

Interactive comment on “Geophysical and Geodetical Investigation of A Landslide Area (Koyulhisar-Sivas, Turkey)” by Sevda Özel et al.

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REVISIONS (for Final Response) Dear Editor

I rearranged and revised the comments and corrections for the article. I submitted the article.

With kind regards, Sevda Özel Corresponding writer

C2 RESPONSES: General comments SC1. Considering the text of the paper, the authors should keep their sentences short and clear instead of trying to over-impress by writing complex sentences, which can often leave the reader tired and confused. Therefore, explanations are generally not clear. ACs1: I checked them. Due to the continuous change of topography, we selected the parallel lines. Contribution to com-

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ment was considered.

SC2. Introduction and mainly whole text include many repetitions. Also, the same conjunctions used in the sentences following each other are unnecessary and very annoying. For instance, “it was understood that” was used four times in the conclusion part of the paper. This may reflect that the paper present as if the results of a report. ACs2: I rearranged this section.

SC3. It must be used the true symbolic character for explanations related to the degree, for instance, 5 degree given in the abstract and the best parts of the paper must be revised as 5 degrees. ACs3: It should be 50. There is an error in writing. It was revised.

SC4. Fig. 8 must be presented earlier in the paper or the design of the article should be revised by the authors. Why readers are forced to look at Fig. 8 after the first figure mentioned in the introduction part? ACs4: I deleted Fig. 8. It was rearranged in Fig. 8.

SC5. References should be carefully checked by the authors since the correct surnames of the researchers have not been provided (for example, Adlaş, Baklaya, should be corrected as Aldaş and Balkaya respectively). ACs5: I rearranged them.

SC6. Abbreviations of geophysical methods should be defined at the first occurrence by following their long names. For instance, ground-penetrating radar (GPR), instead of GPR (ground-penetrating radar). ACs6: I controlled them and rearranged.

SC7. It would be so nice if the data acquisition and evaluation parts of both geophysical methods applied in the survey should be explained and collected in a separate section of the paper. This information is widely spread in this form of the article. ACs7: It was rearranged.

SC8. The numbers on the figures are not clear. ACs8: They were a result of PDF conversion. I checked.

Scientific comments SC 1. It is obvious that researchers were not successful to gen-

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erally identify the geometry of the landslide under consideration. The results obtained show that both SRT and GPR measurement profiles are still between the boundary of the landslide area. To identify share plane, both crown and toe of the landslide must be cut by longitudinal geophysical transects. Therefore, there is no meaning of parallel profiles studied. ACs 1: The landslide area is actually very large (in km). However, this was not our plan. This can be in a large project. I would like to do this in the future and I can do it. This study contains a small area. This area is expressed in the article. The study area is approximately 200x250 square meters in total and is not flat in topography. Therefore it has been studied as separate areas and different direction lines. It was also important for geological layers.

SC 2. An electrical resistivity tomography (ERT) study could be more proper geophysical technique instead of ground-penetrating radar (GPR) together with the seismic refraction tomography (SRT) for investigation. ACs 2: I responded in SC1. But the landslide area should be studied by ERT method. However, there are also difficulties in topographically and geomorphologically. Anyway, I thought about that. I think as a Project for my master student. The study area of the MS Project is larger than 200x250 m².

SC 3. The authors must provide correct scientific definitions for the terms or explanations used. For instance, closed(?) antenna and inversion processing of the GPR data mentioned in the text. It is not clear what is meant here inversion term (time-depth conversion or data analysis). ACs 3: Yes, I mentioned in the text "shielded antenna and data analysis of the GPR data". I rearranged them. Static correction: 10 ns in dry or wet clay and sandy. Muting: It is different in each line. Bandpass filter: 100, 200, 300, 400 Hz. Gain: 0.512 ms. Migration: 0.01 ms.

SC 4. Figure 3 must be presented more illustrative since all images from the area are intertwined, and it is not understood which region of the study area represents. ACs 4: Figure 3 shows the surface deformations both the study area and surrounding. It was showed the geophysical line sites in the study area, in Figure 2.

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SC 5. The explanation of GPR velocity chosen for the time-depth conversion is not adequate since dry or wet soils and clays and sand environment can display various values depending on different environments. ACs 5: I observed them in covered geological units and drilling logs.

SC 6. Vp value of 0.1 km/s is not a low seismic velocity since the values bigger than 0.07 km/s can be also defined as a rock medium. ACs 8: I controlled them. There is an error in writing. I revised it.

SC 7. Considering the seismic observations, the authors must analyze the safety region of the seismic tomograms and provide a comparison of observed and calculated travel times at least for one section. ACs: We had calculated travel times (see from Över (2015)). We used its results in the article. Therefore, it was only showed the geological layering in this study. I controlled and revised them.

SC 8. The results of GPS measurements showing the amount of displacement in the survey area belongs to previous researchers. Therefore, it is seen that the geodetic investigation is not one of the main parts of this exploration. ACs 8: Exactly, it's true. But we had to use this information in this article. The geophysical study joined to Hastaoğlu et al. (2015) Project later. Therefore, the geological layers were important in the Project. It was necessary for interpretation. The landslide area is very large, but our study area is very small. We explained this in the article. However, the rainfall data of the project were never shared in another article. We added them to this article and also compared them with MGM data.

SC 9. Comparative figures considering the results of SRT and GPR at the same scale must be provided by the authors. ACs 9: I tried these, but the size of the figure and section-solution was a problem. I rearranged them.

Comments on the figures: Response for Figure 4: I have tried. But, its image didn't good and so I removed it.

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Response for Figure 6-7: I revised section.

Response for Figure 8: I deleted Fig. 8. It was rearranged in Fig. 8. I rearranged the figure numbers in the article text.

Other corrections:

It was a response to the C2 comments in the C1 comments. I checked and rearranged "Geology, Methods, References" sections. I checked and revised all of the figures in the article text. The seismic velocities were presented as m/sec in the article.

Geophysical and Geodetical Investigation of A Landslide Area (Koyulhisar-Sivas, Turkey) Sevda Özel¹, Demet Över², Kemal Ö. Hastaoğlu³ ^{1,2}Department of Geophysical Engineering, Cumhuriyet University, 58140, Sivas, Turkey ³Department of Geomatics Engineering, Cumhuriyet University, 58140, Sivas, Turkey Correspondence to: Sevda Özel (sozel@cumhuriyet.edu.tr.) Abstract. The study area is in the west of Koyulhisar (Sivas) town center and approximately 200x250 m². This area is one of the most active locations where the landslide displacement amount is the greatest. The aim of this study is to determine the depth of the sliding surface with geophysical (seismic refraction tomography (SRT), ground-penetrating radar (GPR)) methods. The results of TÜBİTAK-111Y111 project were also used. According to the geophysical results, within ~20 m of investigation depth, three layers with the average seismic P-wave velocities (VP) of 650, 1200 and 2100 m/sec were identified. It was determined that the depth of the sliding surface which was between ~3-7 m and the seismic velocities were lower than 650 m/sec from these depths to the surface. The geophysical results demonstrated that the landslide type was identified as planar sliding, with the sliding direction in S-SE, and the tilt of the geological layer was in the same direction with the topography slope, mostly bigger than 5°. It was observed that the deformations in the landslide mass were caused by the geological unit, the layer or topography slope, and precipitation. All of these results can be effective in triggering the landslide area in the future and the landslide activity may continue in the study

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area. Therefore, the study area contains the risk and the natural hazards, and these threaten the settlement area and other constructions in Koyulhisar. 1 Introduction A landslide is a mass movement and can occur in different forms. Koyulhisar landslide area is one of the largest landslide areas, significantly, leading to serious loss of lives and property, in Turkey. Three of the most destructive of these landslides occurred in Koyulhisar (Sivas) on 19 August 1998, 20 July 2000 and 17 March 2005. The Koyulhisar landslide area is one of the most important large landslide areas in the country and mass movements there typically occurs in the form of debris or mudflow (Tatar et al., 2007; Duman et al., 2005). Koyulhisar is also an active landslide area and for the past 17 years, there has been observed an increase in landslide activity (Tatar et al., 2007; Över, 2015). The large and small landslides in Koyulhisar landslide area have mostly occurred due to natural causes until today. Artificial causes mainly constitute the landslides caused by human interventions (blasting, drilling, improper planting, loading, loss of vegetation cover, etc.). The last large landslide occurred with the flow of mud in the north of Koyulhisar landslide area in March 2005. Duman et al. (2005) determined that this landslide was in the excessively fast (6 m/sec) class. Demirel et al. (2016), for the landslide in 2000 years revealed an average of 2.5-7.4 mm/year slip rate. Researchers have stated that these landslides usually have a mechanism involving a circular rotation, this old landslide mass maintains its activity and partial landslides occur on the groundmass (Sendir and Yâşlmaz, 2001; Duman et al., 2005). Therefore, Koyulhisar district center is on an old landslide that occurred in the form of circular rotation. The front of this landslide mass is open, it is always active, activity is not massive and usually in the form of local landslides occurring on the groundmass (Sendir and Yâşlmaz, 2001). The triggering mechanisms of landslides are often complex and further understanding is needed to facilitate the prediction of mobilizations as well as adequate stabilization and remediation measures. Therefore, it is important to investigate the reasons that affect the formation mechanisms and the formation of landslides. Different engineering (geology, geophysics, geodetic, etc.) disciplines have great role and importance especially in decreasing the landslide effects. They can

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of landslides and the area affected by the landslides and between the magnitude and the maximum distance of landslide observations from the epicenter in different geological, topographical, and climatic conditions (Demir, 2018). Large earthquakes affecting Koyulhisar district also occurred in the region. These largest earthquakes are in the south of the NAFZ or Suşehri district and a total of three large earthquakes with $M \geq 5.6$ occurred there (Över, 2015). Among these, the 1992 earthquake is closest to the study area with the least depth but the second largest earthquake (Fig. 2). This earthquake is an earthquake with 6.1 magnitudes that occurred 10 km below the ground. The large earthquakes in the south of Suşehri district which is just 13 km away from the study area occurred in 1909 and 1939. 1909 earthquake occurred 60 km below the ground and is the largest and deepest earthquake with a magnitude of 6.3. 1939 earthquake is also deep and the third largest earthquake that occurred 50 km below the ground with a magnitude of 5.6 (Över, 2015). In addition, when Fig. 2 is analyzed, it is seen that the magnitudes of the other earthquakes in the north of the NAFZ and the upper elevations of the landslide generally vary between 2.5-4. Similarly, it is seen that the other earthquakes in the south of the landslide area are the earthquakes with a magnitude of greater than 3.6. All these earthquakes may have triggered the landslide mass from time to time in places where sliding surfaces, layers, and topography in the landslide area are more inclined than 5-10 degrees (according to the geophysical cross-sections in this article, when it is considered that there are loose units and deformations on the sliding surfaces). In particular, they further affected the landslide mass along with the rain and caused large amounts of displacement in the landslide area.

2.2 Meteorological and geodetic results The data regarding the rainfalls with the effects of triggering the landslides are presented in Table 1 and Fig. 3a and Fig. 3b (MGM, 2016; Hastaoğlu et al., 2015). With these data, the rainfall status of the study area and its surrounding was examined by months as average annual rainfalls and the annual areal amount of rainfall. According to the data obtained between 1950-2015 in Table 1, the rainy periods are generally between October-November-December and January-February-March-April. The highest total

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daily amount of rainfall in the rainiest years was observed as snowfall in 1950 (110 cm) and as rain in 1991 (55 kg/m²). In Fig. 3a, the annual normal average rainfall value calculated for the years between 1981-2010 was calculated as over 483.4 mm (MGM, 2016). However, 1987-1988 and 1997-1998 were the rainiest years. It is seen that the annual areal amount of rainfall exceeded the normal values and was higher than 550 mm in these rainy years that took place in every 10 years. Similarly, it is also seen that there were high rainfalls for 3-4 years after the years of 1985-1995-2005 with an interval of 10 years. Therefore, annual areal rainfalls were observed to be more before some large landslides like the landslide in 1998. When geological features of the region are taken into account, it is remarkable that the landslide in 1998 and 2000 occurred in the summer months after the winter with a heavy fall of snow. However, the landslide in 2005 occurred during the rainy season. In Fig. 3b, GNSS studies and multi-disciplinary studies of Hastaoğlu et al. (2015) have carried out for many years (about 6 years) to determine the deformation and annual sliding amounts especially after the landslides in 1998-2000-2005. The seismological and meteorological data, which were updated by the geodetic (GNSS (DH), geological (IDH (Inclinometer Drilling Holes)) and meteorological data collected in this local study were reorganized and evaluated. Fig. 3a-b and Table 1 which were repared for the study which is the subject of this article were associated with the results of GNSS studies (Fig. 3b). The monthly and annual meteorological data should certainly be evaluated particularly within the scope of monitoring activities, as the area is a landslide area. Hastaoğlu et al. (2015) followed in DH wells in the area in 2013-2014 (Fig. 3b). If Fig. 4 is examined, there are seven DH point in the nearest of the geophysical profiles (DH8, DH12, DH16 is near the area A and DH4, DH6, DH9, DH10 is near the area C). The graphics in Fig. 3b was prepared from the combined data (unpublished data in the project) and the temperature (OC), precipitation (m3) and soil moisture content (cm) were compared in these graphics. The temperature and precipitation were observed to be inversely proportional during the summer months called as a dry period. It is seen that the soil moisture is changeable apart from the rainy period and has very

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high water content during the rainy periods. The soil moisture is very high (average 150 cm) in winter, summer, autumn seasons. In the study area, the water contents in the drilling data change from 24.6 % to 13.3 % at between 0-10 m depth and these values are also high (from 29.1 % to 17.3 %) after 10 m (Hastaoğlu et al., 2015). Water generated from precipitation and melting snow is blocked by the impermeable layer when it infiltrates downward, and the local moisture content increases (see Hu and Shan (2016)). Thus, the water infiltrates the interface between the permeable and impermeable layer, can form a slip zone. Then, these results were compared with geophysical results in interpretation. The GPR results show that the moisture content of soils at the sliding surface of the landslide mass is relatively high. The drilling data and soil moisture values also show very high moisture content of the sliding surface of the landslide mass in the study area, which is completely consistent with the results obtained from the GPR-SRT profiles, meteorological and geological results. On the other hand, it was understood that the precipitation increased by the decrease in temperatures. It is also seen that the total annual amount of rainfall increased about 2-fold in 2014 compared to 2013 (Fig. 3a-b). According to all results, rainfalls are considered to be effective in triggering of the landslide because the ground of this landslide area, which is filled with loose units and old cracks, is supersaturated with water due to the rainfalls. Besides, Hastaoğlu et al. (2015) determined that the groundwater level gets close to the surface for 4-6 m on average at the end of the rainy period, to 10 m at the end of the rainy period and decreases up to 25 m in some wells in the area where geophysical study area is also located, and the groundwater flow direction is SW. Consequently, when the displacements and the landslide directions estimated from the GNSS measurements are also considered, it was determined that these results were compatible with the geophysical sections and the rainfalls were among the reasons that trigger the landslide.

3 Methods 3.1 Geophysical surveys The seismic refraction tomography (SRT) and ground-penetrating radar (GPR) methods are applied in tomography format. The SRT method determining the seismic P-wave velocities (VP) for seismic applications and the GPR method for electromagnetic (EM)

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applications were used in the geophysical data collection in the area (Fig. 4). The high-frequency electromagnetic waves can reach deeper in the environments with low conductivity like sand. However, the conductive units such as clay and shale decrease the penetration depth of the signal transmitted and lead to absorption (Annan et al., 1988; Davis and Annan, 1989). Firstly, SRT and GPR data were collected along multiple transects in two different areas of the study area named A and C (see Fig. 4). Then, the geophysical profiles were processed to the satellite map according to the coordinates along with the topographical elevation curves and GNSS measurement locations for the ease of interpretation (Fig. 4a). Geophysical measurements were taken due to the geologic bedding and topographic features (Fig. 4b-c). SRT profiles and on these seismic profiles GPR profiles in the area defined by A in Fig. 4b is approximately in the NE-SW (SRT2, SRT4, GPR2, GPR4) and NW-SE (SRT3, SRT5, GPR3, GPR5) directions (Fig. 4b). Similarly, in area C, SRT11-SRT12-GPR11-GPR12 profiles are approximately in the E-W directions, SRT9-SRT14-GPR9-GPR14 profiles are approximately in the NE-SW direction, SRT10-GPR10 profiles are in the NW-SE directions and SRT13-GPR13 profiles are approximately in the NE-SW directions (Fig. 4c). The profile lengths usually range from 25 to 60 m according to the method applied. The profile shooting technique in the seismic study, hammer and iron plate of 8 kg weight as the source P geophone of 14 Hz (the total number of geophones is 12) and Geometrics branded seismic device as the receiver was used while collecting the SRT data. In all profiles, the geophone interval was 5 m, offset distance was 2.5 m, the sampling interval was 256 ms and the record length was 512 ms. The geophones were respectively fixed on the ground within the selected geophone range and their connections with the seismic device were made. Then, seismic measurements were recorded by starting from the offset distance of 2.5 m, reducing to sledgehammer plate and making at least 5 times shots between each geophone, respectively. In the evaluation of the SRT data collected in the field, SeisImager program was used for displaying, processing and evaluation of the seismic refraction waves. The marking of the first arrivals of the SRT data was performed using Pickwin, and the evaluation

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of the first arrival data was performed using Plotrefa module. The GPR data were collected by Ramac2 device using a shielded antenna of 250 MHz. The GPR data were processed in Reflexw program. In order to collect the GPR data, other parameters were selected 512 ns-number of samples, 16-number of stacking and 0.1 m-trace interval. 2D GPR data processing for data analysis of the GPR data, it includes Static correction (10 ns in dry or wet clay and sandy), Muting, Bandpass filter (100, 200, 300, 400 Hz), Gain (0.512 ms) and Migration (0.01 ms) steps. The migration was made to show up small vertical structures invisible during data processing. Thus, very large hyperballs with strong reflections may limit the display of non-migrated GPR data. Moreover, the peak points of hyperbolas observed in GPR cross-sections show the reflection surface of the electromagnetic wave. During data processing, velocity analysis was performed on the reflection surfaces through the hyperbola superposition method and EM wave propagation velocity was calculated in all GPR cross-sections. The topographic corrections were made by selecting the "Correct for two layers" option in Static Correction/Muting in the Reflex program. The height values collected in the study area were manually entered and saved in the "Correct for two layers" option. Thus, the models were converted from m to ns and the GPR sections were prepared for interpretation. Thus, the collected geophysical data were converted into 2D (two-dimension) elevation-distance (SRT) and depth-distance (GPR) sections by assessed in the appropriate software. The geophysical study area is one of the most active locations of the landslide area. As it is seen in Fig. 5, geomorphologically the landslide cracks on the surface, displacement traces, and structural damages in the study area and its immediate surroundings can be monitored clearly in this activity area. Visibly, the damaging effects of still active or old landslides on residences, roads and walls are also observed easily by field observations. Therefore, none of the damaged constructions are used in the Koyulhisar. 3.2 Geophysical analysis, results and discussion Geophysical interpretations were made according to these sections and compared with the results of the other studies. SRT sections: 2D seismic cross-sections giving seismic VP-depth information are presented in Fig. 6 and 7.

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In the seismic data evaluation, the coincidence was provided with RMS (Root Mean Square) errors ranging between 3.4-4.5% in 2D inversion operation. According to these sections, two or three layers were identified at about 20 m depth (Fig. 6 and 7). It was understood that the tilts of these layers were southeast oriented, and their tilt was greater than 50. According to the average seismic velocities (VP) calculated, three layers with the layer velocities of 650, 1200 and 2100 m/sec were defined from top to bottom. Thus, the seismic VP velocities were observed that they increased towards the depth. It was determined that the depth of the sliding surface varied about between 3-7 m (Fig. 6 and 7). Therefore, these depths were defined as the layer with the risk of dislocation. This area was considered to have a risk of dislocation due to these loose units, rainfall and tilt conditions. The seismic velocity of the first layer is lower than $VP1 < 650$ m/sec, but the seismic velocity of the third layer may be greater than $VP3 > 2100$ m/sec. GPR sections: The investigation depth was further calculated from the SRT sections compared to the GPR sections due to the differences of geophysical methods in the application. Because GPR sections were obtained in high-resolution for about the first 10 m depth after data processing of the GPR data. It is clearly observed that the strong reflections are within 10 m depth in Fig. 8 and 9. These strong reflections seen in black dashed ellipses are interpreted as deformation areas in the layer. In a similar manner, these areas being interpreted as deformations were also observed in the studies of Bubeck et al. (2015), Hu and Shan (2016), Su et al. (2016) and Popescu et al. (2016). The strong reflected wave signal shows distinctive characteristics, presenting a low-frequency high-amplitude sync-phase axis, which can be inferred as the sliding surface in Fig. 8 and 9. In other words, two layers were identified in GPR sections. The first layer is weak, loose, cracked, moved and also have lost their tightness, and their seismic velocity is low. Therefore, in Fig. 8 and 9, it was thought that deformations developed on the sliding surfaces due to the geology of the study area in A and C area. It was identified the deformations, called sliding surfaces, landslide furrows, scarps, collapsed zones, and cracks. If the areas of A and C are compared, the deformations are more in area C than in area A. Therefore, the

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risk of landslides may be higher in area C. In Fig. 8, the EM wave velocity calculated for the reflection surface in GPR5 cross-section -representing the GPR profiles- was shown as an example. The picks were exported with the attribute of two-way travel time and the velocity of propagation of the wave was calculated about 0.1 m/ns (Fig. 8). This value is generally observed in dry or wet soil, dry or wet clay and sandy environments (Wilchek, 2000; Cardomina, 2002). Therefore, it was thought that this velocity value was compatible with the geological units and electromagnetic waves led to rapid absorption due to the silty sandy clay layer. Because the first geological unit is medium-very stiff, low-high plasticity, silty sandy clay. The deformation structures as sliding surfaces, landslide furrows, scarps, collapsed zones, and cracks were observed in the GPR cross-sections (Fig. 8 and 9). In other words, the geological unit, the layer or topography slope and precipitation cause deformations in the loose upper unit. Therefore, these structures may develop or occur in the landslide mass, as shown in Fig. 8 and 9. Additionally, the geological units were observed in DH wells in the geophysics study area (Fig. 4). These are mostly silty sandy clay and they have different characteristics above and below about 10 m in DH well. The topography of the study area decreases from 925 m to 840 m and the elevation difference is 85 m (Fig. 4). The amount of slope in the topography increases from south to north (>50-100) in the geophysical sections (Fig. 6 and 7). It was determined that the landslide type in the area was planar sliding and observed that the direction of sliding was SE. As this information was associated with topography and the field observations, it was observed that the topography was inclined from the north to the south of the study area. The results of the various studies and also the findings of this article have proved that Koyulhisar landslides are generally caused by the known reasons that trigger the landslide. Therefore, it was seen that the geological bedding was compatible with the topographical sloping and the groundwater was compatible with the direction of flow. 4 Conclusions The landslides may develop under various geological, morphological, topographical and physical reasons. The information provided from many studies (geodetical, geological, morphological, seismological,

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topographic and meteorological) carried out across the region was compared with the geophysical results (SRT and GPR) and found to be compatible. The seismic P-wave velocity (VP) of the layers, the tilt, tilt direction of the layers, depth of the sliding surface, sliding direction and the landslide type was determined from the geophysical sections. The study area was identified by the layers with the average seismic velocities of $650 < 1200 < 2100 < \dots$ m/sec. According to the geophysical cross-sections, it was identified that the depth of the sliding surface varied between 3-7 m due to the topographical differences. These depths were the depths with low seismic velocities (<650 m/sec) and defined as loose units which were also observed in geological drilling logs. It was determined that sliding surfaces, landslide furrows, collapsed zones, scarps, cracks were observed in the landslide mass in the GPR sections. It was observed that the layer tilt was generally more than 50 in all geophysical sections and compatible with the geology and the flow direction of the groundwater. It was determined that the landslide type in the area was planar sliding and the direction of sliding was SE. The geophysical and other results were found to be compatible because it is known that the landslide direction across Koyulhisar is in S-SW and SE. Consequently, the fact that the depth of the sliding surface over the geologic unit is loose, the seismic velocity of the upper layer is low and the tilt is an excessive show that there is a new risk of landslide in the area. The other factors that trigger the landslide were found to be associated especially with the fact that the area is seismically active, receive heavy rain and has a poor vegetation cover. On the other hand, it was thought that blasting and excavation performed by human intervention can trigger the landslides due to the geologically loose unit. Hence, the landslide area can be a potential area which is open to natural/artificial hazards. The identified risks and natural hazards also threaten the settlement area, the buildings and other constructions (e.g. roads, walls, parks et al.) in Koyulhisar. Therefore, there is still a high landslide hazard in the study area and its surroundings, and this hazard will also occur in the future. Acknowledgments: This study was supported by Cumhuriyet University Scientific Research Projects Commission as CÜBAP Project numbered M-464. We would like to thank Project

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Coordinator Assoc. Dr. Kemal Özgür Hastaoğlu and his research team, who allowed us to benefit from the results of TÜBİTAK (The Scientific and Technological Research Council of Turkey) supported project numbered 111Y111, for their contributions. We would like to thank Dr. Çağrı Çaylak for his contributions during the geophysical field measurements, Assoc. Dr. Fatih Poyraz for his contributions during the process of geodetic field measurements. We would like to thank Geological Engineer Mehmet Demirel for his contributions to the Fig. 2. References Aldaş, G. U., Kadıoğlu, S., Uluggerli, E. U.: The effects of concealed discontinuities in blast design Pattern, 4th Int. Scientific and Technical Conference of Young Scientists and Specialists, St. Petersburg-RUSSIA, 6-7, 2003. Annan, A. P., Davis, J.L., Gendzwill, D.: Radar sounding in potash mines: Saskatchewan, Canada, *Geophysics*, 53, 1556-1564, 1988. Bano, M., Marquis, G., Niviere, B., Maurin, J. C., Cushing, M.: Investigating alluvial and tectonic features with ground penetrating radar and analyzing diffractions patterns, *J. Appl. Geophys.*, 43, 3-41, 2000. Benson, A. K.: Applications of ground penetrating radar in assessing some geological hazards: Examples of groundwater contaminants, faults, cavities, *J. Appl. Geophys.*, 33, 177-193, [https://doi.org/10.1016/0926-9851\(95\)90040-3](https://doi.org/10.1016/0926-9851(95)90040-3), 1995. Bichler, A., Bobrowsky, P., Best, M., Douma, M., Hunter, J., Calvert, T., Burns, R.: Three-dimensional mapping of a landslide using a multi-geophysical approach: the Quesnel Forks landslide, *Landslides*, 1, 29-40, DOI 10.1007/s10346-003-0008-7, 2004. Bubeck, A., Wilkinson, M., Roberts, G. P., Cowie, P. A., McCaffrey, K. J. W., Phillips, R., Sammonds, P.: The tectonic geomorphology of bedrock scarps on active normal faults in the Italian Apennines mapped using combined ground penetrating radar and terrestrial laser scanning, *Geomorphology*, 237, 38-51, DOI:10.1016/j.geomorph.2014.03.011, 2015. Cardimona, S.: Subsurface investigation using ground penetrating radar, Presented at the 2nd International Conference on the Application of Geophysics and NDT Methodologies Transportation Facilities and Infrastructure, Los Angeles, California, 2002. Davis, J. L., Annan, A. P.: Ground-penetrating radar for high resolution mapping of soil and rock stratigraphy, *Geophys. Prosp.*, 37, 531-551, DOI: 10.1111/j.1365-2478.1989.tb02221.x, 1989.

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2018-153, 2018.

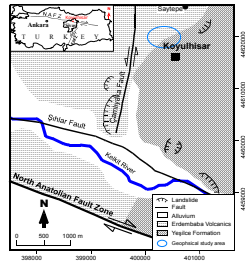


Figure 1. Geological map of study area. Arranged from Sendir and Yilmaz (2002) and Hastaoglu (2016).

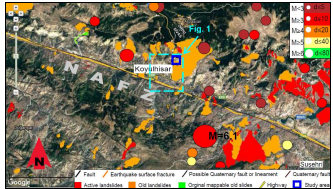


Figure 2. Seismic activity of the study area and its surroundings by the data between 1900-2015 years and the landslide areas (UDM, 2016; MTA, 2018).

Fig. 1.

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Table

Table 1. The annual average meteorological values of Sivas by years between 1950-2015 (MGM, 2016).											
Values	January	February	March	April	May	June	July	August	September	October	November
The average temperature (°C)	-3.2	-2.0	2.9	9.1	13.5	17.2	20.2	20.2	16.2	10.8	4.6
The average the highest temperature (°C)	1.0	2.6	8.1	15.3	20.0	24.0	27.0	26.5	24.7	16.4	10.6
The average the lowest temperature (°C)	-7.0	-6.2	-1.7	3.4	7.2	9.9	12.0	11.9	8.3	4.4	-0.2
The average number of rainy days	2.3	3.3	4.3	6.2	8.1	10.4	12.1	11.4	9.4	6.3	4.1
The average number of rainy days	13.0	12.4	13.7	14.0	14.4	8.8	2.5	2.1	4.3	9.0	9.5
The average monthly total rainfall (mm)	42.0	40.3	46.0	59.1	60.7	34.8	8.5	5.9	16.9	32.9	41.0
The highest and the lowest values according to the years (1950-2015)											
The highest temperature (°C)	14.5	18.1	25.2	30.0	32.0	35.5	40.0	38.4	35.7	30.5	22.8
The lowest temperature (°C)	-34.6	-34.4	-27.6	-18.9	-4.2	-0.3	3.4	3.2	-3.8	-8.1	-24.4
Rainy season											
Rainy season	2 May 1990	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
The highest value	2 May 1990	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0

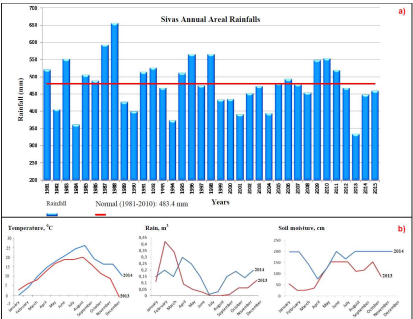


Figure 3. a) Precipitation distribution in between 1981-2015 years of Sivas (MGM, 2016). b) Graphics of monthly average temperature (T, °C), rainfall (mm) and soil moisture content (cm) of the study area and its surroundings in the years of 2013 and 2014. They were prepared from the project data (Hastaoglu et al., 2015).

Fig. 2.

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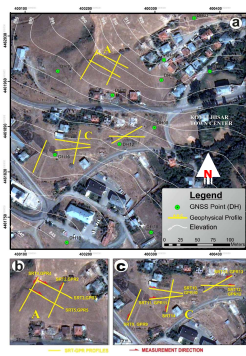


Figure 4. (a) The study area. (b) and (c) The details of geophysics profiles for the A and C areas.

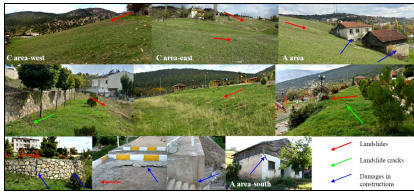


Figure 5. The photos of the study area and its surroundings, in which the landslides, landslide cracks or constructional damages are also observed.

Fig. 3.

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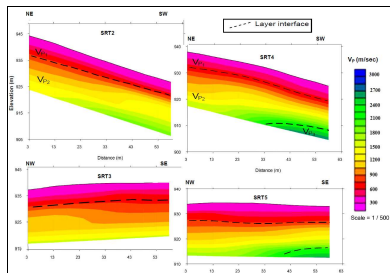
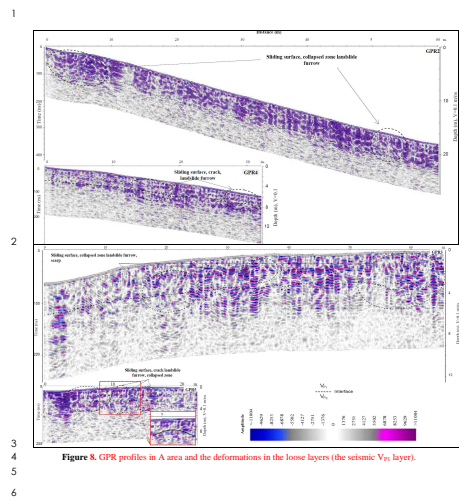


Figure 6. The seismic profiles of the area A. The uppermost boundary of the V_p layer is the depth of the sliding surface (This depth changes between $\sim 3\text{--}7$ m). The lower velocity V_p layer consists of soil and alluvium (the average seismic $V_p < 650$ m/sec).

Fig. 4.

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Fig. 5.

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