

The effects of gravel cushion particle size and thickness on the coefficient of restitution in rockfall impacts

Zhu Chun^{1,2,3}, Wang Dongsheng^{2,3}, Xia Xing^{2,3}, Tao ZhiGang^{2,3}, He ManChao^{1,2,3}, Cao Chen^{*1}

Corresponding Email: ccao@jlu.edu.cn (First Corresponding author)

zhuchuncumb@163.com;

(1. College of Construction Engineering, Jilin University, Changchun 130026, China)

(2. State Key Laboratory for Geomechanics & Deep Underground Engineering, Beijing 100083, China)

(3. School of Mechanics and Civil Engineering, China University of Mining & Technology, Beijing 100083, China)

Abstracts: Gravel cushions are widely used to absorb the impact energy of falling rocks in open-pit mines. A particularly important application is to enhance the energy-absorbing capacity of rockfall sheds. In this paper, we study how varying the thickness and particle size of a gravel cushion influences its energy-consumption and buffering effects. We performed a series of laboratory drop tests by dropping blocks from a fixed height onto cushions of different thicknesses and particle sizes. The results indicate that, for a given impact energy, the cushion thickness has a strong influence on the measured coefficient of restitution (*COR*) and therefore impact pressure. Additional tests were performed to study how the radius of the block and the height it is dropped from affect the measured *COR*. This showed that as the movement height of the block is increased the *COR* also increases, and blocks with larger radii exhibit a larger variability in measured *COR*. Finally, we investigated the influence of rockfall block radius, r , movement height, H , cushion thickness, h , and particle size, d , on the *COR* and the damage depth, L , of the cushion. The test results reveal that the cushion thickness is the primary design parameter, controlling not only *COR* but also the stability of the cushion material. The results provide a theoretical and practical basis for the design of gravel cushions for rockfall protection.

Keywords: Rockfall; cushion thickness; laboratory test; particle size; coefficient of restitution (*COR*).

1 Introduction

Rockfall constitutes a serious hazard in the working areas and facilities of the world's open-pit mines. Where slope surfaces are seriously weathered and the disturbing forces from mining are strong, landslides and rock-body collapse are prone to occur during rainfall. In rockfall, rocks roll down slope due to instability caused by gravity or exogenic action and come to rest at an obstacle or in the gentler part of the slope (Huang et al., 2007). Rockfall is widely distributed and occurs suddenly, posing a serious threat to life and property (Pantelidis, 2010). In response to frequent rockfall disasters in recent years, numerous scholars in China and abroad have conducted in-depth studies into the characteristics of rockfall movement through theoretical analysis, field investigation, and numerical simulation. For example, Mignelli et al. (2014), applied a rockfall risk management approach to the road infrastructure network of the Regione Autonoma Valle D'Aosta in order to calculate the level of risk and the potential for its reduction by rockfall protection devices. A comparative analysis of road accidents in the Aosta Valley was then undertaken to verify the methodology. Asteriou et al. (2016) examined the effects of rock shape by performing tests with spherical and cubic blocks, finding that spherical blocks show higher and more consistent coefficient of restitution (*COR*) values than cubic blocks. Howald et al. (2017) evaluated the protective capacity of existing and newly proposed protection measures and considered the possible reclassification of hazard as a function of the mitigation role played by the

41 measure. Furthermore, numerical simulation software has been adopted to analyze the
42 characteristics of rockfall movement. The software ROCFALL 3.0 has been adopted in dam
43 construction, road construction and the protection of historical places to calculate the velocity and
44 locus of rockfall and avoid damage to the project (Topal et al., 2006; Koleini and Van Rooy, 2011;
45 Saroglou et al., 2012; Sadagah, 2015). State-of-the-art simulation techniques incorporating
46 nonsmooth contact dynamics and multibody dynamics have been applied to and adapted for the
47 efficient simulation of rockfall trajectories, and the influence of rock geometry on rockfall
48 dynamics has been studied through numerical simulation (Leine et al., 2014).

49 The research outlined above indicates that several types of protection measure can be
50 effective in controlling rockfall. Trees have a significant blocking effect on rolling rocks.
51 Interception influence tests of the effect of trees on rockfall have been designed based on analysis
52 of the velocity change, the distance traveled by the rockfall, and the probability of collision
53 between trees and rockfall (Notaro, 2012; Monnet et al., 2017). Semi-rigid rockfall protection
54 barriers have been installed along areas threatened by rockfall events, and Miranda et al. (2015)
55 have carried out a numerical investigation of such protection barriers to obtain essential structural
56 information such as their energy-absorption capacity. Furthermore, Lambert et al. (2014)
57 conducted real-scale impact experiments with impact energies ranging from 200 kJ to 2200 kJ.
58 They studied the response of rockfall protection embankments composed of a 4-m high cellular
59 wall to a rock impact and compared this with previous real-scale experiments on other types of
60 embankment. Finally, Sun et al. (2016) used a tire cushion layer to absorb rockfall impact,
61 utilizing the radial deformation of the tire. They built a reinforced concrete structure model with a
62 tire cushion layer and carried out artificial rockfall tests.

63 The protection research outlined above is mainly applicable to conventional human
64 settlements, and it is expensive and inconvenient to use these measures to control rockfall in an
65 open-pit mine. A relatively common way of preventing and controlling rockfall hazard in an
66 open-pit mine is to lay an energy-consuming layer on a safety platform (Labiouse et al., 1996).
67 However, research into such cushions seldom considers the effects of the particle size of the
68 cushion on the characteristics of rockfall movement. In particular, the combined effects of the
69 particle size and thickness of a gravel cushion on the coefficient of restitution (*COR*) have not yet
70 been explored. A large amount of mullock is produced during mining, and this can be broken into
71 particles of different sizes in a crusher and used to pave the platform as an energy-consuming layer.
72 A certain thickness of gravel cushion on the platform can act as a buffer, effectively absorbing the
73 impact energy of rockfall and reducing the impact load on the protective structure while also
74 reducing the kinetic energy of the rockfall and causing it to stall. Because the impact between the
75 rockfall and gravel cushion is of short duration, it involves complicated elastic-plastic deformation
76 and energy conversion, and the energy absorption performance of gravel cushions of different
77 thicknesses and particle sizes are quite different under rockfall impacts. Determining the
78 energy-consumption buffering mechanism of a gravel cushion and calculating the subsequent
79 rockfall movement has become the key to cushion design. Therefore, to control rockfalls
80 effectively, it is necessary to further study the effects of the particle size and thickness of the
81 cushion on *COR* under rockfall impact.

82 **2 Coefficient of restitution**

83 It is challenging to predict the trajectory of rebound for a rockfall because it is influenced by
 84 several parameters such as the strength, roughness, stiffness, and inclination of the slope and
 85 blocks (Labiose and Heidenreich, 2009). However, the coefficient of restitution (*COR*) is widely
 86 used for this purpose (Giani, 1992).

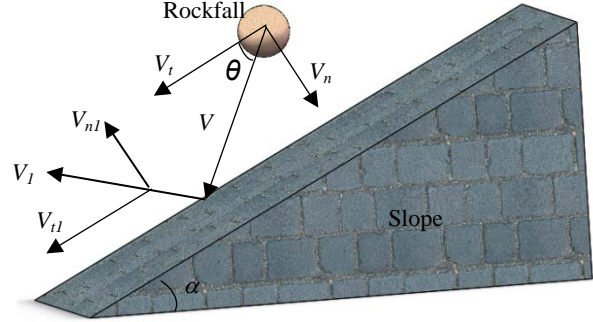


Fig.1 Motion model of rockfall

87
 88 The definitions of *COR* are various (Chau et al., 2002) but for a block impacting a rocky
 89 slope (Figure 1), it can be defined on the basis of the theory of inelastic collision as:

$$91 \quad V_{COR} = \left| \frac{V_1}{V} \right|, \quad (1)$$

92 where V and V_1 are the magnitudes of the incident and rebound velocities at the locus, respectively
 93 (m/s).

94 V_{COR} has normal and tangential components. The normal (R_n) and tangential (R_t) coefficients
 95 are defined as:

$$96 \quad R_n = \left| \frac{V_{n1}}{V_n} \right| \quad \text{and} \quad R_t = \left| \frac{V_{t1}}{V_t} \right|, \quad (2)$$

97 where R_n and R_t are the normal and tangential restitution coefficients, respectively, and V_n and V_{n1}
 98 are the normal components and V_t and V_{t1} are the tangential components of the velocity of the
 99 block before and after the impact, respectively (m/s).

100 The total energy, E , of the block consists of the translational (E_0) and rotational (E_w) energy:

$$101 \quad E = E_0 + E_w = \frac{1}{2}mv^2 + \frac{1}{2}I\omega^2, \quad (3)$$

102 and the total energy coefficient (ET_{COR}) is proposed to be:

$$103 \quad ET_{COR} = \frac{\frac{1}{2}mV_1^2 + \frac{1}{2}I\omega_1^2}{\frac{1}{2}mV^2 + \frac{1}{2}I\omega^2} = \frac{0.6mV_1^2}{0.6mV^2} = \frac{V_1^2}{V^2} = V_{COR}^2, \quad (4)$$

104 where m is the mass of the block, I is its moment of inertia, and ω and ω_1 are the angular velocity
 105 before and after the impact, respectively.

106 When a dangerous rock-body breaks away from the parent body, it will inevitably generate
 107 collisions with the slope during the rolling process and lose energy. A formula for the approximate
 108 calculation of the total kinetic energy of the rockfall has been derived from engineering surveys
 109 (Yang et al., 2005; Zhu et al. 2018):

$$110 \quad E = E_0 + E_w = 1.2E_0 = 0.6mV^2 = 0.6m(V_n^2 + V_t^2), \quad (5)$$

111 3 Experimental studies

112 3.1 Experimental material and apparatus

113 In order to study the effects of the particle size and thickness of the cushion on *COR* under
114 rockfall impact conveniently, a high-strength gypsum material was adopted to simulate the
115 rockfall. A previous study (Chau et al., 2002) recommends a moisture content of 30–50% for the
116 sample, so in this study, all samples were given a moisture content of 40%.

117 A large number of tests have shown that spherical falling blocks have higher and more
118 consistent *COR* values than cubic blocks (Asteriou et al., 2016), and so that the same control
119 methods will have greater difficulty in containing their effects than those of non-spherical blocks
120 with the same properties. This indicates that spherical rocks are a common hazard and that if a
121 cushion is designed to resist these, it can also effectively resist non-spherical rocks. This greater
122 threat should therefore be the primary concern when designing a protective cushion. For this
123 reason, spherical blocks with radii of 2 cm, 3 cm, 4 cm and 5 cm (Figure 2a) were used to simulate
124 rockfall in this study. Additionally, six standard 5-cm diameter, 10-cm high cylindrical samples
125 were created with which to test the uniaxial compressive strength of the gypsum materials. The
126 uniaxial compression test is shown in Figure 2b. Due to the inherent error associated with the test,
127 the ultimate compressive strength of the six samples is different, so the average value is taken as
128 the compressive strength of the material. The average value at which the specimens are destroyed
129 is 6.48 Mpa, indicating that a gypsum sample with 40% moisture content is strong enough not to
130 be shattered during the collision process (Ulusay et al., 2007; Aydin, 2009).



131
132 (a) Spherical gypsum samples of different sizes (b) Standard specimen under a uniaxial compression test (c) Sieved granules of different particle sizes
133

134 Fig.2 Experimental material production and testing process

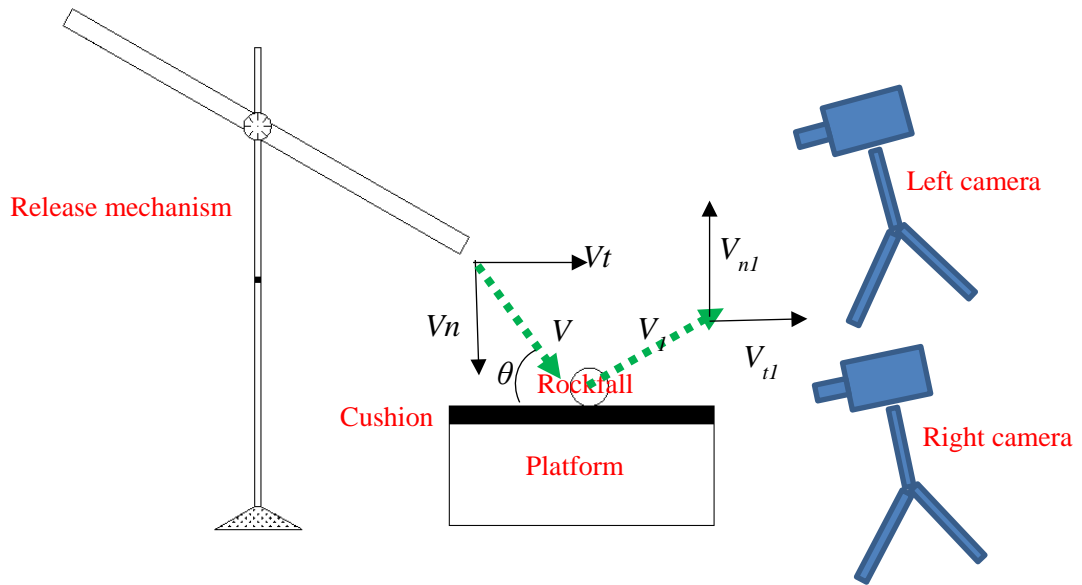
135 In order to explore the effect of different cushion thicknesses and particle sizes on the rolling
136 motion of a rockfall, massive gypsum boards with the same properties as the blocks were broken,
137 and gypsum particles for simulating the gravel cushion were divided by coarseness using 2 mm, 6
138 mm, 10 mm, 14 mm, 18 mm and 24 mm sieves (Figure 2c).

139 A simple rolling stone releasing device is shown in Figure 3. A tube with adjustable
140 inclination and height is used to vary the translational impact velocity of the blocks (Asteriou et al.,
141 2012). The blocks slide and roll through the tube to collide with the plate. Two synchronized
142 digital cameras (1024 × 1024 pixels and a 200 fps capture rate) were used to acquire the velocities
143 of the blocks in stereoscopic space (Bouguet, 2008; Asteriou et al., 2013).

144 The two cameras, which obtained the motion, velocity, and kinetic energy automatically,
145 were placed symmetrically at a distance of approximately 0.9 m from the impact surface (Figure
146 3). The distance between the two cameras was approximately 1.2 m, making the cameras look
147 slightly down at the targeted platform.

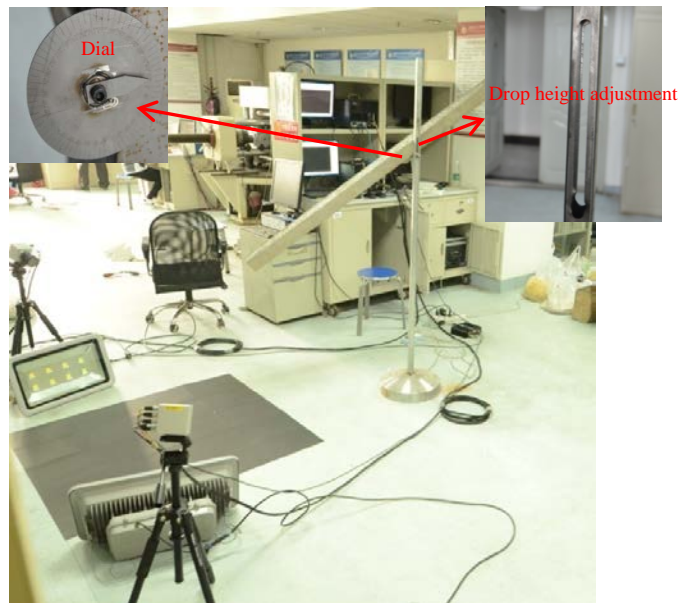
148 The synchronized recordings from the two cameras captured a sequence of image stereopairs
149 at time intervals of 1/200 s. By applying stereo-photogrammetric processing, the position of any
150 point in both images can be computed in 3D space. The image plane has a 2D coordinate system
151 where position measurements can be made using pixel coordinates. The camera has a 3D reference
152 coordinate system that is based on the image plane, pointing in the viewing direction of the camera.

153 The speed of the rocks can be obtained by measuring the distance they have moved between
 154 adjacent frames.



155
 156

(a)



(b)

Fig.3 The experimental apparatus. (a) Model, (b) Laboratory

157
 158
 159
 160
 161
 162
 163
 164
 165
 166
 167
 168

To simulate gravel cushions of different thicknesses, a large number of 40 cm length \times 40 cm width \times 2 cm height hollow gypsum boards were constructed. A 30 cm length \times 30 cm width \times 2 cm height section was cut out of the center of each board. The hollow gypsum boards were stacked on top of each other to simulate gravel cushions of different thickness, and then the hollow parts of the boards were filled with gypsum particles. The hollow boards were fixed to a massive 40 cm length \times 40 cm width \times 6 cm height gypsum base to ensure the preservation of momentum from the impact. In order to accurately measure the speed of the blocks with the cameras and to avoid interference from the motion of cushion particles affected by the collision, the cushion was blackened (Figure 4).

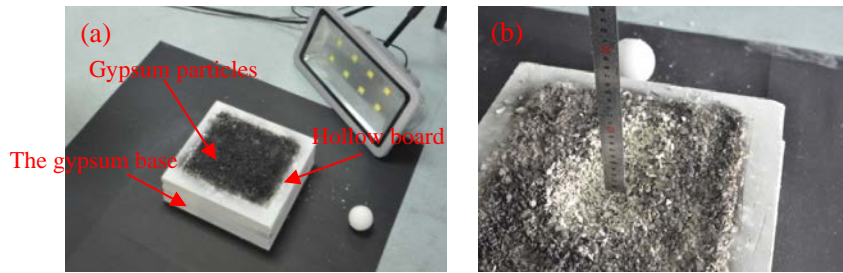


Fig. 4 Photographs of a cushion (a) before and (b) after a rock impact experiment

3.2 Experimental procedure

The main uncertainties in the test results arise in tests with large cushion particles, where the wider scatter of the values is attributed to the contact configuration between the large cushion particles and the blocks: large cushion particles have numerous different configurations. This also affected the deviation in the trajectory caused by the impact, which had a drastically higher uncertainty than for small cushion particles. In order to counteract the effects of chance, a “three tests for the mean” method was adopted, and the average value was set as the final result given for each data point in the figures and tables presented here. For cushion particle sizes of 18 mm and 24 mm, each test was repeated five times and the middle three values were used to obtain the average value, while for cushion particle sizes of less than 18 mm, each test was conducted three times. The obviously outlying results were the two rare conditions that $V_{COR}=0$ or $V_{COR} > 1$, if these results were obtained, the tests were repeated to reduce the error.

The 2 cm, 3 cm, 4 cm, and 5 cm radius spherical blocks (Figure 2) were released from a height of 1.2 m, and the effects of cushion thickness and particle size and of block volume on the COR were studied. V_{COR} for the $CORs$ measured in the experiment was calculated using the magnitudes of the incident and rebound velocities as in Equation (1). The block was inserted into one side of the tube and, after sliding and rolling through the tube, collided with the collision surface. The initial impact surface was the massive gypsum base to simulate the platform before paving with a cushion in an open-pit mine. Paved tests were then performed using thicknesses of 2 cm, 4 cm, 6 cm, 8 cm, 10 cm, 12 cm, and 14 cm and cushion particle sizes of 2 mm, 6 mm, 10 mm, 14 mm, 18 mm, and 24 mm. Five iterations of 628 testing cases were carried out.

In order to investigate the effect of rockfall released from different movement heights on the COR of the collision between rockfall and cushion, experiments were conducted in which blocks of 2 cm, 3 cm, 4 cm, and 5 cm radius fell from 0.4 m, 0.8 m, 1.2 m, and 1.6 m to collide with an 8-cm thick cushion of different particle sizes. Four iterations of 352 testing cases were carried out. Photographs of the cushion before and after a rock impact experiment are shown in Figure 4. The cushion was always repaired completely after each impact experiment to ensure that the next experiment was free from interference. If any particles had been knocked off the platform, new particles were added to supplement the cushion, and the surface was blackened again before the next impact experiment in order for the cameras to obtain accurate measurements of block speed.

3.3 Experimental results and discussion

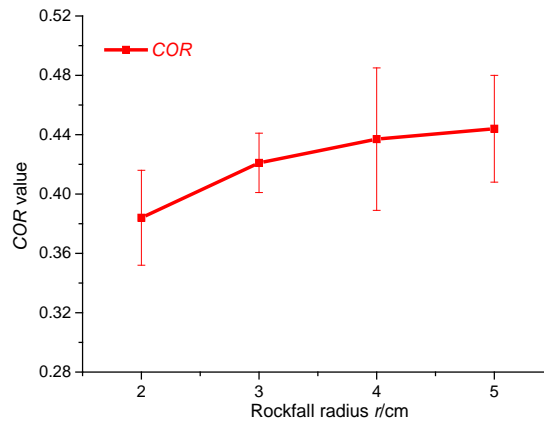
3.3.1 Experimental results

The COR for blocks released from a height of 1.2 m to collide with an uncushioned plate is shown in Table 1 and Figure 5.

Table 1 The COR of block collisions with the plate

$H=1.2\text{m}, h=0\text{cm},$	$r=2\text{cm}(\text{Mean}/\text{Std dev})$	$r=3\text{cm}(\text{Mean}/\text{Std dev})$	$r=4\text{cm}(\text{Mean}/\text{Std dev})$	$r=5\text{cm}(\text{Mean}/\text{Std dev})$
$d=0\text{mm}$	0.384/0.032	0.421/0.020	0.437/0.048	0.444/0.036

206



207

208 Fig. 5 The COR (Mean \pm SD) of block collisions with the plate. (Error bars: one standard deviation)

209 CORs derived from experiments where rocks of different radii were released from a 1.2 m
 210 movement height to collide with a plate paved with cushions of different thicknesses and particle
 211 sizes are plotted in Table 2 and Figure 6. In Figure 6, mean values with error bars are shown for
 212 each test.

213

Table 2 Experimental results for the first group of tests (movement height $H=1.2\text{ m}$)

	$h(\text{cm}) \backslash d(\text{mm})$	$d(\text{mm})$					
		2mm(Mean/Std dev)	6mm(Mean/Std dev)	10mm(Mean/Std dev)	14mm(Mean/Std dev)	18mm(Mean/Std dev)	24mm(Mean/Std dev)
$r=2\text{cm}$	2cm	0.326/0.015	0.332/0.029	0.346/0.029	0.343/0.029	0.348/0.063	0.354/0.059
	4cm	0.294/0.019	0.325/0.029	0.302/0.037	0.323/0.038	0.317/0.062	0.312/0.047
	6cm	0.259/0.017	0.274/0.034	0.282/0.036	0.283/0.042	0.301/0.043	0.296/0.038
	8cm	0.243/0.028	0.254/0.040	0.263/0.048	0.271/0.043	0.277/0.048	0.284/0.074
	10cm	0.241/0.038	0.247/0.048	0.255/0.031	0.258/0.051	0.264/0.068	0.277/0.057
	12cm	0.228/0.027	0.233/0.042	0.247/0.048	0.252/0.057	0.251/0.062	0.266/0.054
	14cm	0.22/0.032	0.232/0.045	0.24/0.032	0.236/0.060	0.249/0.048	0.258/0.054
$r=3\text{cm}$	2cm	0.334/0.019	0.341/0.013	0.347/0.036	0.354/0.050	0.352/0.030	0.368/0.046
	4cm	0.302/0.036	0.315/0.042	0.316/0.044	0.327/0.049	0.326/0.036	0.334/0.065
	6cm	0.277/0.025	0.284/0.024	0.288/0.033	0.318/0.039	0.309/0.053	0.325/0.072
	8cm	0.247/0.026	0.262/0.046	0.267/0.040	0.273/0.055	0.281/0.054	0.292/0.031
	10cm	0.237/0.027	0.246/0.027	0.254/0.031	0.262/0.045	0.257/0.049	0.268/0.051
	12cm	0.226/0.035	0.239/0.045	0.242/0.019	0.248/0.041	0.255/0.035	0.259/0.042
	14cm	0.218/0.053	0.224/0.027	0.229/0.044	0.231/0.054	0.246/0.055	0.262/0.044
$r=4\text{cm}$	2cm	0.336/0.019	0.348/0.022	0.356/0.026	0.365/0.048	0.367/0.036	0.372/0.040
	4cm	0.309/0.026	0.321/0.024	0.315/0.030	0.325/0.023	0.334/0.037	0.343/0.045
	6cm	0.28/0.014	0.309/0.018	0.292/0.023	0.292/0.012	0.312/0.035	0.325/0.033
	8cm	0.256/0.011	0.271/0.023	0.276/0.029	0.274/0.024	0.293/0.031	0.302/0.037
	10cm	0.252/0.015	0.258/0.022	0.269/0.025	0.265/0.024	0.281/0.041	0.278/0.043
	12cm	0.236/0.010	0.245/0.025	0.237/0.027	0.243/0.038	0.252/0.045	0.258/0.035
	14cm	0.224/0.011	0.235/0.022	0.232/0.038	0.237/0.027	0.248/0.038	0.253/0.037
$r=5\text{cm}$	2cm	0.34/0.014	0.342/0.022	0.356/0.035	0.368/0.028	0.371/0.032	0.38/0.036
	4cm	0.324/0.013	0.311/0.017	0.323/0.030	0.344/0.028	0.343/0.037	0.352/0.023
	6cm	0.291/0.009	0.292/0.021	0.318/0.015	0.309/0.025	0.326/0.047	0.33/0.046
	8cm	0.265/0.013	0.28/0.012	0.288/0.025	0.293/0.027	0.302/0.050	0.313/0.043
	10cm	0.263/0.017	0.265/0.029	0.269/0.028	0.272/0.024	0.271/0.040	0.288/0.043
	12cm	0.24/0.012	0.243/0.027	0.252/0.036	0.257/0.028	0.259/0.046	0.266/0.060
	14cm	0.22/0.015	0.23/0.027	0.237/0.012	0.242/0.028	0.234/0.045	0.254/0.034

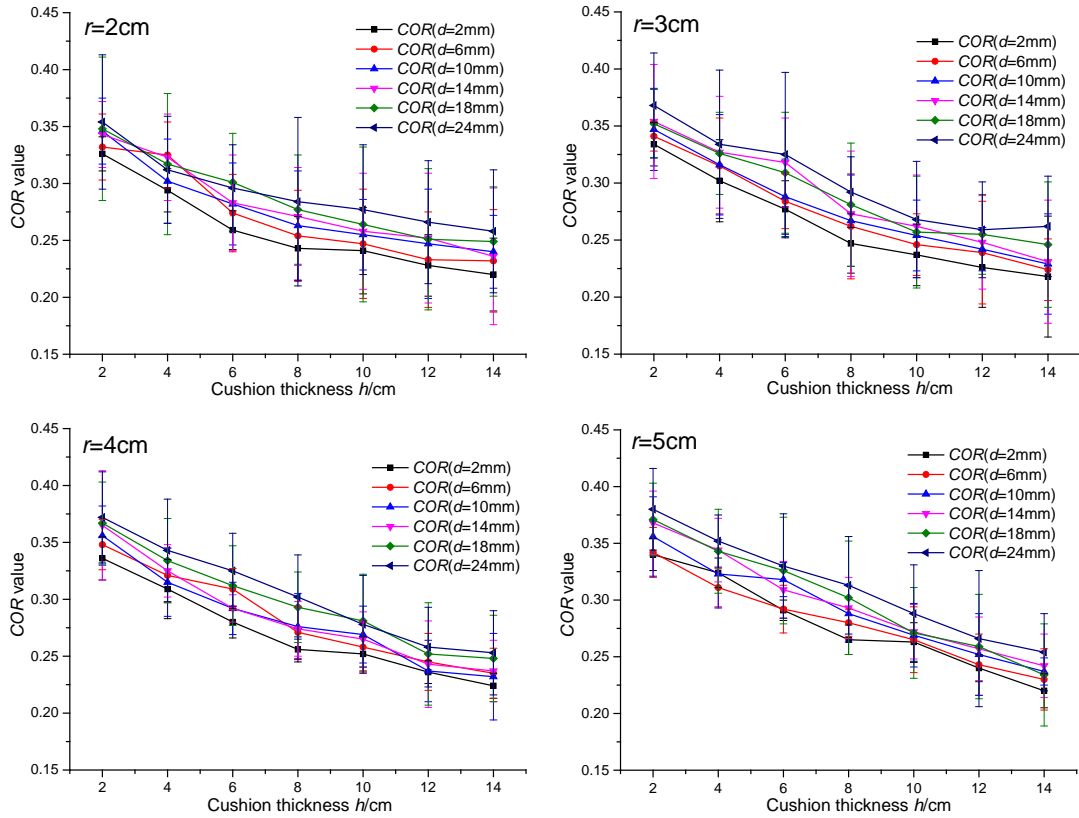


Fig.6 Comparison of the *COR* of blocks of different radii released from a height of 1.2m

CORs derived for rocks of different radii released from different movement heights to collide with an 8-cm thick cushion of various particle sizes are plotted in Table 3 and Figure 7. As with Figure 6, Figure 7 shows mean values with error bars for each test.

Table 3 Experimental results for the second group of tests (cushion thickness $h=8$ cm)

	$H(m)$	$d(mm)$	2mm(Mean/Std dev)		6mm(Mean/Std dev)		10mm(Mean/Std dev)		14mm(Mean/Std dev)		18mm(Mean/Std dev)		24mm(Mean/Std dev)	
			Mean	Std dev	Mean	Std dev	Mean	Std dev	Mean	Std dev	Mean	Std dev	Mean	Std dev
$r=2cm$	0.4m		0.216	0.020	0.228	0.011	0.236	0.025	0.254	0.030	0.256	0.053	0.260	0.037
	0.8m		0.229	0.009	0.234	0.030	0.245	0.027	0.243	0.029	0.262	0.037	0.267	0.053
	1.2m		0.243	0.019	0.254	0.033	0.263	0.033	0.271	0.044	0.277	0.047	0.284	0.032
	1.6m		0.243	0.013	0.252	0.018	0.271	0.042	0.290	0.047	0.283	0.036	0.282	0.051
$r=3cm$	0.4m		0.224	0.015	0.231	0.022	0.243	0.023	0.252	0.037	0.265	0.042	0.268	0.055
	0.8m		0.236	0.015	0.243	0.023	0.264	0.037	0.262	0.037	0.267	0.033	0.276	0.045
	1.2m		0.247	0.020	0.262	0.020	0.267	0.032	0.273	0.046	0.281	0.041	0.292	0.044
	1.6m		0.254	0.014	0.265	0.032	0.286	0.026	0.289	0.035	0.293	0.018	0.301	0.032
$r=4cm$	0.4m		0.231	0.013	0.242	0.015	0.239	0.026	0.264	0.031	0.262	0.029	0.276	0.039
	0.8m		0.245	0.021	0.257	0.012	0.262	0.029	0.287	0.028	0.286	0.039	0.290	0.055
	1.2m		0.256	0.012	0.271	0.036	0.276	0.025	0.284	0.020	0.293	0.038	0.302	0.020
	1.6m		0.261	0.020	0.285	0.018	0.286	0.034	0.299	0.054	0.311	0.041	0.310	0.050
$r=5cm$	0.4m		0.236	0.010	0.253	0.014	0.25	0.036	0.263	0.033	0.276	0.045	0.284	0.036
	0.8m		0.252	0.017	0.267	0.015	0.283	0.022	0.272	0.037	0.294	0.043	0.298	0.045
	1.2m		0.265	0.011	0.28	0.037	0.288	0.030	0.293	0.049	0.302	0.038	0.313	0.045
	1.6m		0.273	0.027	0.287	0.021	0.299	0.042	0.31	0.039	0.308	0.051	0.322	0.038

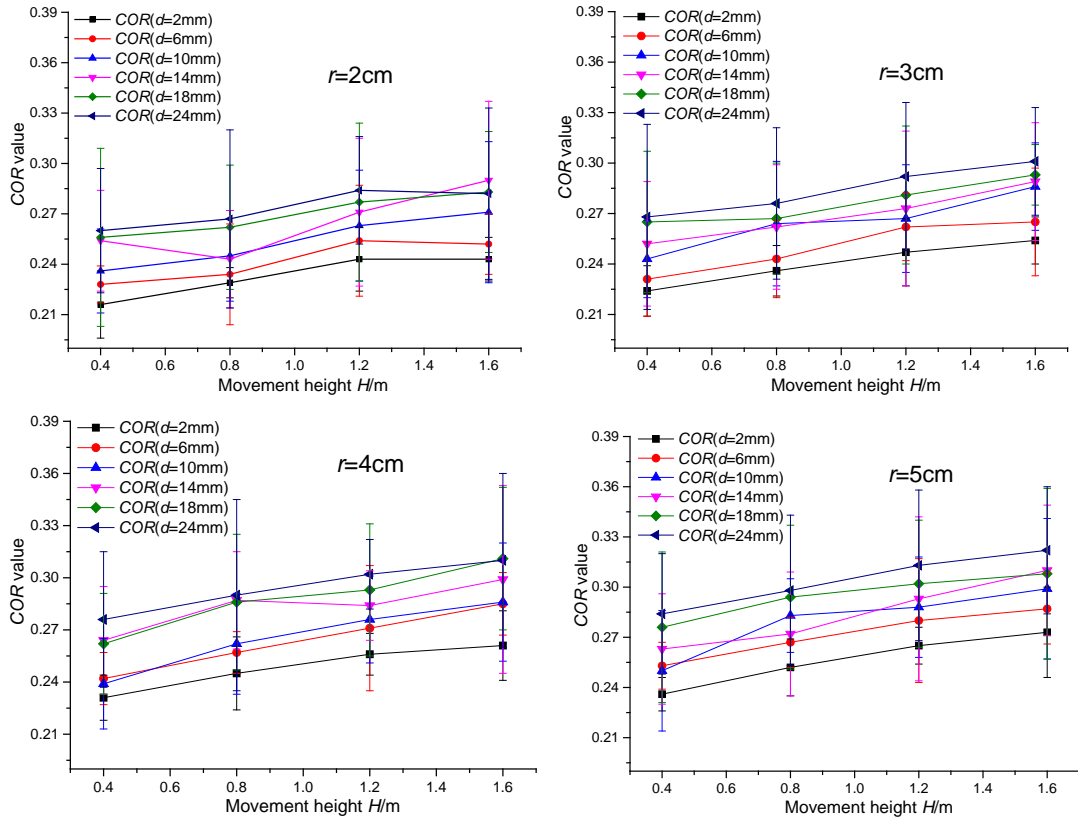


Fig.7 Comparison of the *COR* for blocks of different radii colliding with an 8-cm thick cushion

3.3.2 Discussion

The figures above indicate that cushion thickness and particle size have a strong influence on the *COR* of collisions between a rockfall and a cushion, whereas the influence of rockfall block radius is relatively weak. When the particle size of the cushion is small and its thickness is large, the *COR* of the collision is small, and its effectiveness for energy-consumption is obvious. With an increase in rockfall block radius and movement height, the impact energy increases dramatically for rockfalls colliding with a cushion (Kawahara et al., 1998). Under low impact energy, changes in cushion thickness have a relatively small effect on the *COR* of the collision, and even thin cushions have a certain energy-absorbing effect, as verified by Pei (2016) and Kawahara (2006). However, under high impact energy, the difference in energy-absorption of different thicknesses of gravel cushion is marked. Because a thin cushion can be more easily compressed in a very short time, the rockfall is more likely to be affected by the underlying platform at low cushion thicknesses. This makes reducing the cushion thickness equivalent to increasing the effective stiffness of the cushion, significantly limiting its buffering and energy-absorbing effect. When the cushion thickness is relatively small, the *COR* increases significantly with a decrease in cushion thickness. However, when the cushion's thickness is relatively large, this trend is no longer obvious.

When a constant rockfall release height of 1.2 m is used, the *COR* is large where there is no cushion and decreases significantly with an increase in cushion thickness. This agrees with the observations of Kawahara (2005). However, when the cushion reaches a certain thickness, namely, the ratio of the falling block radius, r , to the cushion thickness, h , is $1/4$ – $1/3$, the rate of reduction in the *COR* with an increase in cushion thickness gradually decreases. *COR* is more sensitive to the thickness of cushions with a small particle size than those with a relatively large particle size: the range in *COR*s caused by thickness variation is wider for small cushion particle sizes, while, as

248 the thickness of cushions with a large particle size is increased, the *COR* of the collision between
249 the rockfall and cushion changes relatively slightly.

250 If the cushion thickness is kept constant at 8 cm, as the movement height of the block
251 increases the *COR* also increases, but when blocks of different radii collide with a cushion of the
252 same thickness, the range in the *COR* of blocks with a large radius is larger than for blocks with a
253 relatively small radius. When the blocks move from a relatively low height, the *COR* of the
254 collision is more likely to be affected by the particle size compared to when blocks are released
255 from a greater height. When the cushion particle size is large, the difference in collision
256 configuration between the rockfall and cushion is more pronounced, resulting in a wide range in
257 the *COR* of the collision.

258 4 Orthogonal test design

259 4.1 Orthogonal test procedure

260 To explore the degree of influence of cushion particle size and thickness on *COR* when a
261 rockfall moves through the cushion, orthogonal test theory was adopted to design a test program
262 (Tao et al., 2017). Orthogonal testing is a design method that allows the testing of multiple factors
263 at multiple levels. It is based on orthogonality and selects representative points from a
264 comprehensive experiment for testing so that fewer trials can fully reflect the impact of the
265 variation of each factor on the index. When these factors cannot be considered in full, the leading
266 factor is considered to achieve the expected effects to a great extent.

267 Four independent parameters, the rockfall block radius, r , movement height, H , cushion
268 thickness, h , and particle size, d , were selected as the basic factors to test. The purpose of doing an
269 orthogonal test was to explore the degree of influence of the four different factors on the *COR* and
270 damage depth, L , and find the combination that will give the optimal protective effect when a
271 rockfall collides with a cushion. The damage depth (L) is the depth to which the cushion is
272 influenced after a rockfall has collided with it and can be used to represent the degree of damage
273 to the cushion. As shown in Table 4, every factor has four levels:

274

Table 4 Factors and levels for the orthogonal test

Factor level	Rockfall radius r/cm	Movement height H/m	Cushion thickness h/cm	Particle size d/mm
Level 1	2	0.4	2	2
Level 2	3	0.8	4	6
Level 3	4	1.2	6	10
Level 4	5	1.6	8	14

275 In order to improve the accuracy of the test, and considering that all of the factors have four
276 levels, the $L_{32}(4^9)$ arrangement factor was selected for the testing program. The damage depth, L ,
277 of the cushion and the *COR* of the rockfall-cushion collision are taken as test indices to explore the
278 degree of influence of the four factors (Pichler et al., 2005).

279 As there is a high degree of randomness inherent in the rockfall motion, each case was tested
280 three times and the mean value was taken as the final result, so as to improve the accuracy of the
281 experiments. The test results are shown in Table 5.

Table 5 Orthogonal test results

Test number	Rockfall radius r/cm	Movement height H/m	Cushion thickness h/cm	Particle size d/mm	Damage depth of cushion L/cm (Mean/Std dev)	COR of collision between rockfall and cushion (Mean/Std dev)
1	2	0.4	2	2	0.65/0.082	0.278/0.012
2	2	0.8	4	6	0.74/0.056	0.273/0.023
3	2	1.2	6	10	0.93/0.082	0.282/0.029
4	2	1.6	8	14	1.05/0.046	0.295/0.028
5	3	0.4	2	6	0.58/0.053	0.294/0.012
6	3	0.8	4	2	1.45/0.165	0.265/0.015
7	3	1.2	6	14	1.03/0.171	0.317/0.041
8	3	1.6	8	10	1.60/0.193	0.280/0.020
9	4	0.4	4	10	0.62/0.036	0.296/0.028
10	4	0.8	2	14	0.56/0.104	0.338/0.029
11	4	1.2	8	2	2.60/0.303	0.256/0.022
12	4	1.6	6	6	2.20/0.375	0.284/0.036
13	5	0.4	4	14	0.61/0.076	0.309/0.031
14	5	0.8	2	10	0.58/0.026	0.328/0.037
15	5	1.2	8	6	2.12/0.217	0.280/0.025
16	5	1.6	6	2	2.85/0.321	0.273/0.022
17	2	0.4	8	2	1.36/0.026	0.216/0.016
18	2	0.8	6	6	1.24/0.106	0.265/0.025
19	2	1.2	4	10	1.13/0.149	0.302/0.031
20	2	1.6	2	14	0.68/0.082	0.358/0.038
21	3	0.4	8	6	0.92/0.121	0.231/0.017
22	3	0.8	6	2	1.49/0.187	0.256/0.012
23	3	1.2	4	14	1.08/0.046	0.327/0.031
24	3	1.6	2	10	0.84/0.076	0.351/0.029
25	4	0.4	6	10	0.77/0.135	0.287/0.035
26	4	0.8	8	14	0.81/0.137	0.281/0.027
27	4	1.2	2	2	1.03/0.159	0.336/0.021
28	4	1.6	4	6	1.96/0.115	0.318/0.030
29	5	0.4	6	14	0.67/0.044	0.292/0.019
30	5	0.8	8	10	1.05/0.092	0.275/0.078
31	5	1.2	2	6	1.14/0.098	0.347/0.025
32	5	1.6	4	2	2.54/0.184	0.294/0.027

283 4.2 Optimization analysis and discussion of test results

284 4.2.1 Optimization analysis method (flow)

285 The method of analysis used to optimize the calculation results and the optimization process
286 is shown in Figure 8.

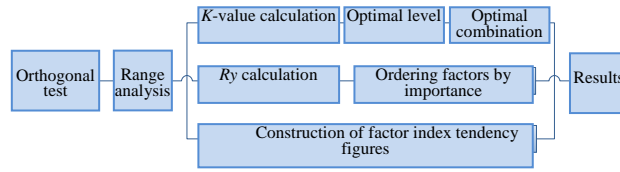


Fig.8 Flow chart for the optimization analysis of the test. R_y is the range in factor y . The K value is the sum of the statistical test results.

The four parameters, rockfall block radius, r , movement height, H , cushion thickness, h , and particle size, d , belong to the factor set $x \in (A, B, C, D)$, and the number of levels for all factors is four. The statistical test parameter of factor set x at level y can be calculated by determining K_{xy} ($x=A, B, C, D; y=1, 2, 3, 4$), i.e., the sum of all the test result indices P_{xy} containing level y of factor x , and dividing it by the total number of levels to obtain the average value k_{xy} in which P_{xy} is the random variable of the normal distribution:

$$k_{xy} = \frac{K_{xy}}{N_y} = \frac{\sum P_{xy}}{N_y}, \quad (6)$$

where K_{xy} is the statistical parameter of factor x at level y , k_{xy} is the average value of K_{xy} , and N_y is the number of levels.

k_{xy} can be used to judge the optimal level and combination of each factor. If a more optimal result is obtained at a higher index value, then the level that increases the index value should be selected, i.e., the level with maximum values for all factors k_{xy} ; conversely, if the smaller the index value is, the more optimal it is, the level with minimum values for all factors k_{xy} should be selected. The combination of parameters corresponding to an optimal level of all factors is the optimal parameter combination. R_y reflects the amount of variation of the test index with fluctuation in factor level y . The larger R_y is, the more sensitive the factor is to the influence of the test index. The order of importance of the factors can be judged using R_y , and the optimal level and combination of factor x can be judged from k_{xy} .

4.2.2 Results of analysis and discussion

Range analysis was used to analyze the orthogonal test results in Table 5. This uses the damage depth, L , of the cushion and the COR of the rockfall-cushion collision (Table 6) as influencing factors to determine the optimum combination of rockfall block radius, r , movement height, H , cushion thickness, h , and particle size, d , for the reduction of COR .

Table 6 Range analysis of two influencing factors for all evaluation indices

Evaluation index	Levels	Rockfall radius r/cm	Movement height H/m	Cushions thickness h/cm	Particle size d/mm
COR of collision between rockfall and cushion	k_{x1}	0.285	0.271	0.325	0.270
	k_{x2}	0.288	0.287	0.296	0.285
	k_{x3}	0.298	0.305	0.281	0.301
	k_{x4}	0.299	0.306	0.267	0.313
	R_y	0.014	0.035	0.058	0.043
Damage depth of cushion L	k_{x1}	0.97	0.78	0.76	1.75
	k_{x2}	1.12	0.99	1.26	1.35
	k_{x3}	1.33	1.38	1.40	0.94
	k_{x4}	1.44	1.72	1.44	0.81
	R_y	0.47	0.94	0.68	0.94

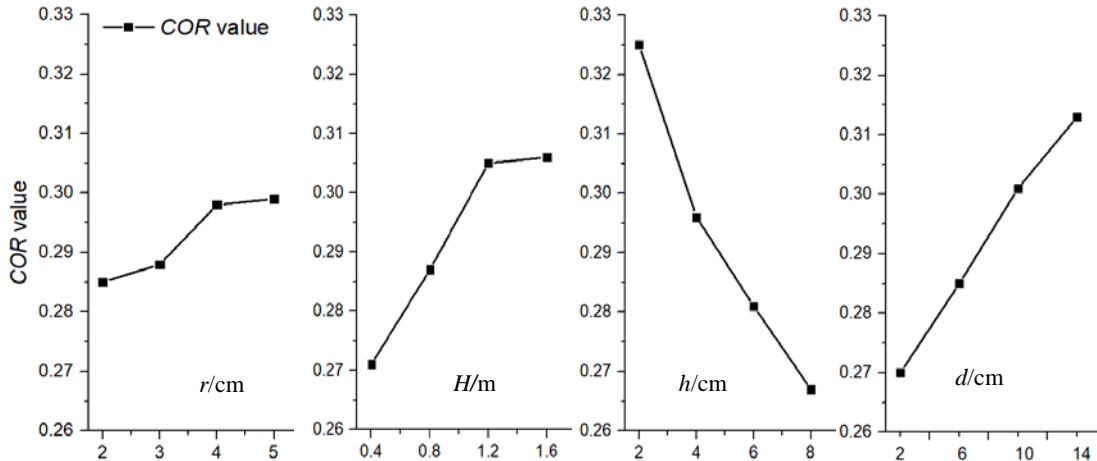
The following conclusions can be drawn from Table 6:

(1) The degree of influence of the four factors on the COR of the rockfall-cushion collision

316 is: cushion thickness (h) > particle size (d) > movement height (H) > block radius (r);

317 (2) The degree of influence of the four factors on the damage depth, L , of the cushion is:
 318 movement height (H) = particle size (d) > cushion thickness (h) > block radius (r).

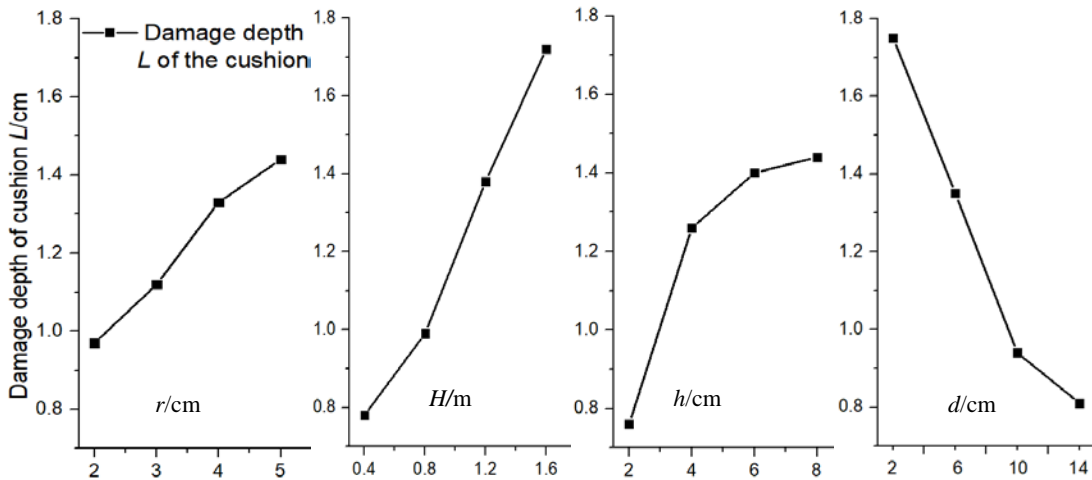
319 $E-I$ tendency figures (Tao et al., 2017) are used to further explore the effects of each factor on
 320 the test indices. The level of all factors is the X -coordinate (E), and the average value of the test
 321 index is the Y -coordinate (I). The $E-I$ tendency plots, Figure 9 and Figure 10, intuitively reflect the
 322 tendency of the test index with a change in factor level and can point the way to further testing.



323

324

Fig.9 Tendency of each factor as regards the COR of the rockfall-cushion collision



325

326

Fig.10 Tendency of each factor as regards damage depth L of the cushion

327 The following conclusions can be derived from Figures 9 and 10:

328 (1) The smallest optimal combination of parameters of the COR of the rockfall-cushion
 329 collision is A1B1C4D1; that is, when $r=2$ cm, $H=0.4$ m, $h=8$ cm, and $d=2$ mm, the COR of the
 330 collision is smallest (Figure 9).

331 (2) The shallowest optimal combination of parameters of the damage depth, L , of the cushion
 332 is A1B1C1D4; that is, when $r=2$ cm, $H=0.4$ m, $h=2$ cm, and $d=14$ mm, the damage depth, L , of the
 333 cushion is the shallowest (Figure 10).

334 To sum up, the cushion thickness, h , has the most significant influence on the COR of the
 335 rockfall-cushion collision, while it has a relatively minor effect on the damage depth, L , of the
 336 cushion. The second most important factor is particle size, d , it also can effectively affect the COR ,
 337 but the cushion can easily be destroyed when a rockfall with high kinetic energy collides with a

338 cushion of small particle size. The degree of influence of the rockfall block radius, r , on the two
339 indices is far less than that of the other factors. When a gravel cushion is used to control rockfall
340 down a slope, both the effectiveness with which it controls the rockfall and its durability are taken
341 into account (Pichler et al., 2005) so the cushion thickness, h , should be the primary consideration
342 in cushion design. The optimal thickness is 3–4 times the radius of the majority of the rockfall
343 blocks. The smaller the particle size is, the smaller the *COR* is, but the cushion is also more likely
344 to be destroyed. Therefore the appropriate particle size must be determined by combining the
345 expected block size and drop height of the rockfall so that the cushion not only achieves the effect
346 of reducing *COR* but also maintains its stability.

347 5 Conclusions

348 The buffering and energy-dissipation mechanism of gravel cushions with different properties
349 under different impact energies were studied in laboratory collision tests, leading to the following
350 conclusions:

351 1. Unlike conventional protection measures, a gravel cushion makes full use of waste
352 mullock produced in the process of mine extension, which can be conveniently broken up into
353 particles of the appropriate size. This can not only reduce the costs of reducing rockfall hazard and
354 of mullock transportation and relieve overloading of the mine's dump but can also achieve better
355 control of rockfalls, realizing the goal of "stone conquers stone."

356 2. In a series of laboratory tests, blocks of different radii were dropped from different
357 heights onto different cushion materials. The results indicate that, for a given impact energy, the
358 cushion thickness, h , has a strong influence on the measured coefficient of restitution (*COR*) and
359 therefore impact pressure. From the point where the ratio of the falling block radius, r , to the
360 cushion thickness, h , is 1/4–1/3, the rate of reduction in the *COR* with an increase in cushion
361 thickness gradually decreases. When the blocks move from a relatively low height, the *COR* of the
362 rockfall-cushion collision is more likely to be affected by the particle size than when blocks are
363 released from a greater height. Therefore, in the process of cushion design, the estimated physical
364 properties and drop height of the potentially dangerous rock should be investigated to estimate the
365 impact energy of the rockfall.

366 3. Through an orthogonal test, it is found that the cushion thickness, h , has the most
367 significant influence on the *COR* of the rockfall-cushion collision. The second most important
368 factor is particle size, d , with a smaller particle size leading to a smaller *COR*. However, the
369 cushion can easily be destroyed when a rockfall with high kinetic energy collides with a small
370 particle size cushion. Therefore, cushion design should take structural reliability as well as
371 effectiveness and any economic constraints into account. The appropriate particle size must be
372 determined on the basis of the block size and drop height of the expected rockfall so that the
373 cushion can not only achieve the effect of reducing *COR* but also maintain its stability.

374 4. Until now, it has not been possible to dictate a universal rule that the majority of
375 engineering personnel can follow in the design of gravel cushions for a platform. This is a
376 troubling blind spot. However, this work shows that, as well as increasing the cushion thickness,
377 changing its particle size can improve the rockfall-controlling effect, and that the optimal particle
378 size can be determined on the basis of the expected block size and drop height of the rockfall. This
379 provides a widely applicable theoretical and practical basis for cushion design for open-pit mine

380 rockfall protection.

381 **Acknowledgements**

382 This work is supported by the National Natural Science Foundation Item of China (No.
383 41502323), Beijing Natural Science Foundation of China (8142032) and China Postdoctoral
384 Science Foundation Funded Project (No. 2017M621212).

385 **References**

- 386 [1] Asteriou P, Tsiambaos G. Empirical model for predicting rockfall trajectory direction. *Rock Mechanics*
387 *and Rock Engineering*, 2016, 49:927-941.
- 388 [2] Aydin A. ISRM Suggested method for determination of the Schmidt hammer rebound hardness: revised
389 version. *International Journal of Rock Mechanics and Mining Sciences*, 2009, 46(3):627-34.
- 390 [3] Asteriou P, Saroglou H, and Tsiambaos G. Geotechnical and kinematic parameters affecting the coefficients
391 of restitution for rockfall analysis. *International Journal of Rock Mechanics and Mining Sciences*, 2012,
392 54(1):103-113.
- 393 [4] Asteriou P, Saroglou H, Tsiambaos G. Rockfall: scaling factors for the coefficient of restitution. *Eurock Rock*
394 *Mechanics for Resources, Energy & Environment*, Taylor & Francis Group, London, 2013, 19(2):109-113.
- 395 [5] Bouguet JY. Camera calibration toolbox for Matlab. http://www.vision.caltech.edu/bouguetj/calib_doc.
396 Accessed 20 Jan, 2012.
- 397 [6] Chau KT, Wong RHC, Wu JJ. Coefficient of restitution and rotational motions of rockfall impacts.
398 *International Journal of Rock Mechanics and Mining Sciences*, 2002, 39(1):69-77.
- 399 [7] Giani GP. Rock slope stability analysis. Balkema, Rotterdam, 1992.
- 400 [8] Huang R, Liu W, Zhou J, and Pei X. Rolling tests on movement characteristics of rock blocks. *Chinese*
401 *Journal of Geotechnical Engineering*, 2007, 29(9):1296-1302. (in Chinese)
- 402 [9] Howald EP, Abbruzzese JM, Grisanti C. An approach for evaluating the role of protection measures
403 in rockfall hazard zoning based on the Swiss experience. *Natural Hazards & Earth System Sciences*, 2017,
404 17(7):1127-1144.
- 405 [10] Koleini M, Van Rooy JL. Falling rock hazard index: a case study from the Marun Dam and power plant,
406 south-western Iran. *Bulletin of engineering geology and the environment*, 2011, 70(2):279-290.
- 407 [11] Kawahara S, Muro T. Effects of weight mass and drop height on vertical distribution of dry density of sandy
408 soil in one-dimensional impact compaction. In: *Proceedings of the 5th Asia-Pacific regional conference of*
409 *the ISTVS*; 1998:151-61.
- 410 [12] Kawahara S, Muro T. Effects of dry density and thickness of sandy soil on impact response due to rockfall.
411 *Journal of Terramechanics*, 2006, 43 (3): 329-340.
- 412 [13] Leine RI, Schweizer A, Christen M, et al. Simulation of rockfall trajectories with consideration of rock shape.
413 *Multibody System Dynamics*, 2014, 32(2):241-271.
- 414 [14] Lambert S, Heymann A, Gotteland P, et al. Real-scale investigation of the kinematic response of a rockfall
415 protection embankment. *Natural Hazards & Earth System Sciences*, 2014, 14(5):1269-1281.
- 416 [15] Labiouse V, Descoedres F, Montani S. Experimental study of rock sheds impacted by rock blocks.
417 *Structural Engineering International*, 1996, 6(3):171-176.
- 418 [16] Labiouse V, Heidenreich B. Half-scale experimental study of rockfall impacts on sandy slopes. *Natural*
419 *Hazards & Earth System Sciences*, 2009, 9(6):1981-1993.

- 420 [17] Mignelli C, Peila D, Russo SL, et al. Analysis of rockfall risk on mountainside roads: evaluation of the effect
421 of protection devices. *Natural Hazards*, 2014, 73(1):23-35.
- 422 [18] Monnet JM, Bourrier F, Dupire S, et al. Suitability of airborne laser scanning for the assessment of
423 forest protection effect against rockfall. *Landslides*, 2017, 14(1):299-310.
- 424 [19] Miranda SD, Gentilini C, Gottardi G, et al. Virtual testing of existing semi-rigid rockfall protection barriers.
425 *Engineering Structures*, 2015, 85:83-94.
- 426 [20] Notaro S, Paletto A. The economic valuation of natural hazards in mountain forests: an approach based on
427 the replacement cost method. *Journal of Forest Economics*, 2012, 18 (4): 318–328.
- 428 [21] Pantelidis L. An alternative rock mass classification system for rock slopes. *Bulletin of engineering geology
429 and the environment*, 2010, 69(1):29-39.
- 430 [22] Pichler B, Hellmich C, Mang H A. Impact of rocks onto gravel design and evaluation of experiments.
431 *International Journal of Impact Engineering*, 2005, 31(5):559-578.
- 432 [23] Pei X, Liu Y, Wang D. Study on the energy dissipation of sandy soil cushions on the rock-shed under rockfall
433 impact load. *Journal of Sichuan University (Engineering Science Edition)*, 2016, 48(1):15-22.(in Chinese)
- 434 [24] Saroglou H, Marinos V, Marinos P, Tsiambaos G. Rockfall hazard and risk assessment: an example from a
435 high promontory at the historical site of Monemvasia, Greece. *Natural Hazards & Earth System Sciences*,
436 2012, 12(6):1823–1836.
- 437 [25] Sadagah B. Back analysis of a rockfall event and remedial measures along part of a Mountainous Road,
438 Western Saudi Arabia. *Int J Innov Sci Mod Eng*, 2015, 3(2): 2319-6386.
- 439 [26] Sun J, Chu Z, Liu Y, et al. Performance of used tire cushion layer under rockfall impact. *Shock and Vibration*,
440 2016, 2016 (10):1-10.
- 441 [27] Topal T, Akın M, Özden AU. Analysis and evaluation of rockfall hazard around Afyon Castle, Turkey.
442 *Environmental Geology*, 2006, 53 (1) :191-200.
- 443 [28] Tao Z, Zhu C, He M, et al. Test of V shaped groove structure against rockfall based on orthogonal design.
444 *Journal of China Coal Society*, 2017, 42(9):2307-2315.
- 445 [29] Ulusay R, Hudson JA. The complete ISRM suggested methods for rock characterization, testing and
446 monitoring: 1974–2006. Ankara: ISRM Commission on Testing Methods; 2007.
- 447 [30] Yang Y, Zhou Y, Jiang R, et al. Theory and practice of slope geological disaster flexible protection
448 [M].Beijing: Science Press, 2005. (in Chinese)
- 449 [31] Zhu C, Tao Z, Yang S, et al. V shaped gully method for controlling rockfall on high-steep slopes in China.
450 *Bulletin of Engineering Geology and the Environment*, 2018:1-17.
451 <https://doi.org/10.1007/s10064-018-1269-7>.