction. Hourly JONSWAP spectra, generated through the measured data with the AWAC at $25\,\mathrm{m}$ depth, are propagated from the seaward boundary to the coast for the period of March 17th to April 8th, after S3 (summing up 528 runs of one hour of real time). The seaward boundary is imposed as absorbing-generating (weakly-reflective) boundary condition and the lateral boundaries as Neumann-type where the

longshore gradients are set to zero. The incoming wave directions in almost all simulations come from the East perpendicular to the shoreline (Fig. 4c).

Sediment characteristic measured before the experiment (D_{50} and D_{90}) are interpolated along the sampled profile and then they are extrapolated along-shore according to the depth of each grid point. The dimensionless porosity of the sediment is set to 30% and the density considered as $2650\,\mathrm{kg/m^3}$.

4   Results and Discussion

4.1   Bathymetry extraction from model and video images

The analyses based on XBeach and on timestacks are used to obtain the bathymetry and beach profiles to address changes in sediment mass balance. The initial bathymetry was measured before the storms (March 17th). The numerical model is run for the period between March 17th to April 8th, as stated. For each day a model derived bathymetry is obtained and 9 profiles extracted at the same locations of the measured cross-shore profiles. Table 1 shows the error parameters between measured profiles and the XBeach modeled profiles from March 17th to March 26th. The computed error parameters are the correlation coefficient (R^2), the Scatter Index (SCI) normalized with the maximum of the rms of the data and the absolute value of the mean of the data; and the Relative Bias (RB) normalized in the same way as the Scatter Index, used in Roelvink et al. (2009):

$$R^2 = \frac{\mathrm{Cov(m,c)}}{\sigma_{\mathrm{m}}\sigma_{\mathrm{c}}}, \tag{4}$$

$$SCI = \frac{\mathrm{rms_{c-m}}}{\max(\mathrm{rms_m}, |\langle m \rangle|)}, \tag{5}$$

$$RB = \frac{\langle c - m \rangle}{\max(\mathrm{rms_m}, |\langle m \rangle|)}, \tag{6}$$

being m the field data and c the modeled results.

The profiles derived from the model compare well with the measured ones from the aerial beach ($h = 2\,\mathrm{m}$) to the depth of closure ($h = -7\,\mathrm{m}$, according to Hallermeier (1981) formulation). Being the minimum R^2 of 99.31%, the maximum SCI of 0.11 and the maximum RB of 0.03 in the central profile. Therefore the modeled bathymetries (XBeach) can be considered as an efficient and reliable tool for unravelling the beach storm effects.

As an additional source of data, a cross-shore seabed profile in the SIRENA/Ulises central camera (Fig. 6) is obtained following the above described methodology. Table 2 compares the cross-shore profiles derived from timestacks against the instrumental measured profiles for the period between March 19th to April 26th (there are not timestacks available for March 17th and 18th). Since the timestack is defined in a cross-shore transect located between profiles #07 and #09, in situ measurements are daily interpolated to the timestack transect for comparison purposes. Error parameters from in situ measurements

and from video images are shown in Table 2, having R^2 a value of 97.95%, SCI of 0.14 and RB of 0.04. The largests differences tend to be located at deep profile positions where the model is known to perform worse since the accepted assumption on Eq. (1) only is valid for shallow waters. In general, there is a good agreement between both sets of data. Both comparisons, XBeach vs. instrumental and timestack vs. instrumental, present the same order of magnitude than the obtained in Roelvink

5 et al. (2009). This allows us to compare beach sediment mass-balance before and after the storm group as well as during the longer period of calm after the storms using different datasets and different techniques. This would allow a correct management of the beach avoiding unnecessary engineering works between touristic seasons.

4.2 Beach morphological response to storms and recovery

Although the individual storms are not exceptional in terms of intensity, their occurrence as a storm group has a significant

10 imprint on the beach morphology. The initial bathymetry, performed before the storm group (on March 17$^{\text{th}}$ 2014), shows a sinuous-parallel and patchily bar at $-1\,\text{m}$ and a cross-shore profile with attenuated secondary forms with a mean slope of 2.6%, whereas the bathymetry obtained for April 8$^{\text{th}}$ 2014 from XBeach shows a marked dissipative configuration. This is consistent with the obtained timex through SIRENA video monitoring station (see Fig. 7). The seabed variation after the storm group (S1, S2 and S3, in Fig. 4,a) is presented in Fig. 8,a. This morphological change is obtained as the difference between the bathymetry

15 obtained with XBeach after storm S3 (April 8$^{\text{th}}$) and the initial bathymetry. The effect of consecutive storms is to erode mainly the aerial beach mobilizing the sediment from the berm to depths between $-1\,\text{m}$ and $-5\,\text{m}$ forming a bar (around $100\,\text{m}$ from the shoreline, Fig. 8,a). The sediment mobilized to the bar is around $2.69 \cdot 10^4$ m^3 and coming from the aerial beach, where the volume loss is estimated as $3.01 \cdot 10^4$ m^3. This approximation of the sediment transport is calculated as the variation in depth at each spatial grid point between the initial bathymetry on March 17$^{\text{th}}$ and the simulated bathymetry for April 8$^{\text{th}}$. All

20 gridpoints are finally summed to obtain an approximated value of the sediment transport. The same methodology is applied to determine the sediment volume during the recovery period, but in this case the initial bathymetry is the simulated by XBeach in April 8$^{\text{th}}$ and the final one is the one measured during June 12$^{\text{th}}$. The redistribution can be also examined by analysing the profile at the center of the beach using video images. Fig. 9,a shows the beach profile change using video images from March 19$^{\text{th}}$ (the selection of the 19$^{\text{th}}$ is made since no images are available for the previous days) to April 8$^{\text{th}}$ (after S3).

25 Deepening in the beach response to the storm group, we analyze the differences between the initial bathymetry (March 17$^{\text{th}}$ 2014, previous to S1) and the bathymetries after storms S1, S2 and S3 (March 28$^{\text{th}}$, April 1$^{\text{st}}$ and 8$^{\text{th}}$ respectively) obtained from XBeach. Fig. 10 shows the differences, i.e., the impact of each of the storms. The first storm, S1, with moderate H_s and short duration, produces erosion at the beach face (volume loss of $1.18 \cdot 10^4$ m^3) accumulating large volumes of sand between $-1\,\text{m}$ and $-2\,\text{m}$ (not shown in Fig. 10). During the second storm, S2, which is the most energetic the beach-face suffers a

30 new episode of intense erosion, with depth variations between $1\,\text{m}$ and $1.5\,\text{m}$ and moving the bar offshore (Fig. 10,b). The gain in volume in the bar zone is around $1.51 \cdot 10^4$ m^3. Finally, the third storm (S3), with moderate wave heights but with large duration, continues eroding the aerial beach with little changes in the submerged beach (Fig. 10,c). This indicates that a sequence of storms does not necessarily result in cumulative erosion, supporting previous findings by Birkemeier et al. (1999)

and Coco et al. (2014). The eroded sediment that is transported offshore but not lost has the capacity to modify the cross-shore morphology and promotes the wave attenuation contributing to the sediment transport feedback.

The three-dimensional beach response to three successive storms highlights the importance of the storm duration in the sedimentary budget. This has been recently addressed in different studies (Ruiz de Alegria-Arzaburu and Masselink, 2010;
5   Vousdoukas et al., 2012; Coco et al., 2014; Senechal et al., 2015, among others) and particularly for the Mediterranean by Jiménez et al. (2008). This scenario fits with the usual 'storm-post storm' behavior model (Stive et al., 2002; Archetti et al., 2016) and highlights the need of more research, especially in the physical description and numerical modeling, in order to improve our knowledge of the characterization of the temporal scales associated with the beach sedimentary budget. Here, we found evidences that recovery times, jointly with antecedent morphology, play a crucial role in shoreline and beach dynamics
10   as stated by Senechal et al. (2015) or Jara et al. (2015).

After S3 storm the beach is under relatively calm conditions. A new bathymetry was done on June 12th 2014 allowing us to address the behavior of the beach during this period. Fig. 8,b shows the differences between the bathymetry at June 12th and the post-storm bathymetry obtained with numerical modeling for April 8th 2014. As seen, two months after the storm group, there is an opposite scenario. The sand reservoir below feeds up the shore-face again, but also redistribute sediment along the
15   beach at different depths. The sand volume recovered at the aerial beach during this period is $1.58 \cdot 10^4$ m^3 that is half of the volume lost during the storm period. This behavior is confirmed from the analysis of the beach cross-shore profile obtained from the timestack video image. Fig. 9,b shows the difference between the summer profile (June 12th 2014) and the beach profile after S3 (April 8th 2014) supporting a recovery of the upper part of the beach.

The proposed approach aims to be a tool to assist to the beach management especially during adverse conditions, when
20   field surveys are not possible. The combination of numerical models, video-monitoring and in situ data provide alternatives for the lack of data especially during adverse conditions. This approach follows the change in the paradigm in ocean studies where multiplatform approaches are being developed abroad the globe in order to fill spatial and temporal gaps in the measured time-series.

In the studied beach, the results show that the beach is able to recovery the lost sediment in a larger scale than the erosion
25   and that is crucial to know the beach configuration at any time in order to know its evolution in front specific wave climate episodes.

5   Conclusions

The response of a low energy microtidal beach in front of storm groups on time scales related to processes of beach erosion and accretion is studied. For this purpose, different techniques and approaches including DGPS-RTK and bathymetry surveys,
30   modeling and video monitoring are combined. The observations confirm that the previous morphological conditions are crucial for controlling the sediment exchange and the morphological response of the beach.

Focusing on the effect of individual storms, the first one mobilizes sand mostly from the aerial area generating a parallel bar at depths ~ 1 m modifying the beach profile from near reflective to more dissipative. The effect of S2, lasting for more than 30

hours, is to mobilize a large volume of sediment redistributing the profile along all the beach and generating a large submerged sandbar at depths $\sim -2.5\,\text{m}$ ($\sim 100\,\text{m}$ from the shoreline). This profile shows to be very efficient in protecting the beach from the third storm, which has a duration of 48 hours, being the sediment mobilized during this event, almost negligible. The largest changes in sediment mobilization occur in the transition from the reflective to the dissipative states, when the beach adjusts its profile to the incoming wave conditions. The combined effects of this storm group confirm that in low energy systems as the one here analyzed, it is necessary to know the previous morphological state in order to properly assess the new beach conditions.

Results highlight the well known different temporal scales for the erosion and accretion in low energetic systems. While offshore sand migration is produced at storm timescales, the onshore sediment transport has a much slower characteristic timescale. In particular, a group of relatively energetic storms has the capacity to generate significant erosion in three days. Despite the moderate conditions and the lack of storms during the next two months only half of the sediment is recovered. In this study the recovery of the beach is not documented, neither in sediment mass balance nor in shoreline width. Nevertheless from Fig. 2,a it can be appreciated that the aerial beach remains relatively stable and slightly increase the beach width at the end of 2014. Then in December 2014 and early January 2015, a new set of storm groups events affect the beach and since then the beach shoreline width has not recovered former conditions, despite punctually there has been some advance in shoreline position.

Time recovery after storms is a key issue for local beach managers who are pressed by touristic stakeholders to nourish the beach after energetic process in order to reach the quality standards required by beach users. The combined use of remote sensing data, in situ observations and numerical models, should already be integrated in management tools to take short term decisions, as beach nourishment, based on reliable physical data.

Data availability. All data are accessible from http://www.socib.es.

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Table 1. Error statistics for the simulated profiles by XBeach compared with the measured profiles during Riskbeach.

Profile #	$R^2(\%)$	SCI	Relative bias
01	99.79 ± 0.08	0.07 ± 0.03	0.02 ± 0.04
03	99.77 ± 0.09	0.07 ± 0.03	-0.02 ± 0.03
05	99.48 ± 0.13	0.08 ± 0.01	0.01 ± 0.01
07	99.53 ± 0.08	0.09 ± 0.01	0.00 ± 0.03
09	99.31 ± 0.21	0.11 ± 0.02	0.03 ± 0.01
11	99.73 ± 0.18	0.06 ± 0.02	0.00 ± 0.01
13	99.72 ± 0.03	0.07 ± 0.01	0.02 ± 0.03
15	99.59 ± 0.49	0.08 ± 0.03	-0.03 ± 0.02
17	99.90 ± 0.04	0.04 ± 0.01	0.03 ± 0.02

Table 2. Error statistics for the estimated profile from timestacks compared with the measured profiles during Riskbeach.

$R^2(\%)$	SCI	Relative bias
97.95 ± 1.4	0.14 ± 0.07	0.04 ± 0.06
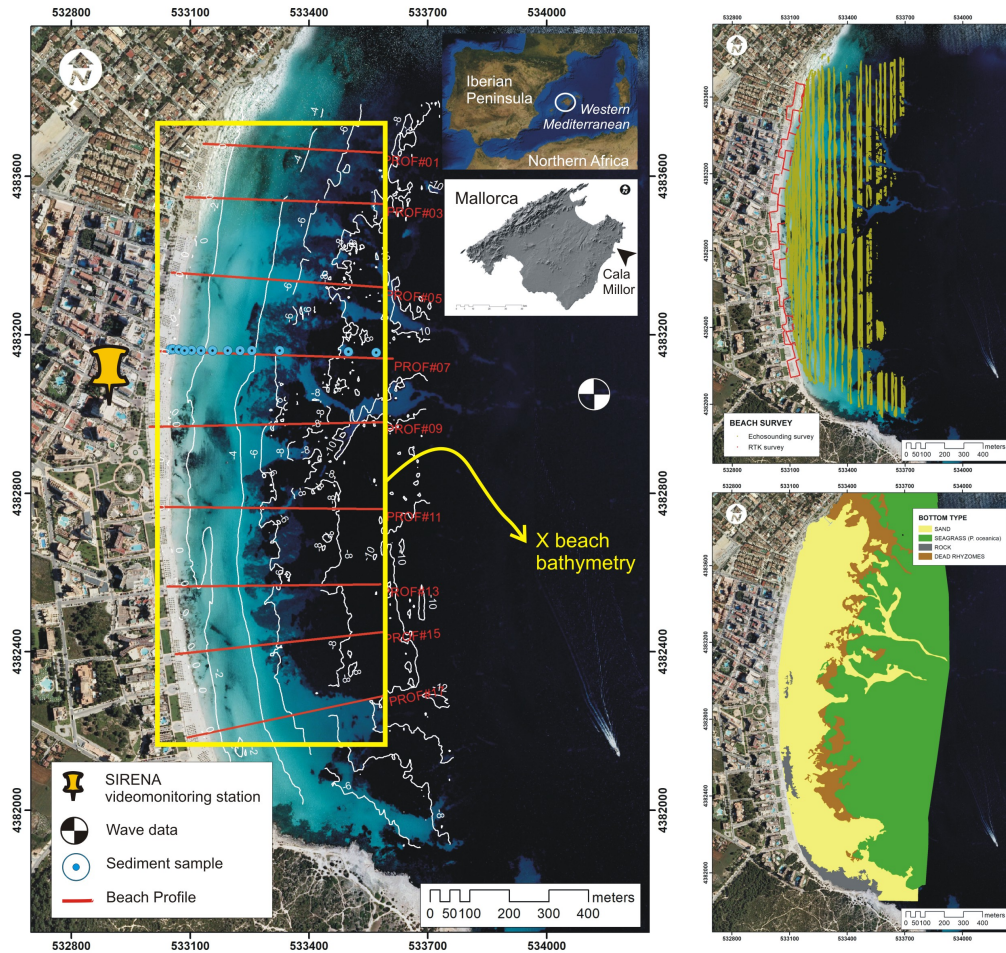
Figure 1. Study site location and major features of Cala Millor. Right panel: White dashed lines corresponds to bathymetric survey (isolines equistance 2 m); Yellow frame covers the bathymetry area obtained by means of Xbeach; and red lines to the beach profile described in text. The bottom ortophoto is provided by the Govern de les Illes Balears-SITIBSA (June 2008). Upper right panel shows the combination of multibeam bathymetric survey (green points) and RTK-GPS survey for dry beach and very shallow submerged beach (red points). Lower Right panel: bottom type at Cala Millor.
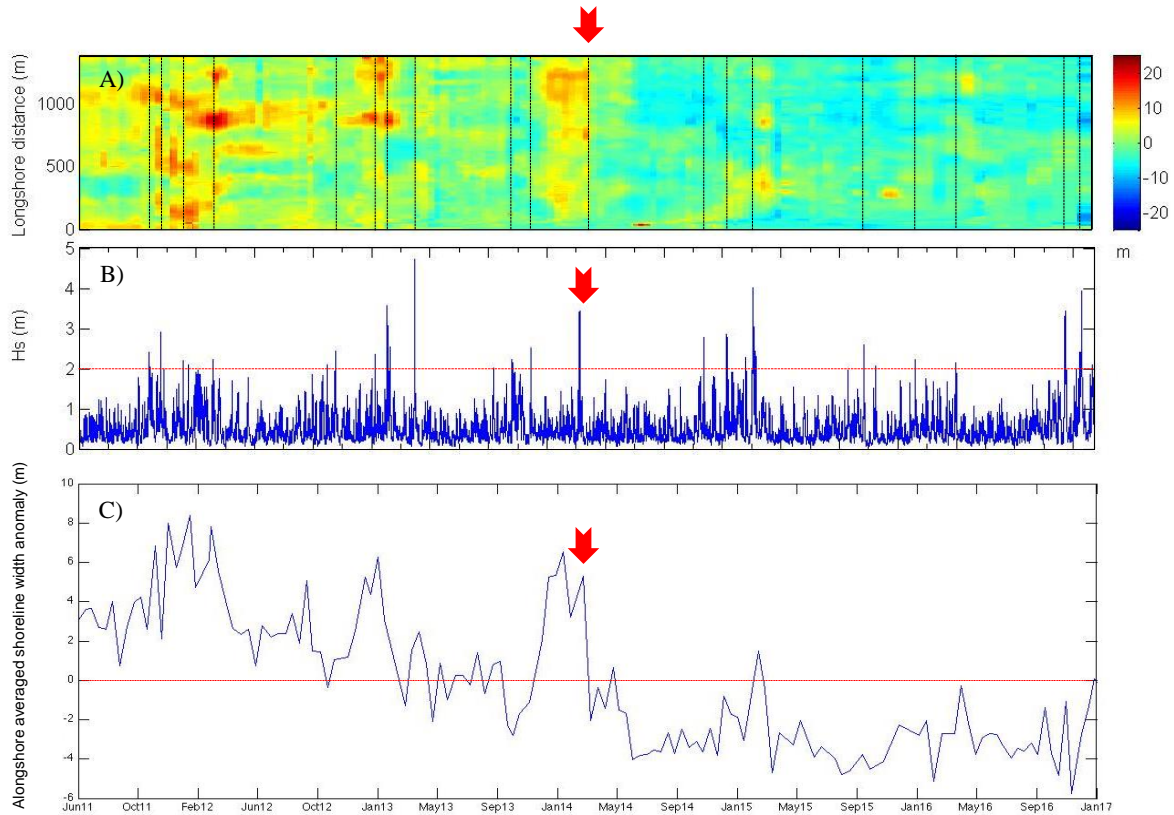
Figure 2. A) Alongshore shoreline width anomaly at Cala Millor from November 2010 to January 2017. Red colors indicate shoreline advance, whereas blue ones indicate shoreline recession. The dashed black lines show the sea storm events larger than 2 m. B) Wave significant height from a wave recorder located at -17 m in the middle of Cala Millor embayment. C) Alongshore averaged shoreline width anomaly at Cala Millor. The red arrows highlight the storm group event at April 2014.
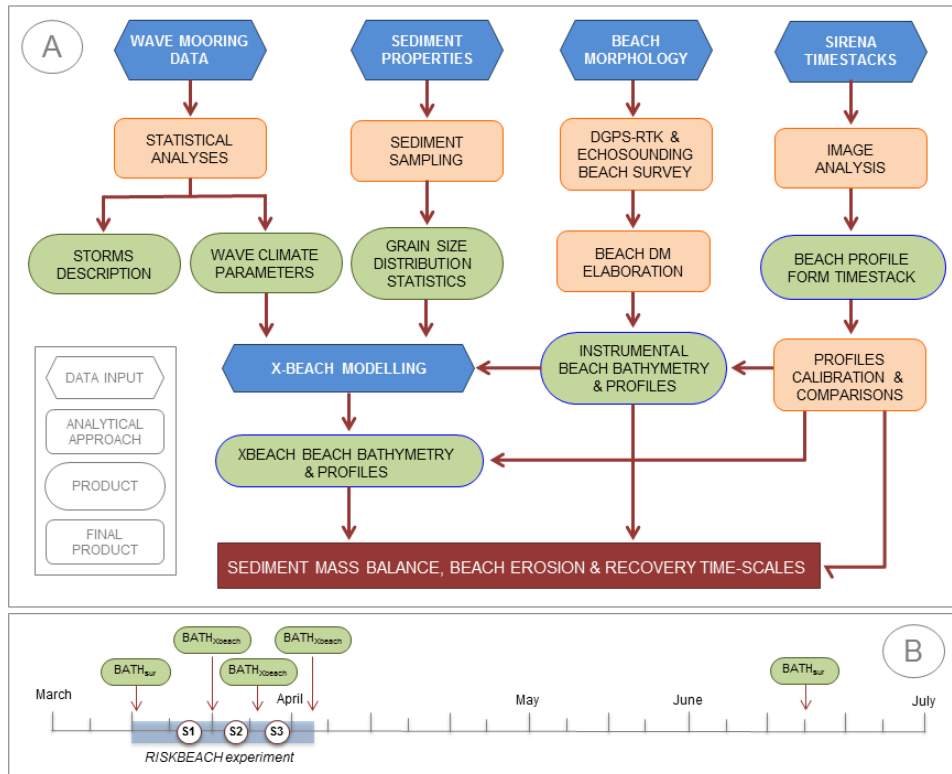
Figure 3. A) Workflow of the aproach followed in the study. B) Calendar showing the date for the samples used in the study.
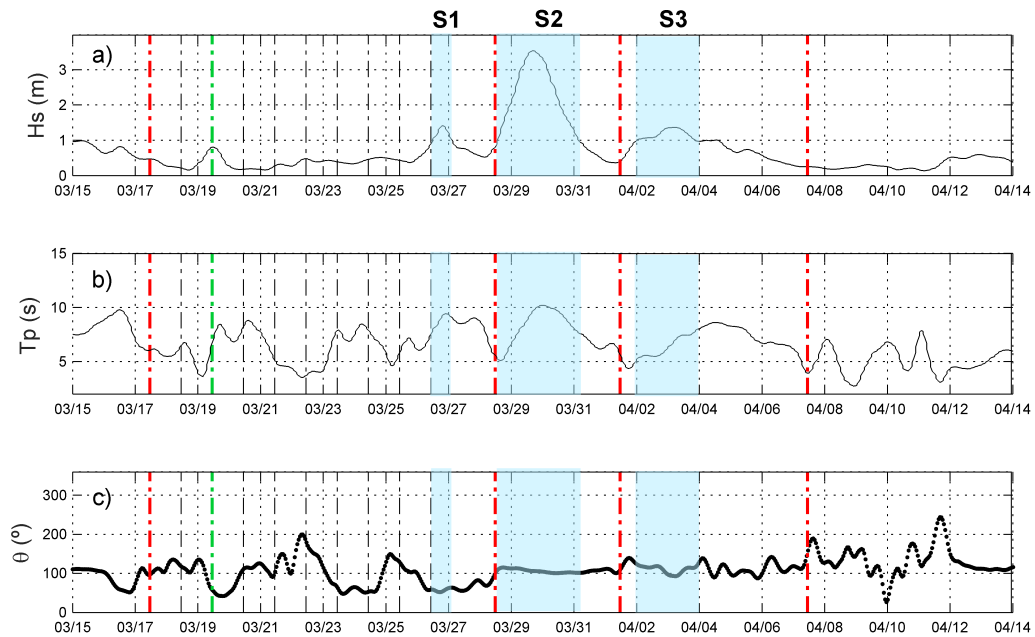
Figure 4. a) H_s (m) at $25\,\mathrm{m}$ in Cala Millor between March 15th to April 14th 2014; b) Tp (s) and c) Wave direction. The blue shading shows the period corresponding of the storms. Vertical red dotted line indicates the initial bathymetry obtained while dash dotted lines indicate the dates when cross shore profiles where measured. Vertical green dotted line states the day where the model was validated using the corresponding shore profiles. Vertical red lines show the date when bathymetry inferred from Xbeach is used for comparison betweem storms.
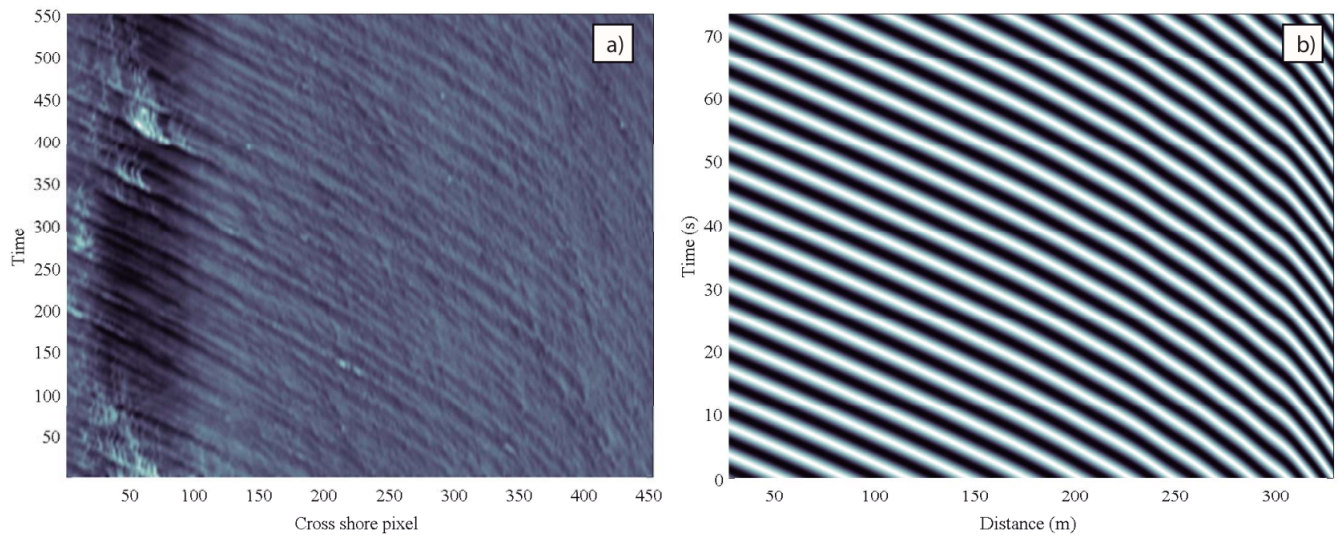
Figure 5. a) Timestack image for March 19th at 9.00 am for the central camera. The abscissa corresponds to the cross-shore direction and the ordinate for the time. b) Reconstruction for the same date assuming a constant wave height using the Fourier mode of the detected period (*i.e.* $\cos\left(\phi(x, f_w) - 2\pi f_w\right)$).
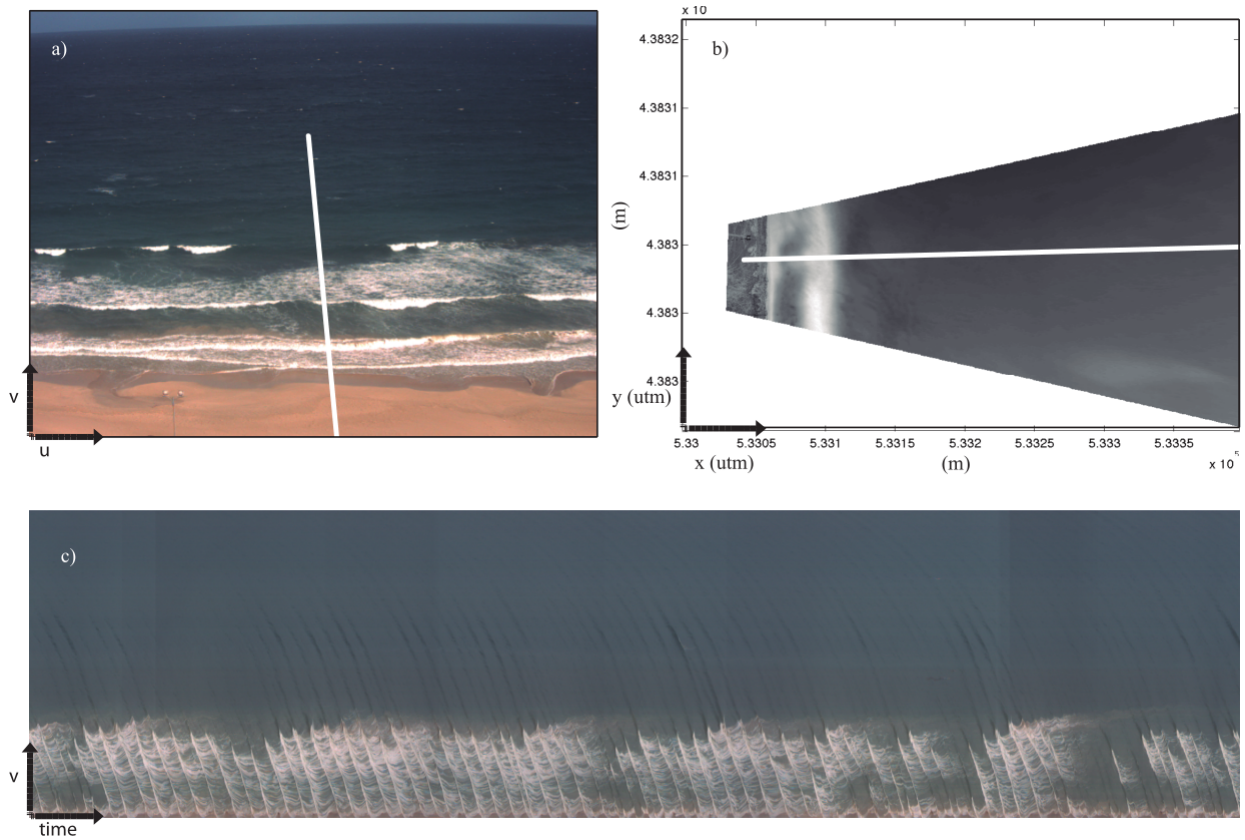
Figure 6. a) Cross-shore transect defined for the timestack image on camera #3. The Figure shows the original image in the (u, v) ≡ pixel coordinate system. b) The same after rectification in the (x, y) ≡ UTM coordinate system. c) Resulting timestack for March 19th at 10.00 am.
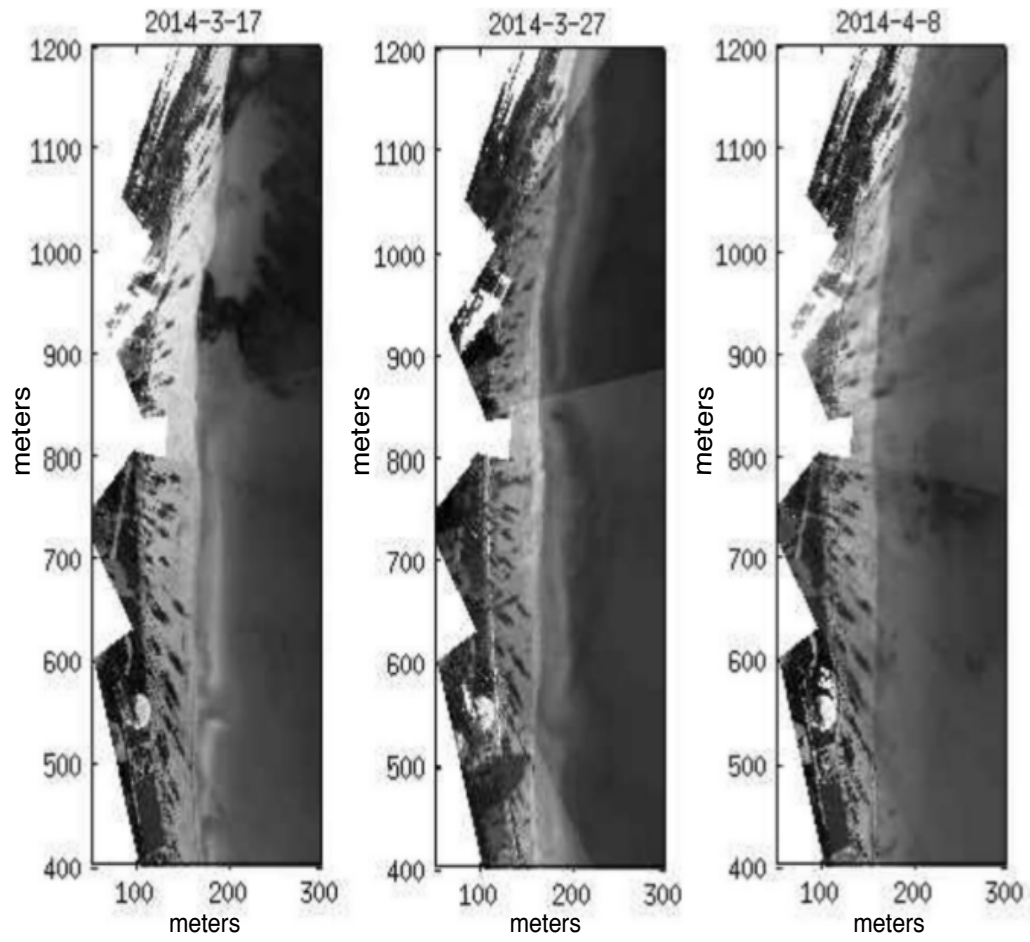
Figure 7. Timex images with dates referred in each image. Notice the intermediate configuration with a sinuous parallel bar along the coast (ca. 180 m) for March 17th and March 27th and the dissipative scenario without bar for April 8th.
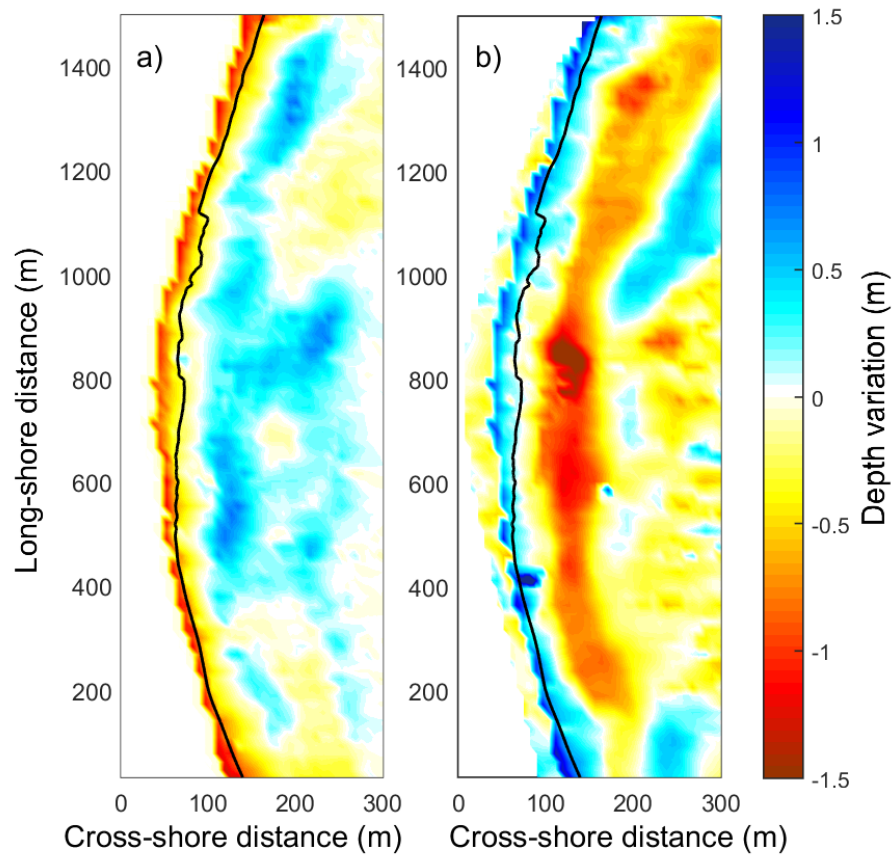
Figure 8. Depth variation estimated from XBeach and from measurements. a) Bottom variation during the storm group (March 17$^{\text{th}}$ to April 8$^{\text{th}}$). b) Bottom variation for the period of calms (April 8$^{\text{th}}$ to June 12$^{\text{th}}$).
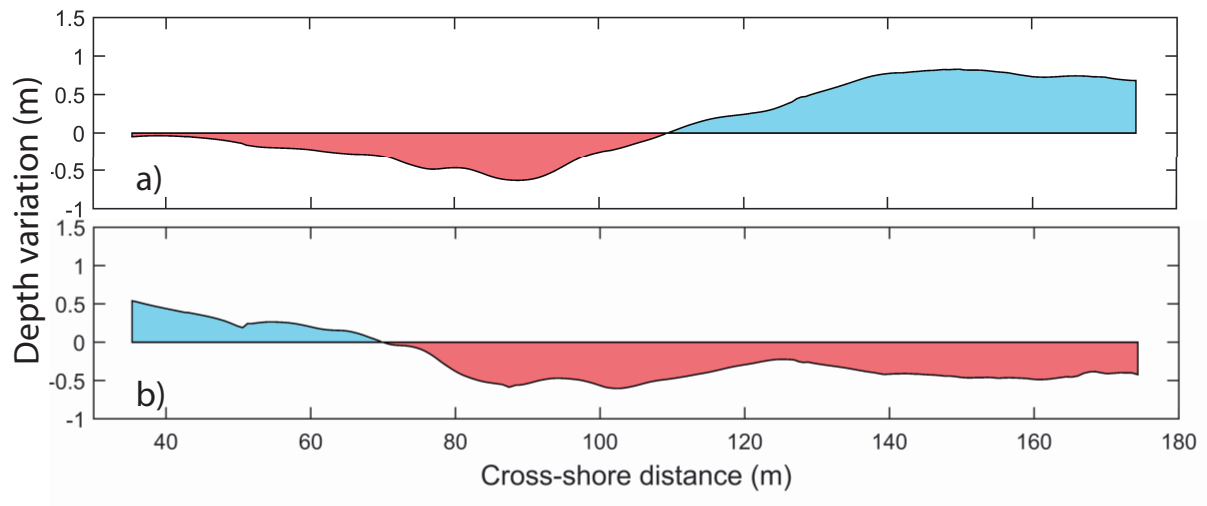
Figure 9. Depth variation estimated from bathymetry inversion of the timesatck during storm conditions; a) between April 8th and March 20th (storm conditions); b) between June 12th and April 8th (calm conditions).
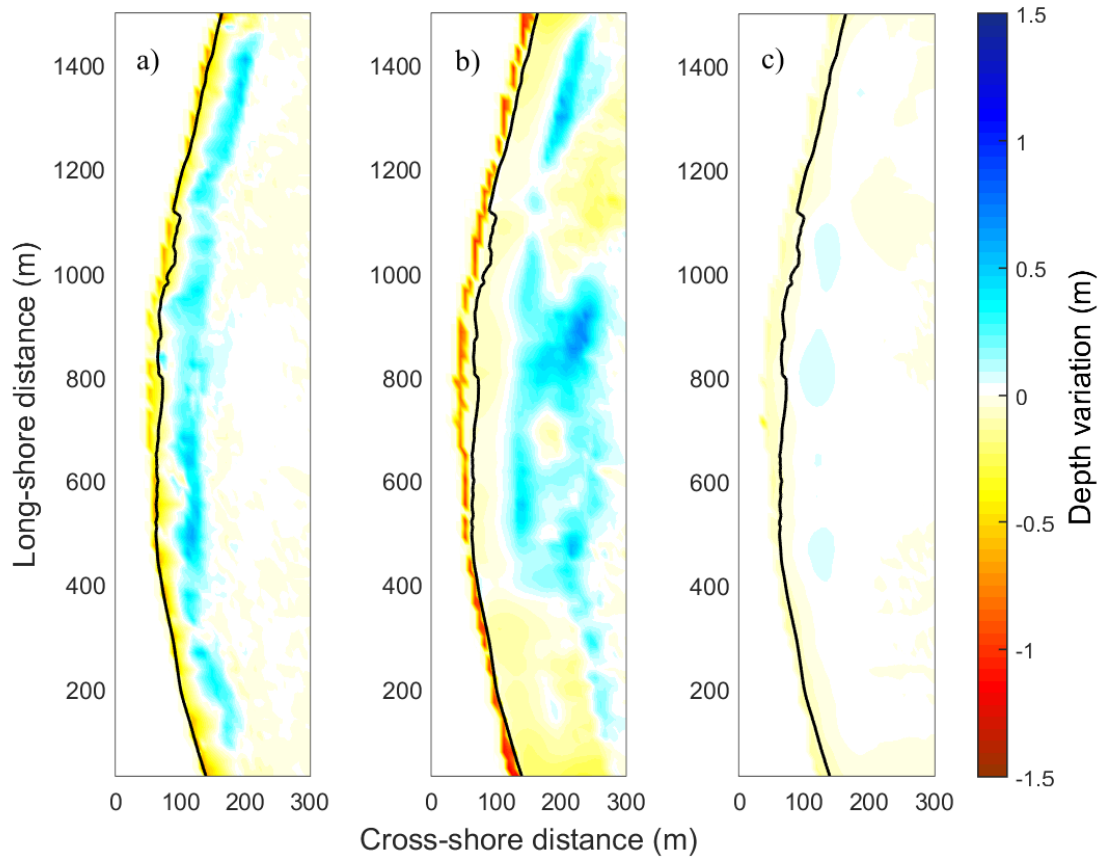
Figure 10. Depth variation estimated from XBeach and from measurements. a) Bottom variation between March 17th to March 28th (storm S1). b) Depth variation between March 28th to April 1st (storm S2). c) Depth variation between April 1st April to 8th (storm S3).