





41 the movement velocity and locus of rockfall, avoiding the damage of project. (Topal et al., 2006;  
42 Koleini and Van Rooy, 2011; Saroglou et al., 2012; Sadagah, 2015).

43 Based on the above research, the protection measures are put forward to control rockfall.  
44 The trees have a significant blocking effect on the rolling stones, the interception influence tests of  
45 trees on rockfall are designed based on the analysis of collision probability of trees and rockfall,  
46 and the velocity change, movement distance of rockfall and the collision probability between trees  
47 and rockfall are researched (Huang, 2010; Notaro, 2012). A large-scale field test of the impact  
48 caused by rockfall on reinforced concrete beams is conducted, the dynamic response process is  
49 studied and compared with the numerical simulation results (Kishi et al., 2002 and 2010; Bhatti et  
50 al., 2009 and 2010). Rockfall concrete barriers are classified as rigid barriers, they absorb most of  
51 the impact and all of the residual kinetic energy of the falling rock instead of dissipating it as  
52 flexible nets do. Experiences have shown that rigid walls have a tendency to break under  
53 high-impact loads, and shatter, sometimes violently (Badger et al., 2009). Because of their  
54 relatively small size, these barriers cannot contain large-sized rocks or high-energy rockfall.  
55 Concrete barriers are generally believed to be suitable for rockfall protection where the resulting  
56 impact energy is in the range of 60 kJ to 100 kJ or where catchment ditch effectiveness needs  
57 improvement (Descoedres et al., 1999). The method that setting short CFT members between  
58 pillar and cover plate in rock shed is proposed, and the deformation and energy absorption  
59 characteristics of the supporting member are studied through test and theoretical analysis  
60 (Delhomme et al, 2005; Mommessin et al, 2004). Combing the blocks' quality and drop height, a  
61 large number of experiment for different soil are carried out, the influence of soil characteristics  
62 on the impact response of rockfall are studied (Kawahara et al., 2006).

63 The above protection researches are mainly applicable to the conventional human  
64 settlements, and it is expensive and inconvenient to take these measures to control rockfall in  
65 open-pit mine. The energy consumption layer laid on the safety platform is a relatively common  
66 way to prevent and control rockfall in open-pit mine (Heierli et al., 1981; Labiouse et al., 1996).  
67 However the previous researches on cushion are seldom concerned with the effects of cushion's  
68 particle size on the movement characteristic of rockfall, especially for the joint effects of gravel  
69 cushion's particle size and thickness on coefficient of restitution (*COR*) have not been explored so  
70 far. During the process of mining, a large amount of mullock are produced, mullock can be broken  
71 into particle of different size through the crusher, which can be paved on the platform as energy  
72 consumption layer. A certain thickness of gravel cushion on the platform can effectively absorb the  
73 impact energy of rockfall to achieve a buffer effect, reducing the impact load caused by rockfall on  
74 the protective structure and the kinetic energy of rockfall, which makes the rockfall eventually  
75 resisted on the platform. Because the impact of rockfall and gravel cushion is short, it involves  
76 complicated elastic-plastic deformation and energy conversion, and the energy absorption  
77 performance of gravel cushion with different thickness and particle size are quite different under  
78 the rockfall impacts, how to determine the energy-consumption buffer mechanism of gravel  
79 cushion has become the key to the cushion design and calculate the following rockfall movement,  
80 so the effects of cushion's particle size and thickness on *COR* under the rockfall impacts should be  
81 furtherly studied to control the rockfall effectively.

82 .

## 83 **2 Coefficient of restitution**



84 It is difficult to predict a rebound locus, several parameters, such as the strength, roughness,  
 85 stiffness and inclination of slope and blocks, have obvious influences on the rebound locus of  
 86 rockfall (Labrousse and Heidenreich 2009), but the calculation method of rebound locus according  
 87 to the COR is widely used (Giani, 1992), and the definitions of COR are various (Chau et al.  
 88 2002).

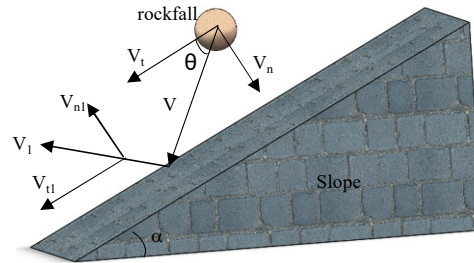


Fig. 1 Motion model of rockfall

89  
 90  
 91 For a block impacting a rocky slope (Figure 1), based on the theory of inelastic collision, the  
 92 coefficient of restitution (COR) is defined as Eq.1:

$$V_{COR} = \frac{V_1}{V} \quad (1)$$

94 Where  $V$  and  $V_1$  are the velocity magnitude of the incident and rebound stage of the locus,  
 95 respectively (m/s).

96 The  $V_{COR}$  consists of normal and tangential parts, and the normal ( $R_n$ ) and tangential ( $R_t$ )  
 97 coefficients are defined as Eq.2:

$$R_n = \frac{V_{n1}}{V_n} \quad R_t = \frac{V_{t1}}{V_t} \quad (2)$$

99 Where  $R_n$  and  $R_t$  are the normal and tangential restitution coefficients, respectively,  $V_n$ ,  $V_{n1}$   
 100 are the normal parts and  $V_t$ ,  $V_{t1}$  are the tangential parts of the block's velocity, before and after the  
 101 impact, respectively (m/s).

102 The blocks' total energy  $E$  of the block consist of the translational ( $E_0$ ) and the rotational ( $E_w$ )  
 103 energy (Eq.3), and the total energy coefficient (ETCOR) is proposed (Eq. 4):

$$E = E_0 + E_w = \frac{1}{2}mv^2 + \frac{1}{2}I\omega^2 \quad (3)$$

$$ET_{COR} = \frac{\frac{1}{2}mV_1^2 + \frac{1}{2}I\omega_1^2}{\frac{1}{2}mV^2 + \frac{1}{2}I\omega^2} = \frac{0.6mV_1^2}{0.6mV^2} = \frac{V_1^2}{V^2} = V_{COR}^2 \quad (4)$$

106 Where  $m$  is the block' quality,  $I$  is the block's inertia moment,  $\omega$  and  $\omega_1$  are the angular  
 107 velocity, before and after the impact, respectively.

108 When the dangerous rock-body breaks away from the parent body, it will inevitably generate  
 109 collision with slope during rolling process along the slope and lose the energy. The approximate  
 110 calculation formula for the total kinetic energy of rockfall is derived from engineering surveys  
 111 (Yang et al., 2005).

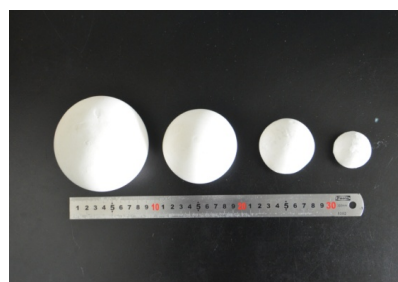
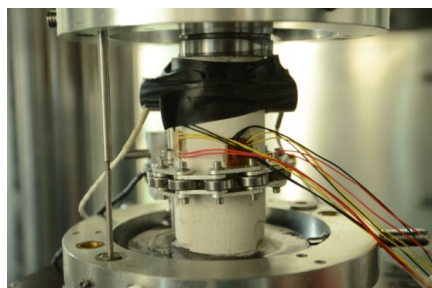
$$E = E_0 + E_w = 1.2E_0 = 0.6mV^2 = 0.6m(V_n^2 + V_t^2) \quad (5)$$

### 113 3. Experimental Studies



### 114 3.1 Experimental material and apparatus

115 In order to study the effects of cushion's particle size and thickness on COR conveniently  
116 under the rockfall impacts, the high-strength gypsum material are adopted to simulate rockfall in  
117 the test, the recommend value of sample's moisture content is in the range 30% to 50% in previous  
118 study (Chau et al., 2002). Spherical blocks with diameters of 2cm, 3cm, 4 cm and 5cm are made  
119 with a moisture content of 40% (Figure 3), for further research on the properties of gypsum  
120 materials, six standard cylindrical samples with a moisture content of 40%, which possess 5cm  
121 diameter and 10cm height, are tested to obtain the uniaxial compressive strength. The uniaxial  
122 compression test is shown in Figure 2. Due to the test error, the ultimate compressive strength of  
123 six samples is different, so the average value is considered as the compressive strength of the  
124 material. The average value when the specimens are destroyed is 6.48Mpa, indicating that the  
125 gypsum sample of present moisture content is enough to prevent shattering during the collision  
126 process (Ulusay et al., 2007; Aydin, 2009).



127  
128 Fig. 2 standard specimen under uniaxial compression test

129 Fig. 3 Sample of different sizes of spherical gypsum

130 In order to explore the effect of different thickness and particle size of cushion on the rolling  
131 motion of rockfall, massive gypsum boards made of same proportion as blocks are broken,  
132 gypsum particles groups with sizes of 0.2, 0.6, 1.0, 1.4, 1.8 and 2.4 cm are selected by sieve to  
133 simulate the gravel cushion, as shown in Figure 4.

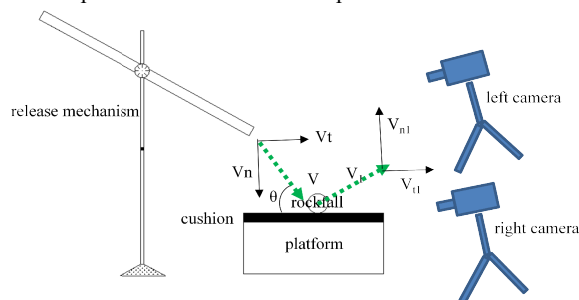


134  
135 Fig. 4 Screening granules of different particle sizes

136 A simple rolling stone releasing device is shown in Figure 5, a slant tube with adjustable  
137 inclination and height is used to adjust the impact translational velocity of blocks (Asteriou et al.,  
138 2012). The colliding blocks slide and roll through the tube to collide with the plate. Two  
139 synchronized digital cameras (1024\* 1024 pixels) are adopted in the tests to acquire the blocks'  
140 velocity in Stereoscopic space (Bouguet 2008; Asteriou et al., 2013). The cameras, which can  
obtain the motion, velocity, and kinetic energy automatically, are placed near the impact surface

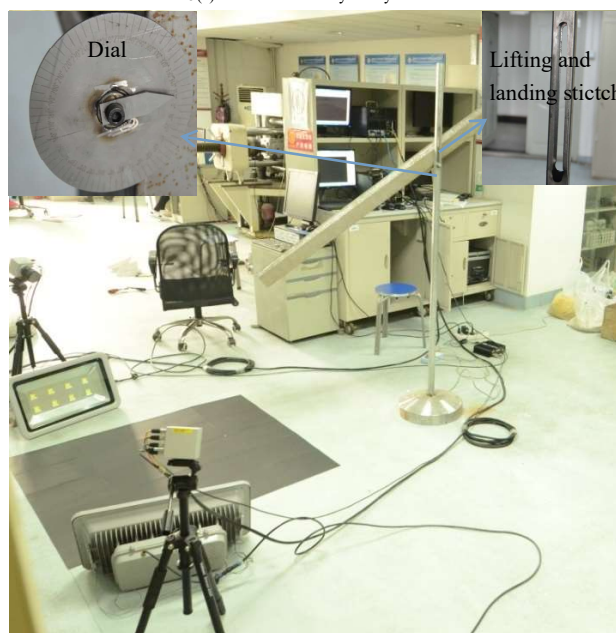


141 (see Figure 5), and the capture rate of camera is 200 fps.



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5(a) model of laboratory test system

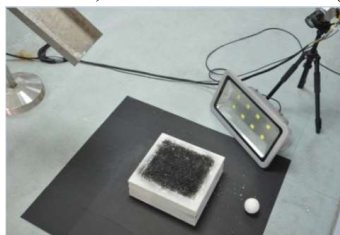


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5(b) laboratory test system

Fig. 5 The experimental apparatus

147 The collision surface is a plate made of the same material as the blocks. The plate is  
 148 40cm\*40cm\* 8cm in volume. For simulating different thickness of cushion, a great amount of  
 149 hollow gypsum board whose volume is 40cm\*40cm\* 2cm are made, and a part whose volume is  
 150 30cm\*30cm\* 2cm is cut in the middle of each board to be filled with the gypsum particle. In order  
 151 to accurately measure the speed of blocks by cameras, and avoid the interference of the motion of  
 152 cushion's particles affected by the collision, the cushion is blackened (see Figure 6).



153



154

Fig. 6 Laboratory rolling stones model test

### 155 3.2 Experimental procedure

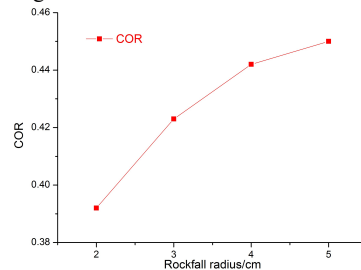
156 The spherical blocks of 2cm, 3cm, 4cm, 5cm radius (see Figure 3) are applied in laboratory  
157 test, and the falling blocks are released from 1.2m height, the effects of cushion's thickness and  
158 particle size and block volume on *COR* are studied in this experiment. The block is inserted into  
159 one side of tube, and after sliding and rolling through the tube to collide with the collision surface.  
160 The impact surface is a plate to simulate the platform before paving the cushion. After paving the  
161 cushion on the plate, for each series, the thickness is adopted as 2cm, 4cm, 6cm, 8cm, 10cm, 12cm,  
162 14cm respectively. The cushion's particle sizes are taken as 0.2cm, 0.6cm, 1.0cm, 1.4cm, 1.8cm  
163 and 2.4cm, respectively. In order to avoid the chance of test, "three tests for the mean" method is  
164 adopted, and the average value is set as the final results. In total, four series for 516 testing cases  
165 are carried out.

166 Meanwhile, in order to investigate the effect of rockfall released from different movement  
167 height on the *COR* of cushion, the experiments that blocks of 2cm, 3cm, 4cm and 5cm radius fall  
168 down from 0.4m, 0.8m, 1.2m and 1.6m respectively to collide with 8cm thickness cushion of  
169 different particle sizes are carried out, four series for 288 testing cases are carried out.

### 170 3.3 Experimental results and discussion

#### 171 3.3.1 The experiment results

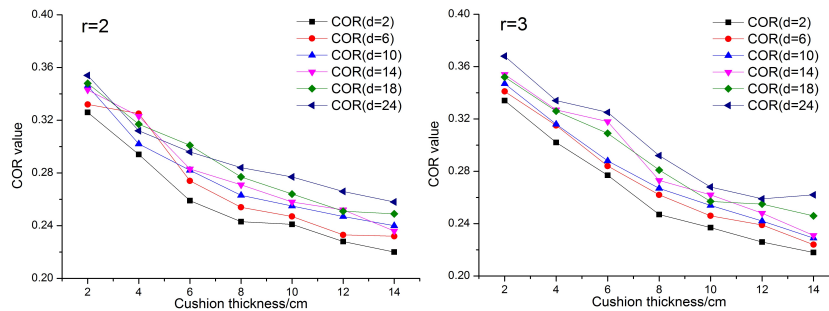
172 The blocks are released from 1.2m height to collide with the plate before paving the cushion,  
173 the results of *COR* are shown in Figure 7.



174

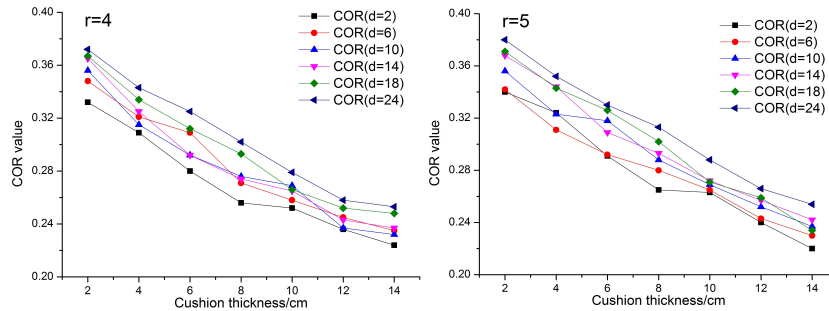
175 Fig. 7 The *COR* of different blocks' collision with the plate

176 After paving the cushion on the plate, the experiments that rockfall of different volume  
177 released from 1.2m movement height collide with various cushion and particle size of cushion are  
178 conducted, the results of which are given in Figure 8.



179





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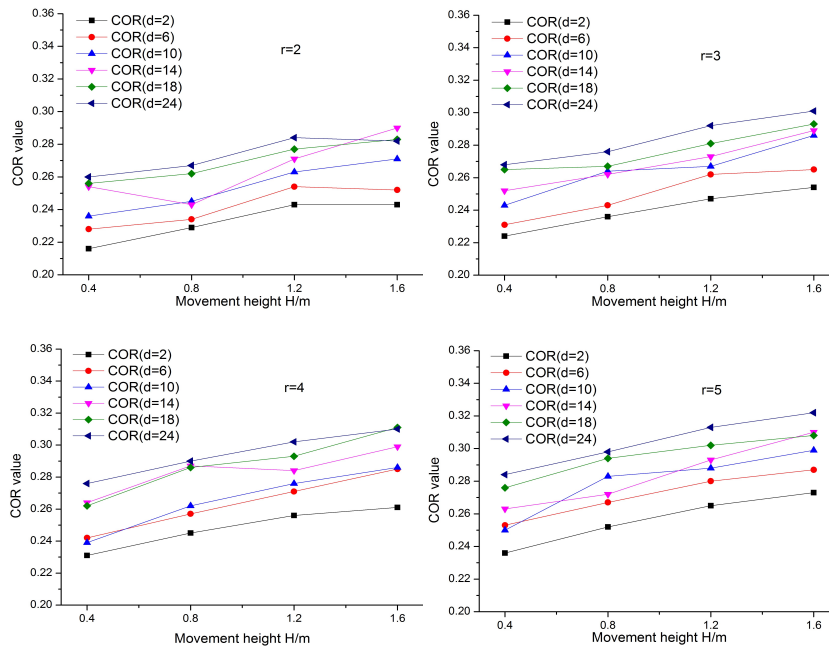
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Fig. 8 The COR comparison of different blocks released from 1.2m height  
 The tests that rockfall of different volume released from different movement height collide with 8cm thickness cushion of various particle size respectively, the results of COR are shown in figure 9.



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Fig. 9 The COR comparison of different blocks' collision with 8cm thickness cushion  
 3.3.2 The discussion

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From the above figures we can see that the cushion's thickness and particle size have a great influence on the COR of cushion, while the influence of rockfall radius is relatively low. When the cushion's particle size is small and thickness is great, the COR of cushion will be small, and its energy-consumption effects can be obvious. With the increase of rockfall's radius and movement height, the impact energy increases dramatically when rockfall colliding with cushion (Kawahara et al., 1998). Under the low impact energy, the change of cushion's thickness has a relatively low effect on the COR of cushion, and the cushion of small thickness also has certain energy-absorbing effect, which can be verified by Pei (2016) and Kawahara (2006). However, under the high impact energy, the energy-absorption effect of different thickness gravel cushion is obviously different.



198 Because the small thickness cushion can be compressed in a very short time, which make the  
 199 rockfall is more likely to be affected by the adamant platform, reducing the cushion's thickness is  
 200 equivalent to increasing the effective stiffness of the cushion, which limit the buffer and  
 201 energy-absorbing effect of cushion to a great extent. When the cushion's thickness is relatively  
 202 small, the *COR* increase significantly as the decrease of cushion's thickness. However, when the  
 203 cushion's thickness is relatively large, this trend is no longer obvious.

204 When the blocks are released from 1.2m, the *COR* is large before laying the cushion, the  
 205 *COR* decrease obviously with the increase of cushion's thickness after laying the gravel cushion,  
 206 which agrees with the observation given by Kawahara (2005), but when the cushion reaches a  
 207 certain thickness, namely, the ratio of the rockfall radius *r* to the cushion's thickness *h* is form 1/4  
 208 to 1/3, with the increase of cushion's thickness, the reduction rate of *COR* become low gradually.  
 209 As the decrease of cushion's particle size, *COR* is more sensitive to the cushion's thickness of  
 210 small particle size than the cushion thickness of relatively big particle size, the change range of  
 211 *COR* of small particle size caused by the variety of thickness is more wider, and as the increase of  
 212 cushion's thickness of big particle size, the *COR* of cushion change relatively slightly.

213 When the cushion's thickness is 8cm, as the movement height of block increases, the *COR*  
 214 also increases, but when the blocks of different radius colliding with the cushion of same thickness,  
 215 the *COR* change range of blocks of a big radius is larger than those blocks of a relatively small  
 216 radius. When the blocks move from a relatively low height, the *COR* of cushion is more likely to  
 217 be affected by the particle size compared with the blocks released from high height. As the  
 218 cushion's particle size is large, the difference of collision parts between the rockfall and cushion  
 219 are great, resulting in a wide range of *COR* of cushion.

## 220 4 Orthogonal test design

### 221 4.1 Orthogonal test procedure

222 To explore the influence degree of cushion's particle size and thickness on the *COR* when  
 223 rockfall moving through the cushion, orthogonal test theory is adopted to take test program design  
 224 (Tao et al., 2009). When these factors cannot be considered in full, the leading factor is considered  
 225 preferentially to achieve the expected effects to a great extent. The rockfall radius *r*, movement  
 226 height *H*, cushion's thickness *h* and particle size *d* four parameters are selected to be taken as the  
 227 basic factors of test. According to the characteristics of 4 factors, the number of level of every  
 228 factor is 4, as shown in Table 1:

229

Table 1 All factors and levels of orthogonal test

Factor level	Rockfall radius <i>r/cm</i>	Movement height <i>H/m</i>	Cushion's thickness <i>h/cm</i>	Particle size <i>d/cm</i>
Level 1	2	0.4	2	0.2
Level 2	3	0.8	4	0.6
Level 3	4	1.2	6	1.0
Level 4	5	1.6	8	1.4

230 In order to improve the test accuracy, and all the factors are 4 levels, the testing program of  
 231  $L_{32} (4^9)$  arrangement factor can be selected. The *COR* and damage depth *L* of cushion are taken as





232 test indexes to explore the influence degree of 4 factors (Pichler et al., 2005).  
 233 Considering that the rockfall motion has large randomness, each case is tested three times to  
 234 obtain the mean as the final result, so as to improve the accuracy of experiments, the test results  
 235 are shown in Table 2.  
 236

Table 2 Orthogonal test results

Test number	Rockfall radius $r/cm$	Movement height $H/m$	Cushion's thickness $h/cm$	Particle size $d/cm$	Damage depth of cushion $L/cm$	<i>COR</i> of cushion
1	2	0.4	2	0.2	0.65	0.278
2	2	0.8	4	0.6	0.74	0.273
3	2	1.2	6	1.0	0.93	0.282
4	2	1.6	8	1.4	1.05	0.295
5	3	0.4	2	0.6	0.58	0.294
6	3	0.8	4	0.2	1.45	0.265
7	3	1.2	6	1.4	1.03	0.317
8	3	1.6	8	1.0	1.60	0.280
9	4	0.4	4	1.0	0.62	0.296
10	4	0.8	2	1.4	0.56	0.338
11	4	1.2	8	0.2	2.60	0.256
12	4	1.6	6	0.6	2.20	0.284
13	5	0.4	4	1.4	0.61	0.309
14	5	0.8	2	1.0	0.58	0.328
15	5	1.2	8	0.6	2.12	0.280
16	5	1.6	6	0.2	2.85	0.273
17	2	0.4	8	0.2	1.36	0.216
18	2	0.8	6	0.6	1.24	0.265
19	2	1.2	4	1.0	1.13	0.302
20	2	1.6	2	1.4	0.68	0.358
21	3	0.4	8	0.6	0.92	0.231
22	3	0.8	6	0.2	1.49	0.256
23	3	1.2	4	1.4	1.08	0.327
24	3	1.6	2	1.0	0.84	0.351
25	4	0.4	6	1.0	0.77	0.287
26	4	0.8	8	1.4	0.81	0.281
27	4	1.2	2	0.2	1.03	0.336
28	4	1.6	4	0.6	1.96	0.318
29	5	0.4	6	1.4	0.67	0.292
30	5	0.8	8	1.0	1.05	0.275
31	5	1.2	2	0.6	1.14	0.347
32	5	1.6	4	0.2	2.54	0.294

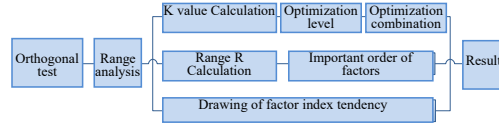
237 **4.2 Optimization analysis and discussion of test results**

238 4.2.1 Optimization analysis method (flow)

239 In this test, analysis method is preferred to optimize the calculation result and the



240 optimization process is shown in Figure 10.



241

242

Fig. 10 Optimization analysis flow chart of test

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The rockfall radius  $r$ , movement height  $H$ , cushion's thickness  $h$  and particle size  $d$  four parameters belong to factor set  $x \in (A, B, C, D)$ , level number of all factors is set as 4, then it can calculate the test statistical parameter under  $y$  level of factor set  $X$  that  $K_{xy}$  ( $x=A, B, C, D; y=1, 2, 3, 4$ ), that is, sum the all test result index  $P_{xy}$  containing the  $Y$  level of factor  $X$ , then divide the total number of level to obtain the average value  $k_{xy}$ , in which,  $P_{XY}$  is the random variable of normal distribution:

249

$$k_{xy} = \frac{K_{xy}}{4} = \sum P_{xy} / 4 \quad (6)$$

250

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Where  $K_{xy}$  is the statistical parameter of the  $x$  factor at the  $y$  level,  $k_{xy}$  is the average value of  $K_{xy}$ , and  $R_y$  is the range of the  $y$  factor.

It can be judged from  $k_{xy}$  that  $x$  factor optimization level and optimization combination, if the larger the index value is, the optimum it is, then select the level increasing the index value, that is, the corresponding level of maximum value of all factors  $k_{xy}$ ; on the contrary, if the smaller the index value is, the optimum it is, select the corresponding level of minimum value of all factors  $k_{xy}$ . The corresponding parameter combination of optimal level of all factors is the optimal parameter combination.  $R_y$  has reflected the amount of variation of test index when the  $y$  factor level is fluctuating. The larger the  $R_y$  is, the more sensitive the factor to the influence of test index. According to  $R_y$ , the importance order of factors can be judged and the optimization level and optimization combination of  $x$  factor can be judged from  $k_{xy}$ .

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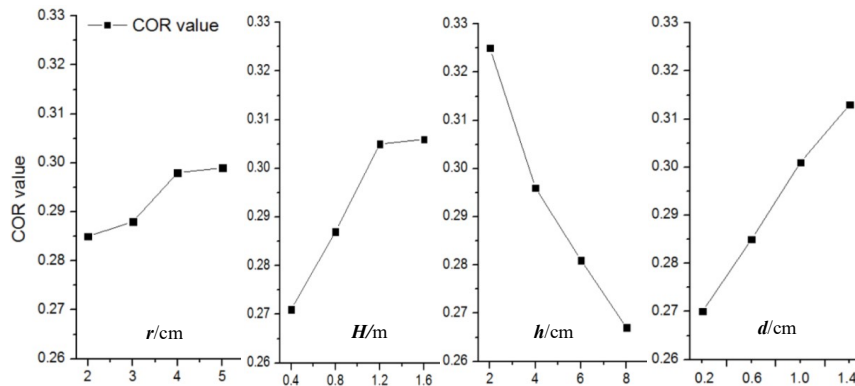
4.2.2 The analysis results and discussion  
 Range analysis is taken to analyze the orthogonal test results shown in Table 2, the influencing factors range analysis of  $COR$  and damage depth  $L$  of cushion are shown in Table 3, then the optimum parameter combination including rockfall radius  $r$ , movement height  $H$ , cushion's thickness  $h$  and particle size  $d$  are obtained to reduce  $COR$  effectively according to it.

Table 3 influencing factors range analysis of all evaluation indexes

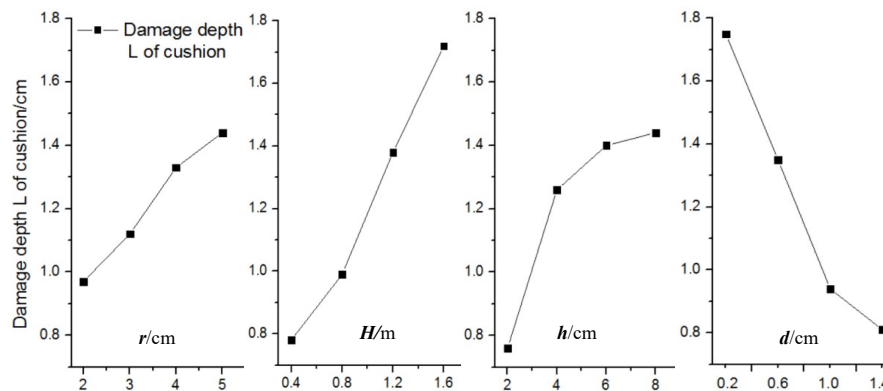
Evaluation index	Levels	Rockfall radius $r/cm$	Movement height $H/m$	Cushion's thickness $h/cm$	Particle size $d/cm$
COR of cushion	$k_1$	0.285	0.271	0.325	0.270
	$k_2$	0.288	0.287	0.296	0.285
	$k_3$	0.298	0.305	0.281	0.301
	$k_4$	0.299	0.306	0.267	0.313
	$R_y$	0.014	0.035	0.058	0.043
Damage depth of cushion $L$	$k_1$	0.97	0.78	0.76	1.75
	$k_2$	1.12	0.99	1.26	1.35
	$k_3$	1.33	1.38	1.40	0.94
	$k_4$	1.44	1.72	1.44	0.81
	$R_y$	0.47	0.94	0.68	0.94



267 The following conclusions can be obtained through Table 3:  
 268 (1) The influence degree of all factors on *COR* of cushion is respectively: cushion's thickness  
 269  $h$  > particle size  $d$  > movement height  $H$  > rockfall radius  $r$ ;  
 270 (2) The influence degree of all factors on damage depth  $L$  of cushion is respectively: particle  
 271 size  $d$  = movement height  $H$  > cushion's thickness  $h$  > rockfall radius  $r$ .  
 272 To further explore the effects of each factor on test indexes, the *E-I* tendency figures are  
 273 drawn (Tao et al., 2017), the level of all factors is X-coordinate (*E*), the average value of test index  
 274 is Y-coordinate (*I*). The *E-I* tendency drawings have intuitively reflected the tendency of test index  
 275 with the change of factor level, which can point direction for further test, as shown in Figure 11  
 276 and Figure 12.



277  
 278 Fig. 11 Tendency of each factor on *COR* of cushion



279  
 280 Fig. 12 Tendency of each factor on damage depth  $L$  of cushion

281 The following conclusions can be obtained through Figure 11 to Figure 12:  
 282 (1) The smallest optimal parameter combination of *COR* of cushion is: A1B1C4D1; that is,  
 283 when  $r=2cm$ ,  $H=0.4m$ ,  $h=8$ ,  $d=1.4$ , the *COR* of cushion is the smallest (Figure 11).  
 284 (2) The most shallow optimal parameter combination of damage depth  $L$  of cushion is:  
 285 A1B1C1D4. That is, when  $r=2cm$ ,  $H=0.4m$ ,  $h=2$ ,  $d=1.4$ , the damage depth  $L$  of cushion is the  
 286 most shallow (Figure 12);  
 287 To sum up, the cushion's thickness  $h$  has the most significant influence on the *COR* of  
 288 cushion, while it has the relatively minor effects on the damage depth  $L$  of cushion; secondly is



289 particle size  $d$ , but the cushion is easy to be destroyed, when the rockfall of high kinetic energy  
290 colliding with the cushion of small particle size; the influence degree of rockfall radius  $r$  on the  
291 two indexes is far less than other factors. When the gravel cushion are used to control the rockfall  
292 of slope, the control effect and durability are taken into account (Pichler et al., 2005), therefore the  
293 cushion's thickness  $h$  should be considered firstly, the optimal thickness is 3 or 4 times of the  
294 universal size of rockfall radius. The smaller particle size is, the smaller  $COR$  is, but the cushion is  
295 more likely to be destroyed, so the reasonable particle size can be determined combined with the  
296 size and position of rockfall, so that the cushion not only achieve the effect of reducing  $COR$ , but  
297 also maintain its stability.

## 298 5 Conclusions

299 Through the laboratory collision tests, the buffer and energy-dissipation mechanism of  
300 various cushion under different impact energy is studied, the following conclusions are obtained:

301 1. Compared with conventional protection measures, the gravel cushion design makes full  
302 use of waste mullock produced in the process of mine extension, which can be broken into  
303 different particle size conveniently, it can not only reduce the cost of preventing rockfall and  
304 mullock transportation obviously, relieving the problem that the dump of mine is overloaded, but  
305 also achieve the better control effect, which realizes the goal of 'stone conquers stone'.

306 2. Under the low impact energy, the change of cushion's thickness has a relatively low effect  
307 on the  $COR$  of cushion, while under the high impact energy, the energy-absorption effect of  
308 different thickness gravel cushion is obviously different. Therefore, in the process of the cushion  
309 design, the estimated quality and falling height of the potential dangerous rock are investigated,  
310 and the impact energy of the rockfall can be roughly estimated.

311 3. The cushion's thickness  $h$  has the most significant influence on the  $COR$  of cushion, the  
312 optimum cushion's thickness and particle size can be obtained by taking the control effect,  
313 economic rationality and structural reliability into account. The smaller particle size is, the smaller  
314  $COR$  is, but the cushion is more likely to be destroyed, the reasonable particle size can be  
315 determined combined with the size and position of rockfall, so that the cushion can not only  
316 achieve the effect of reducing  $COR$ , but also maintain its stability.

317 4. At present, the gravel cushion design on the platform cannot have a relatively reasonable  
318 rule to follow for majority of engineering personnel, which is a large blindness. The change of  
319 cushion's particle size could improve the effects of controlling rockfall, instead of only increasing  
320 the cushion thickness, which provides certain theoretical and practical basis for the wide  
321 application of cushion design to control rockfall in open-pit mine.

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## 325 Reference



- 326 [1] Huang, R. Q., Liu, W. H., Zhou, J. P., and Pei, X. J. Rolling tests on movement characteristics of rock blocks.  
327 Chinese Journal of Geotechnical Engineering, 2007, 29(9):1296-1302. (in Chinese)
- 328 [2] Pantelidis, L. Rock slope stability assessment through rock mass classification systems. International Journal  
329 of Rock Mechanics and Mining Sciences, 2009, 46(2): 315-325.
- 330 [3] Pantelidis, L. An alternative rock mass classification system for rock slopes. Bulletin of engineering geology  
331 and the environment, 2010, 69(1):29-39.
- 332 [4] Heidenreich B. Small- and half-scale experimental studies of rockfall impacts on sandy slopes. Dissertation,  
333 EPFL, 2004.
- 334 [5] V Labiouse, B Heidenreich Heidenreich. Half-scale experimental study of rockfall impacts on sandy slopes.  
335 Natural Hazards & Earth System Sciences, 2009, 9(6):1981-1993.
- 336 [6] Thornton C, Ning Z. A theoretical model for the stick/bounce behavior of adhesive, elastic –lastic spheres.  
337 Poeder Technology, 1998, 99 (2):154-162.
- 338 [7] Topal T, Akin M, Özden AU. Analysis and evaluation of rockfall hazard around Afyon Castle, Turkey.  
339 Environmental Geology, 2006, 53 (1) :191-200.
- 340 [8] Koleini M, Van Rooy JL Falling rock hazard index: a case study from the Marun Dam and power plant,  
341 south-western Iran. Bull Eng Geol Environ, 2011, 70 (2) :279-290.
- 342 [9] Saroglou H, Marinos V, Marinos P, Tsiambaos G. Rockfall hazard and risk assessment: an example from a  
343 high promontory at the historical site of Monemvasia, Greece. Nat Hazards Earth Syst Sci, 2012,  
344 12(6):1823–1836.
- 345 [10] Sadagah B. Back analysis of a rockfall event and remedial measures along part of a Mountainous Road,  
346 Western Saudi Arabia. Int J Innov Sci Mod Eng (IJISME). 2015, 3(2): 2319-6386.
- 347 [11] Huang, R. Q., Liu, W. H., Gong, M. F., and Zhou, J. P. Study of trees resistance effect test on rolling rock  
348 blocks. Chinese Journal of Rock Mechanics and Engineering, 2010, 29(s1): 2895-2901. (in Chinese)
- 349 [12] S. Notaro, A. Paletto. The economic valuation of natural hazards in mountain forests: an approach based on  
350 the replacement cost method. J. For. Econ, 2012, 18 (4): 318–328.
- 351 [13] Kishi N, Bhatti A Q. An equivalent fracture energy concept for nonlinear dynamic response analysis of  
352 prototype RC girders subjected to falling-weight impact loading. International Journal of Impact Engineering,  
353 2010, 37(1):103–113.
- 354 [14] Kishi N, Konno H, Ikeda K, et al. Prototype impact tests on ultimate impact resistance of PC rock-sheds.  
355 International Journal of Impact Engineering, 2002, 27(9):969–986.
- 356 [15] Bhatti A Q, Kishi N. Impact response of RC rock-shed girder with sand cushion under falling load. Nuclear  
357 Engineering and Design, 2010, 240(10):2626-2632.
- 358 [16] Bhatti A Q, Kishi N, Mikami H, et al. Elasto-plastic impact response analysis of shear-failure-type RC  
359 beams with shear rebars. Materials & Design, 2009, 30(3):502–510.
- 360 [17] Badger, T. C., Duffy, J. D., and Schellenberg, K., “Chapter 14-Protection,” Rockfall Characterization and  
361 Control. 2009: 48-62.
- 362 [18] Descoedres, F., Stoffel, S. M., Boll, A., Gerber, W. and Labiouse, V., “Chapter 4-Rockfalls,” Disaster  
363 Resilient Infrastructure, Minor, H. E. (Ed.) International Decade Natural Disaster Reduction (IDNDR), 1999:  
364 37-46.
- 365 [19] F. Delhomme, M. Mommessin, J. P. Mougouin, and P. Perrotin, “Behavior of a structurally dissipating rock-shed:  
366 experimental analysis and study of punching effects,” International Journal of Solids and Structures, 2005,  
367 42(14):4204–4219.
- 368 [20] M. Mommessin, A. Agbossou, F. Delhomme et al., “Horizontal and slanting reinforced concrete slabs for  
369 structurally dissipating rock-shed: experimental analysis,” in Proceedings of the 5th International Conference



- 370 on Fracture Mechanics of Concrete and Concrete Structures, V. C. Li, C. K. Y. Leung, and K. J. Willam, Eds.,  
371 2004, 2(1):965–972, American Concrete Institute, Vail, Colo, USA.
- 372 [21] Heierli W, Merk A, Temperli A. Schutz gegen Steinschlag—Protection contre les chutes de pierres  
373 [Protection against rockfall]. Forschungsarbeit 6/80. Vereinigung Schweizerischer Strassenfachleute (VSS),  
374 1981 [in German].
- 375 [22] Labiouse V, Descoedres F, Montani S. Experimental study of rock sheds impacted by rock blocks. *Struct*  
376 *Eng Int*, 1996, 6(3):171–176.
- 377 [23] S. Kawahara, T. Muro. Effects of dry density and thickness of sandy soil on impact response due to rockfall.  
378 *Journal of Terramechanics*, 2006, 43 (3): 329–340.
- 379 [24] Giani GP (1992) Rock slope stability analysis. Balkema, Rotterdam.
- 380 [25] Chau KT, Wong RHC, Wu JJ. Coefficient of restitution and rotational motions of rockfall impacts. *Int J Rock*  
381 *Mech Min Sci* 2002, 39(1):69–77.
- 382 [26] Yang Youkui, Zhou Yingqing, Jiang Ruiqi, et al. Theory and practice of slope geological disaster flexible  
383 protection [M]. Beijing: Science Press, 2005. (in Chinese)
- 384 [27] Ulusay R, Hudson JA. The complete ISRM suggested methods for rock characterization, testing and  
385 monitoring: 1974–2006. Ankara: ISRM Commission on Testing Methods; 2007.
- 386 [28] Aydin A. ISRM Suggested method for determination of the Schmidt hammer rebound hardness: revised  
387 version. *Int J Rock Mech Min Sci*. 2009; 46(3):627–34.
- 388 [29] Asteriou, P., Saroglou, H., and Tsiambaos, G. “Geotechnical and kinematic parameters affecting the  
389 coefficients of restitution for rockfall analysis.” *International Journal of Rock Mechanics and Mining*  
390 *Sciences*, 2012, 54(1):103-113.
- 391 [30] Bouguet JY. Camera calibration toolbox for Matlab. [http://www.vision.caltech.edu/bouguetj/calib\\_doc](http://www.vision.caltech.edu/bouguetj/calib_doc).  
392 Accessed 20 Jan, 2012.
- 393 [31] Asteriou P, Saroglou H, Tsiambaos G Rockfall: scaling factors for the coefficient of restitution. In:  
394 Kwasniewski M, Lydzba D (eds) Rock mechanics for resources, energy and environment. Taylor & Francis  
395 Group, London, 2013:195–200.
- 396 [32] Kawahara S, Muro T. Effects of weight mass and drop height on vertical distribution of dry density of sandy  
397 soil in one-dimensional impact compaction. In: Proceedings of the 5th Asia-Pacific regional conference of  
398 the ISTVS; 1998:151–61.
- 399 [33] Pei, X. J., Liu, Y., Wang, D., P. Study on the Energy Dissipation of Sandy Soil Cushions on the Rock-shed  
400 Under Rockfall Impact Load. *Journal of Sichuan University (Engineering Science Edition)*, 2016,  
401 48(1):15-22. (in Chinese)
- 402 [34] Pichler B, Hellmich C, Mang H A. Impact of rocks onto gravel design and evaluation of experiments.  
403 *International Journal of Impact Engineering*, 2005, 31(5):559–578.
- 404 [35] Tao, Z. G., Zhu, C. Test of V shaped groove structure against rockfall based on orthogonal design. *Journal of*  
405 *China Coal Society*, 2017, 42(9): 2307-2315.