# Response to Reviewer's Comments

The authors would like to thank the reviewer for the careful reading and constructive comments that have helped sharpen this manuscript. In this revision, all the comments of the reviewers have been carefully addressed. Specific responses to the review comments are listed below. The line numbers refer to those in the revised manuscript.

**Anonymous Referee #1**

***1. Introduction: Pg 1_line 28 – Geological conditions here are referred to as an external factor influencing landslides. Geological conditions should be considered as an internal factor as is later suggested in the manuscript.***

Response: Thank you for the careful reading. Indeed, geological conditions are considered as an internal factor in the manuscript. We have revised the manuscript accordingly. Please see **Introduction: Pg 1_lines 35-36** in this revision.

***2. Introduction: Pg 1_line 31-35 – These final sentences of the first paragraph should really be the start of the introduction as this sets out the general motivation for the study before linking this to the site.***

Response: Thank you for the careful reading and constructive comment. We agreed with the reviewer. We have revised the text accordingly. Please see **Introduction: Pg 1_lines 28-33** in this revision.

***3. Introduction: Pg 2_lines 31-34 – I am not sure that this is needed I would suggest deleting this.***

Response: Thank you for the comment. In this submission, we have revised the text accordingly. Please see **Introduction: Pg 2_lines 35-36** in this revision.

***4. Methodology: Pg 2_ lines 38-39 – I'm not sure I fully understand this point. Landslide displacement is caused by both internal and external factors but why does the lithology, geological structure and topography cause result in monotonic displacement through time? Also groundwater (pore water pressures) should be considered here. Most likely the ground water table remains high enough for ongoing movement to continue.***

Response: Thank you for the careful reading and kind comment. To avoid the potential confusion, we have revised the text accordingly, with slight modification. Please see **2.1 Time series analysis of displacement: Pg 2_lines 41-46** in this revision.

The nonlinear evolution process of the cumulative displacement of landslides is controlled by primary factors such as geological conditions, and trigger factors such as rainfall and reservoir water level changes. The displacement of landslide sequence is an instability time series. Based on the time series analysis, total displacement of landslide can be broken down into different corresponding components according to the different influential factors. Total displacement of landslide can be divided into trend

component displacement, which is affected by the periodic dynamic functioning of inducing factors such as rainfall, reservoir water level, groundwater. Trend component displacement nearly increases under large time scales, and periodic component displacement fluctuated increases under small time scales. The trend component revealed the long-term trend of the sequence, which is determined by the potential energy and constraint condition of the slope.
Many landslides exhibit long-lasting, continuous movements under gravity loads that are affected by the creep property of slope materials (Desai et al. 1995). One of the important factors that influence the behavior of creeping slopes is appropriate characterization of the response of geologic materials and interfaces; in the case of creeping slopes, the latter can occur at the junction of the creeping mass and the essentially stationary (rock) mass below it. Landslide deformation is often characterized by creep, which generally need to undergo three stages, initial deformation, stable deformation and accelerated deformation stage. In the evolution scheme of three deformation phases of landslide, the landslide displacement generally increases monotonically with time.

Furthermore, we agreed with the reviewer that groundwater (pore water pressures) should be considered here. Groundwater, which is regarded as an active geologic agent, is one of the main factors that induces landslide instability. In the rising phase of reservoir water level, the groundwater level gradually increases, with a slight lag behind the increase in the reservoir water level. The groundwater remains high enough for ongoing movement to continue.

***5. Methodology: Pg 5_lines 3-16 – This section This section introduces the GA computational model but largely explains this through its previous biological applications. It would be much easier for the reader to explain how this has been adapted for landslide studies.***

Response: Thank you for the kind comment. We have revised the text accordingly.

We agreed with the reviewer that this section is not relevant to landslide, just introduces the GA model from the biological point of view. Thus, we deleted this section in the revision. About how this has been adapted for landslide studies, we had explained it in the introduction: Pg 2_lines 25-27.

***6. Case Study: Pg 6_ line 29 – Why is landslide monitoring considered a qualitative approach to analyse landslide development. This is quantitative data.***

Response: Thank you for the careful reading and kind comment. We have revised the text accordingly. Indeed, landslide monitoring is considered a qualitative approach to analyse landslide development. We are very sorry for that our mistake in spelling words. Please see **Case Study: Pg 6_line 28** in this revision.

***7. Case Study: Pg 8_lines 17-18 – This should be the other way around- the landslide stability decreased and the deformation increased.***

Response: Thank you for the careful reading and kind comment. We have revised the text accordingly. Please see **Case Study: Pg 8_line 20-21** in this revision.

***8. Case Study: Pg 9_line 24 – Statement 'materials in the sliding mass are degraded by excess moisture and additional hydrodynamic pressure' is not correct. The excess pore water pressure reduces the mean effective stress at the landslide shear surface making it more susceptible to movement.***

Response: Thank you for the careful reading and constructive comments. We agreed with the reviewer about the explanation. We have revised the text accordingly. Please see **Case Study: Pg 12_line 23-24** in this revision.

***9. Case Study: Pg 12_lines9-10 – How has the sliding force increased? Is it not the case that the confining pressure reduces with the lowering of the lake but the pore water pressure remains high so this change in stress state makes the slope more unstable?***

Response: Thank you for the constructive comments. We have revised the text accordingly. Please see **Case Study: Pg 12_line 5-13** in this revision for details.

Although the variation in reservoir water level was small before April 2007, the periodic displacement still exhibited small fluctuations due to the effects of rainfall and groundwater. This behavior could be explained in terms of stress changes within the landslide in that the rainfall events cause increased pore water pressures in the landslide shear zone which reduced the effective stress and increased instability. After April 2007, several distinct peaks can be observed in the periodic displacement-time curves during periods of decreasing reservoir water level. For example, the periodic displacement increased from May to July 2009 and from May to September 2012. However, when the reservoir water level increased from 145 m to 175 m, the periodic displacement gradually decreased. The main reason for the above conditions was that the rise of the reservoir water level increased the confining stress on the surface of the landslide and the hydrodynamic pressure, the direction of which was toward the interior of sliding body. Similarly, the lowering of the reservoir water level reduced the confining stress whilst pore water pressures were still high which would promote accelerated movement.

***10. Case Study: Pg 12_line 12 – Is this an actual piezometer or standpipe installation or is this water observed within the inclinometer tube itself? If the latter is there any certainty as to where this has come from? If not an installed piezometer it could have come from the top cap of the installation and therefore may not be a reliable groundwater measurement.***

Response: Thank you for the careful reading and constructive comments. We have revised the text accordingly. Please see **Case Study: Pg 12_line 15-16** in this revision for details.

The water gauge used in this landslide was 730 type water level sensor, with the characteristics of measuring range as deep as 210 meters, high measuring accurancy and stable performance. The data acquisition and memory used NetL G-301 data storage device. At the head scarp of the landslide at an elevation of 181m, groundwater depth was measured by water gauge within inclinometer monitoring hole QZK3.

***11. Figures Fig 4. The key is not explained. A clear key showing instrument type and borehole***

***locations is needed Fig 5. As with figure 4 the key is not clear. Also the borehole and inclinometers should be drawn on to show their depth. Fig 8. Diagrams are hard to read. It would be better to display these as conventional inclinometer plots with depth on the y axis and displacement on the x axis.***

Response: Thank you for the careful reading and constructive comments. We have revised the text accordingly.

About Fig. 4 and Fig.5, we have added the key for legend information accordingly. Please see **Fig.4 and Fig.5** in this revision for details.

Furthermore, Fig 8. Diagrams is displayed as conventional inclinometer plots with depth on the *y* axis and displacement on the *x* axis. Please see **Fig 8** in this revision.

## References

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Sun, M., Tang, H., Wang, M., Shan, Z., Hu X.: Creep behavior of slip zone soil of the Majiagou landslide in the Three Gorges area, Environ Earth Sci, 16, 1-12, 10.1007/s12665-016-6002-x, 2016.

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# Landslide displacement prediction using the GA-LSSVM model and time series analysis: a case study of Three Gorges Reservoir, China

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**Abstract** Predicting landslide displacement is challenging, but accurate predictions can prevent casualties and economic losses. Many factors can affect the deformation of a landslide, including the geological conditions, rainfall, and reservoir water level. Time series analysis was used to decompose the cumulative displacement of landslide into a trend component and a periodic component. Then the least squares support vector machine (LSSVM) model and genetic algorithm (GA) were used to predict landslide displacement, and we selected a representative landslide with episodic movement deformation as a case study. The trend component displacement, which is associated with the geological conditions, was predicted using a polynomial function, and the periodic component displacement which is associated with external environmental factors, was predicted using the GA-LSSVM model. Furthermore, based on a comparison of the results of the GA-LSSVM model and those of other models, the GA-LSSVM model was superior to other models in predicting landslide displacement, with the smallest root mean square error (*RMSE*) of 62.4146 mm, mean absolute error (*MAE*) of 53.0048 mm, and mean absolute percentage error (*MAPE*) of 1.492% at monitoring station ZG85, while these three values are 87.7215 mm, 74.0601 mm and 21 1.703% at ZG86 and 49.0485 mm, 48.5392 mm and 3.131% at ZG87. The results of the case study suggest that the model can provide good consistency between measured displacement and predicted displacement, and periodic displacement exhibited good agreement with trends in the major influencing factors.

**Keywords** landslide; displacement prediction; least squares support vector machine; genetic algorithm; reservoir water level; rainfall

## 1 Introduction

In the Three Gorges Reservoir region, landslides are the main type of geo-hazard, and they cause critical harm to individuals and property each year (Du et al. 2013; Yao et al. 2013; Lian et al. 2014; Cao et al. 2016). The displacement prediction of landslides is a major focus in the field of landslide research (Sassa et al. 2009; Du et al. 2013). Comprehensive analyses of landslide response and displacement predictions of landslide based on external factors are effective methods that rely on landslide deformation data. The evolution process of landslide is a complex non-linear process caused by the complex interaction of different factors. The accurate prediction of reservoir landslide processes is an important basis for early prevention, and it can reduce the loss of property and lives (Corominas et al. 2005). Therefore, geological surveying, monitoring, landslide prevention and landslide prediction must be improved to minimise the losses caused by landslides (Kirschbaum et al. 2010; Miyagi et al. 2011; Ahmed 2013). A landslide can be regarded as a nonlinear and dynamic system that is affected by external factors, such as rainfall, reservoir water levels, groundwater, etc. (Guzzetti et al. 2005; Kawabata and Bandibas 2009). Due to the influences of external factors, deformation displacement of landslide generally exhibits the same tendencies as the variations in external factors, which can result in misleading landslide prediction. Displacement time series is usually considered as a direct representation of complex nonlinear dynamical behavior of landslide.

In recently years, grey system models, time series models, neural network models, extreme learning machines, support vector machines (SVM), etc. have been widely used for landslide displacement prediction (Wang 2003; Pradhan et al. 2014; Gelisli et al. 2015; Goetz et al. 2015; Kavzoglu et al. 2015). Previously, landslide susceptibility maps were assessed using a back propagation artificial neural network and logistic regression analysis (Nefeslioglu et al. 2008). Additionally, dynamic time series predictors were proposed based on echo state networks (Yao et al. 2013). Lian et al. (2013) used an extreme learning machine and ensemble empirical mode decomposition to predict landslide displacement. Although these models were constructed based on

different algorithms, each has strengths and weaknesses. Grey system models are widely used in analyses of exponential time series. However, for complex nonlinear slope displacement series, prediction results can yield considerable error (Yin and Yu 2007; Sun et al. 2008). Additionally, autocorrelation coefficients, partial correlation coefficients and pattern recognition features are difficult to determine via time series analysis (Brockwell and Davis 2013; Turner et al. 2015). The neural network method is a powerful tool in landslide prediction (Liu et al. 2014; Lian et al. 2015). However, the conventional neural network has many limitations, including overfitting and a shortage of theoretical guidance in the selection of the number of network nodes in the hidden layer, which diminishes its prediction ability (Hwang et al. 2014). In addition, the neural network neglects practical issues by using a pre-defined activation function. Compared with traditional learning algorithms, although extreme learning machines are characterized by high generalization, good performance and fast computing speed, their output is different at different times due to the use of randomly selected input (Lian et al. 2014). Thus, it is difficult to reflect large quantities of information completely and predict landslide displacement accurately using these models because landslide displacement is actually a finite time series.

The SVM model can effectively overcome the limitations of other methods, including small sample sizes, high dimensionality and nonlinearity. Many studies have illustrated the ability of SVM models to recognize learning patterns, such as nonlinear regression, and obtain the global optimum solutions to these problems (Feng et al. 2004; Marjanović et al. 2011; Micheletti et al. 2011; Hong et al. 2016). Although these problems can be transformed into quadratic convex programming problems, the computation speed of the SVM model is slow when the training data set is large or the dimensionality is high (Zhang et al. 2009). To overcome these inadequacies, we use the least squares support vector machine (LSSVM) proposed by Suykens and Vandewalle (1999), which is a supervised learning model that has been widely applied in other machine learning problems, such as function fitting. The LSSVM model uses the square sum of the least square linear system error as the loss function and solves the problem by transforming it into a set of equations, which increases the solution speed and reduces the required calculation resources (Suykens et al. 2002; Lv et al. 2013; Xu and Chen 2013; Zhang et al. 2013). Additionally, this method yields good performance in pattern recognition and nonlinear function fitting. However, the selection of parameters is crucial to developing an efficient LSSVM model due to its sensitivity to small variations in the parameters.

The genetic algorithm (GA) is a global optimization algorithm that uses highly parallel, random and adaptive searching based on biological natural selection and optimization. Thus, the method is particularly suitable for solving complex and nonlinear problems (Li et al. 2010; Ali et al. 2013). In this paper, the GA is selected as the method of parameter optimization in the LSSVM due to its advantages in determining the unknown parameters that are consistent between the predicted data and the measured data. By introducing the GA, some key parameters of the LSSVM model can be derived automatically. Therefore, we select the combination of the LSSVM model and the GA to predict landslide displacement.

Due to the influences of rainfall, reservoir water level and human activities on the monitoring data of landslide displacement, most monitoring data series are incomplete or highly variable. These issues introduce uncertainty into the mathematical model and increase the difficulty of prediction. To overcome this and obtain the main error sources, a time series analysis of displacement is conducted by decomposing the monitoring data series into several components (Du et al. 2013). Then, the monitoring data series are simulated using the moving average method. Shuping landslide, a typical landslide with episodic movement deformation, was taken as an example to validate and the GA-LSSVM model with time series analysis.

## 2 Methodology

### 2.1 Time series analysis of displacement

Cumulative displacement of landslides is caused by the combined effects of internal geological conditions (lithology, geological structure, topography, etc.) and external environmental factors (rainfall, reservoir water level, groundwater, etc.). The displacement of landslide sequence is an instability time series. The landslide displacement caused by the former increases generally with time, which reflects the trend in cumulative displacement. Landslide deformation exhibit long-lasting and continuous movements under gravity loads that is affected by the creep characteristic (Desai et al. 1995). One of the important reasons that influence the creep behavior is the expression of the response of geological materials and interfaces. Landslide deformation is often characterized by creep, which generally need to undergo three stages, initial deformation, stable deformation and accelerated deformation stage. However, the landslide displacement induced by the latter is approximately periodic. Therefore, a landslide displacement sequence is an instability time series with a periodic episodic movement characteristic. According to time

series analysis, cumulative displacement can be decomposed into three portions as follows:

$$y_t = p_t + q_t + \varepsilon_t \qquad (1)$$

where $y_t$ is the cumulative displacement, $p_t$ is the trend component displacement, $q_t$ is the periodic component displacement, and $\varepsilon_t$ is the random component displacement.

However, it is difficult to obtain relevant data regarding the random component (wind loads, car loads, etc.) due to the lack of advanced monitoring methods. In this paper, the random component displacement is not considered. Therefore, we can simplify the time series model as follows.

$$y_t = p_t + q_t \qquad (2)$$

The trend component can be extracted using the moving average method as follows:

$$A_i = \{a_1, a_2, \cdots, a_j, \cdots, a_n\} \qquad (3)$$

$$\overline{p_t} = \frac{a_t + a_{t-1} + \cdots + a_{t-k-1}}{k} (t = k, k+1, \cdots, n) \qquad (4)$$

where $A_i$ is the time series of cumulative displacement of the *i*th monitoring system (*i*=1, 2,…, *m*), $a_j$ is the cumulative displacement of the *i*th monitoring system at time *j* (*j*=1, 2,…, *n*), $\overline{p_t}$ is the extracted value of the trend component, and *k* is the moving average period.

The periodic component displacement can be acquired by subtracting the trend component displacement from the cumulative displacement. Therefore, the time series model not only reflects the relationship between each component of cumulative displacement but also provides mathematical and physical meaning for landslide displacement prediction.

**2.2 LSSVM**

The LSSVM model is a regression prediction method with nonlinear characteristics based on a statistical learning theory, and it is regarded as an improved form of the SVM (Vapnik 1995; Abdi and Giveki 2013). First, after dividing the sample data into training samples and testing samples, the training samples are plotted in a high-dimension feature space via nonlinear mapping. Then, the optimal decision function model is obtained for the best-fitted results by training the sample data $\{x_j, y_j\}$, where *j*=1, 2, 3,…, *n*. The regression function of the LSSVM can be expressed as follows:

$$f(x) = W^T \varphi(x) + b \qquad (5)$$

where $W^T$ is the weight vector, $\varphi(x)$ is a nonlinear mapping function that maps the sample data into the feature space, *x* is the input, *y* is the output, and *b* is the offset.

By searching or a function *f*(*x*) that adjusts the dispersion degree of the training samples, we can obtain a risk-minimized solution. This solution can be written using the structural risk minimization principle:

$$\text{Minimize: } \frac{1}{2} W^T W + \frac{C}{2} \sum_{j=1}^{n} \xi_j^2 \qquad (6)$$

$$\text{Subject to: } y_j = W^T \varphi(x_j) + b + \xi_j (j = 1, 2, \cdots, n) \qquad (7)$$

where *C* is a penalty factor representing the penalty degree of the training samples, *b* is the offset, and $\xi_j$ is the relaxation factor.

Based on the Lagrange equation and duality theory, the optimization problem can be converted into a dual problem:

$$L(W,b,\xi,\alpha)=\frac{1}{2}W^TW+\frac{C}{2}\sum_{j=1}^{n}\xi_j^2-\sum_{j=1}^{n}\alpha_j(W^T\varphi(x_j)+b+\xi_j-y_j) \quad (8)$$

where $\alpha_j$ is the Lagrange multiplier.

The solution of the optimization equation is obtained by solving the partial differential form of the Lagrange equation with respect to $W$, $b$, $\xi_j$, $\alpha_j$. The optimization equations are expressed as follows.

$$\begin{cases} \dfrac{\partial L}{\partial W}=0\Rightarrow W=\sum_{j=1}^{n}\alpha_j y_j\varphi(x_j) \\ \dfrac{\partial L}{\partial b}=0\Rightarrow \sum_{j=1}^{n}\alpha_j y_j=0 \\ \dfrac{\partial L}{\partial \xi_j}=0\Rightarrow \alpha_j=C\xi_j \\ \dfrac{\partial L}{\partial \alpha_j}=0\Rightarrow y_j[W^T\varphi(x_j)+b]-1+\xi_j \end{cases} \quad (9)$$

The linear equations can be obtained by solving Eq. (9) with the elimination of $W$ and $\xi$:

$$\begin{bmatrix} 0 & I^T \\ I & ZZ^T+C^{-1}E \end{bmatrix}\begin{bmatrix} b \\ \alpha \end{bmatrix}=\begin{bmatrix} 0 \\ y \end{bmatrix} \quad (10)$$

where $y=[y_1,y_2,\cdots,y_l]^T$, $I=[1,\cdots 1]^T$, $\alpha=[\alpha_1,\alpha_2,\cdots,\alpha_l]^T$, $Z=[\varphi(x_1),\varphi(x_2),\cdots,\varphi(x_l)]^T$ and $E$ is the unit matrix with $l$ dimensions.

Then, the regression prediction model of the LSSVM can be rewritten based on the above optimization problem:

$$f(x)=\sum_{j=1}^{n}\alpha_j K(x,x_j)+b \quad (11)$$

where $K(x_j,x)$ is a kernel function.

In the paper, we select the radial basis kernel function as the kernel function in the LSSVM model to obtain the optimal solutions due to its strong nonlinear mapping ability and wide convergence domain (Min and Lee 2005; Altınel et al. 2015; Elbisy 2015; Farzan et al. 2015):

$$K(x_j,x)=\exp(-(x-x_j)^2)/(2\sigma) \quad (12)$$

where $\sigma$ is a parameter of the kernel function.

The parameter of the model $C$ and the parameter of the kernel function $\sigma$ significantly influence the prediction performance. The parameter $C$ represents the error tolerance. The more accurate the parameter is, the higher the prediction performance is, but this can lead to overtraining. The parameter $\sigma$ implicitly determines the spatial distribution of data mapping in the new feature space. Therefore, some measures should be taken to optimize the LSSVM parameters.

**2.3 GA**

Currently, several intelligent algorithms are used to solve optimization problem, such as the GA (Li et al. 2010; Ali et al. 2013), grid algorithm (Lin 2001), particle swarm optimization (Vandenbergh and Engelbercht 2006) and genetic programming (Garg and Tai 2011; Shen et al. 2012). However, compared with the GA, the grid algorithm is tedious and cannot yield satisfactory

results (Gu et al. 2011). For discrete optimization problems, particle swarm optimization performs poorly and often yields local optima (Fei et al. 2009). In addition, genetic programming, which was developed by Koza (1992), provides solutions to complex problems using evolutionary algorithms, and the method is typically expressed as a tree structure that consists of terminals and functions; however, it is difficult to generate new individuals, which seriously affects the convergence rate (Garg et al. 2014). In this paper, we select the GA to determine the best parameters ($C$ and $\sigma$) of the LSSVM for predicting landslide displacement.

The GA is a computational model commonly used to simulate natural selection and the biological evolution processes of genetic mechanisms. The GA provides solutions for complicated problems using evolutionary algorithms (Levasseur et al. 2008; Hejazi et al. 2013). The typical genetic operations include selection, crossover and mutation.

Based on certain methods and theories, selection operations, such as the fitness-ratio selection algorithm, ranking algorithm, Monte Carlo selection and tournament selection, are commonly used to choose a parental generation from a population based on an individual's fitness value. Crossover operation can generate two new offspring by selecting random codes from two parents and then exchanging their respective branches. Point mutation is commonly used as the mutation operator. By selecting a random node from a parent, a new individual is generated by substituting the selected random node into another parent branch. A typical genetic algorithm is shown in Fig. 1. Selection operations, crossover operations and mutation operations are probabilistic, and with a probability of over 90%, crossover operations are the most widely used.

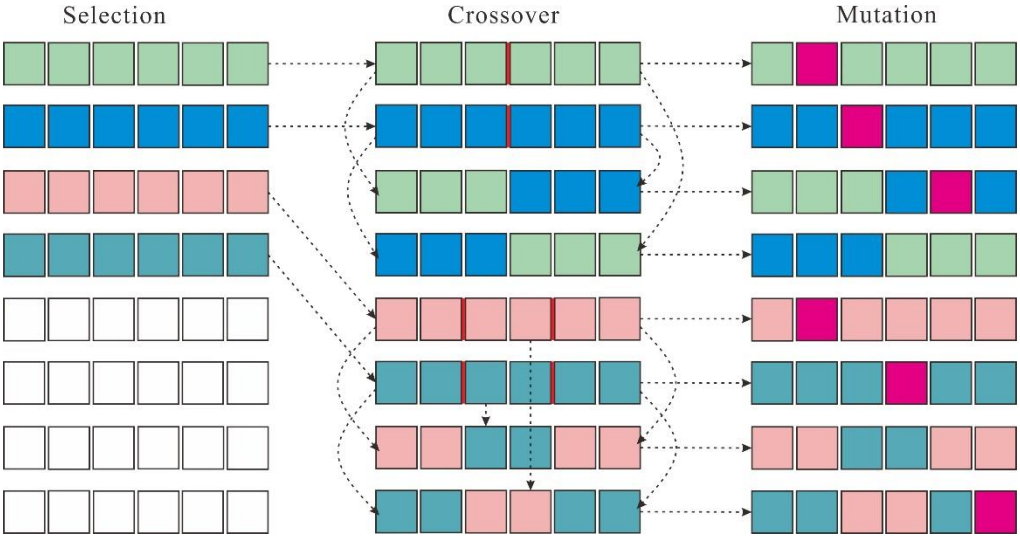



Fig. 1 Diagram of genetic operations

**2.4 GA-LSSVM model**

To obtain the best model, the parameters of the model must be carefully selected in advance (Duan et al. 2003). According to some research results (Lessmann et al. 2005; Pourbasheer et al. 2009), the GA has the advantages of reducing the blindness of artificial selection and enhancing the discrimination ability of the LSSVM model. Modeling with this method can achieve high precision if the training samples are reliable. The sampling data used for landslide displacement prediction are continuous and mutually dependent landslide data applicable for a specific method; thus, the data are essentially independent sampling data. In this paper, the periodic component displacement is predicted by the GA-LSSVM model, which has higher accuracy than other models due to the consideration of the trigger factors. MATLAB software is used to execute the model. The flowchart of the GA-LSSVM model is presented in Fig. 2.

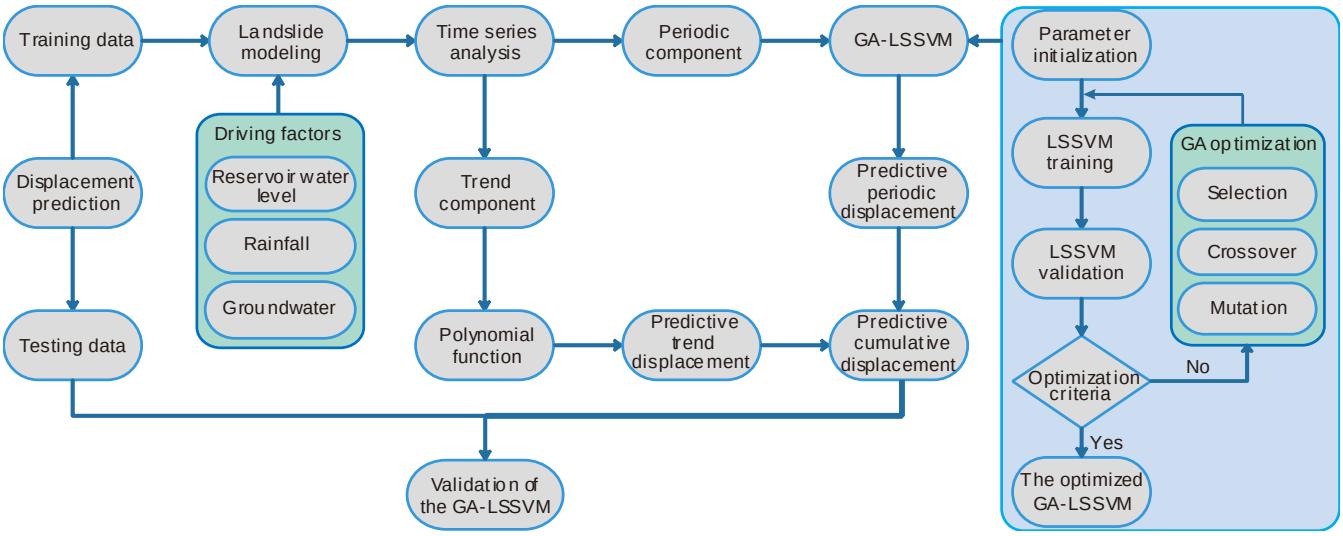



Fig. 2 The basic flowchart of the GA-LSSVM model, including the establishment of the GA-LSSVM model and the validation of the model

## 3 Case study: Shuping landslide

### 3.1 Geological conditions

The Shuping landslide is located in Shazhenxi town, Zigui country, Hubei province, China, near the Yangtze River and approximately 47 km into the upper reach of Three Gorges Dam (Fig. 3). The sliding direction of Shuping landslide is N11°E, and the landslide presents a sector in a topographic map (Fig. 3). The reservoir water level in Fig. 3 is 166 m. The topography is relatively flat, with a mean slope angle of 22°. The highest elevation of the landslide is 400 m above sea level. The head scarp of the landslide reaches to the riverbed of the Yangtze River at 60 m in elevation. The landslide covers an area of approximately $54\times10^4$ $m^2$, with an average length of 800 m in the longitudinal direction and an average length of 670 m in the transverse direction. The landslide volume is $2070\times10^4$ $m^3$, with an average sliding surface depth of 40 m (Fig. 4). Fig. 4 shows the eight GPS monitoring stations installed on the ground surface of the landslide, as well as four inclinometers monitoring holes. The bedrock is mainly sandy mudstone. The strata comprise the Triassic Badong formation. The dip direction of the bedrock is between 120° and 165°, and the dip angle is between 10° and 35°. The landslide is divided into an eastern portion and a western portion, and the materials of the landslide mainly include Quaternary deposits and soils containing silty clay and rock fragments with a loose and disorderly structure (Fig. 5). Fig. 5 shows a longitudinal section of the eastern portion. The sliding surface is steep in the upper area, which is located between the deposits and the bedrock.

Underground moisture beneath the landslide is primarily ground water flowing through a loose medium that consists of colluviums, deposits, etc. After the water storage began in Three Gorges Reservoir in June 2003, the landslide deformation became more active. Various external factors affect the landslide displacement, including rainfall, the reservoir water level, surface water infiltration, groundwater, etc.

### 3.2 Monitoring data and deformation characteristics of the landslide

Field investigations revealed that there was no obvious deformation of this landslide before the first impoundment of the reservoir on June 15, 2003. However, cracking occurred through roads and houses, after the first impoundment. To measure the deformation characteristics and stability of the landslide, monitoring stations were installed to observe the interactions between different portions of the landslide. The monitoring methods include geodetic surveys, drilling, meteorological observations and geological investigations. Thus, the development processes and evolution of the landslide can be analyzed quantitatively using monitoring data from eight monitoring stations and four inclinometer monitoring holes located along the longitudinal direction of the landslide (ZG85 to ZG90, SP-2 and SP-6, QZK1 to QZK4 in Fig. 4).

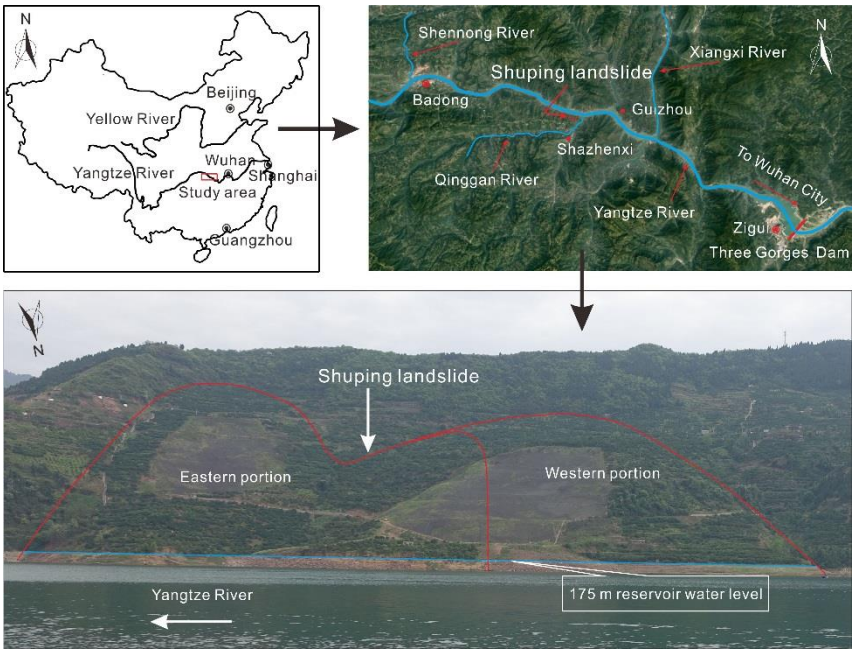



Fig. 3 Location of the study area and panorama of the Shuping landslide and landslide subzones

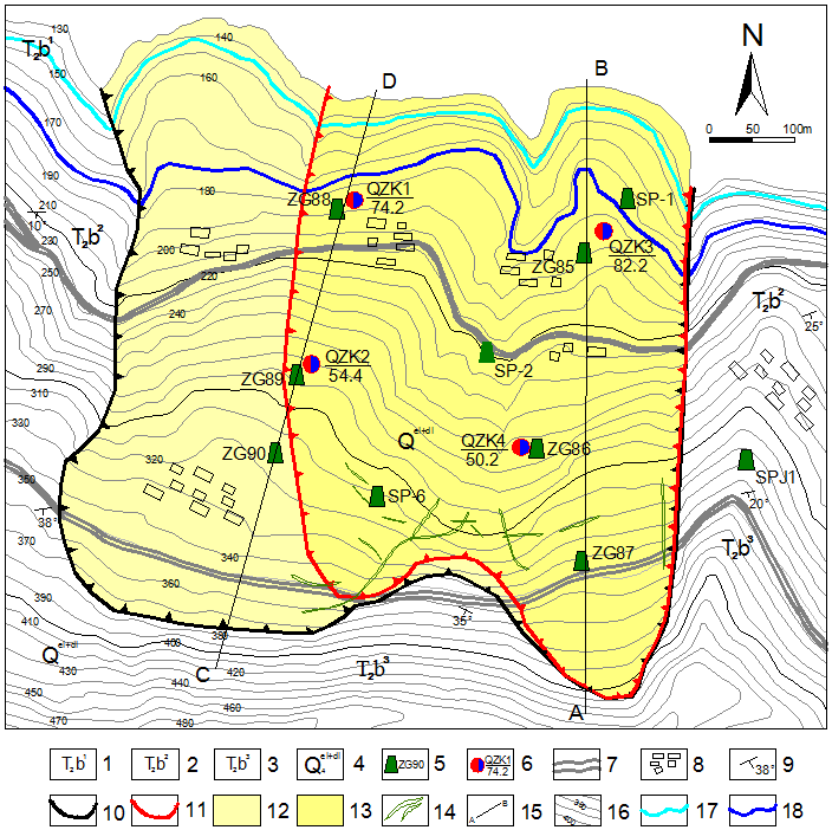



Fig. 4 Geology and deformation monitoring map of Shuping landslide. 1 Middle Triassic Badong Formation Section 1, 2 Middle Triassic Badong Formation Section 2, 3 Middle Triassic Badong Formation Section 3, 4 Quaternary colluviums, 5 GPS monitoring stations and number, 6 inclinometer monitoring hole and its depth (the unit of depth is the meter), 7 roads, 8 houses, 9 lithology orientation, 10 landslide boundary, 11 main sliding boundary, 12 western portion zone, 13 eastern portion zone, 14 cracks, 15 longitudinal section, 16 counter line, 17 reservoir water level (145 m), and 18 reservoir water level (175 m)

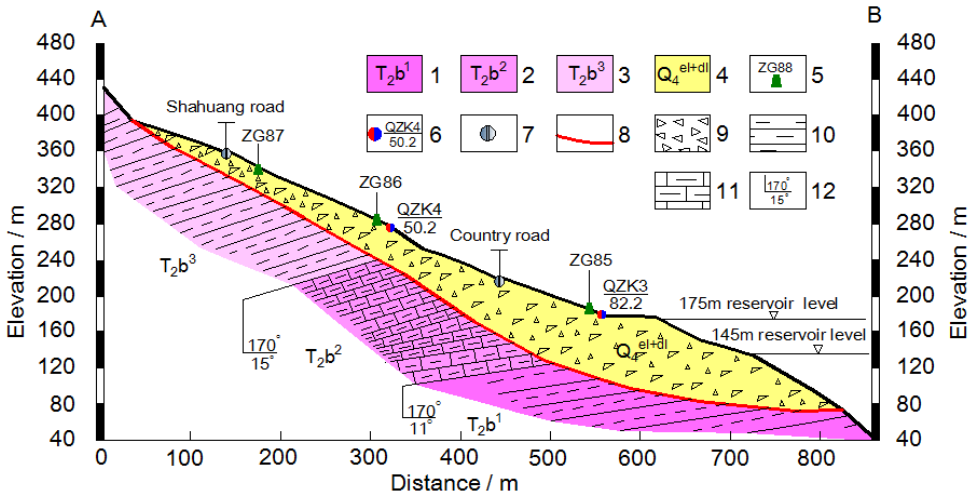



Fig. 5 Geological longitudinal section (line A-B in Fig. 4) of Shuping landslide. 1 Middle Triassic Badong Formation Section 1, 2 Middle Triassic Badong Formation Section 2, 3 Middle Triassic Badong Formation Section 3, 4 Quaternary colluviums, 5 GPS monitoring stations and number, 6 inclinometer monitoring hole and its depth (the unit of depth is the meter), 7 roads, 8 sliding zone, 9 colluvial gravel soil, 10 silty mudstone, 11 argillaceous limestone, and 12 lithology orientation

Fig. 6 shows the monitoring results between July 2003 and October 2013, including rainfall and reservoir water level, which exhibit near episodic movement characteristics after the first impoundment. The displacements in the middle (ZG86) and head scarp (ZG85) areas were greater than that in the back scarp (ZG87) area of longitudinal section A-B, and the displacements in the head scarp (ZG88) and middle (ZG89) areas were greater than that in the back scarp (ZG90) area in the western zone. These observations suggest that landslide displacements increased steadily, and Shuping landslide displayed retrograde style deformation from the lower part to the upper part. The cumulative displacements at the monitoring stations located in the frontal areas were relatively low, with an average value of 880 mm, and the cumulative displacements at the monitoring stations located in the middle-rear areas were very high, with an average value of 3890 mm. Overall, landslide deformation in the eastern zone was greater than that in the western zone. Based on the reservoir water level data and the displacements measured at eight monitoring stations, the cumulative displacement rate increased after the initial impoundment. Due to the increased rainfall and decreased reservoir water level between April and August each year, the cumulative displacement rises rapidly. Notable landslide accelerations can be observed in 2007, 2009, 2011 and 2012. The variations in reservoir water level and heavy rainfall increase pore water pressure and reduce the effective stress in the slope. In addition, the uplift pressure, hydrostatic pressure and hydrodynamic pressure acting on the landslide changed periodically. As a result, the landslide stability decreased and the deformation increased.

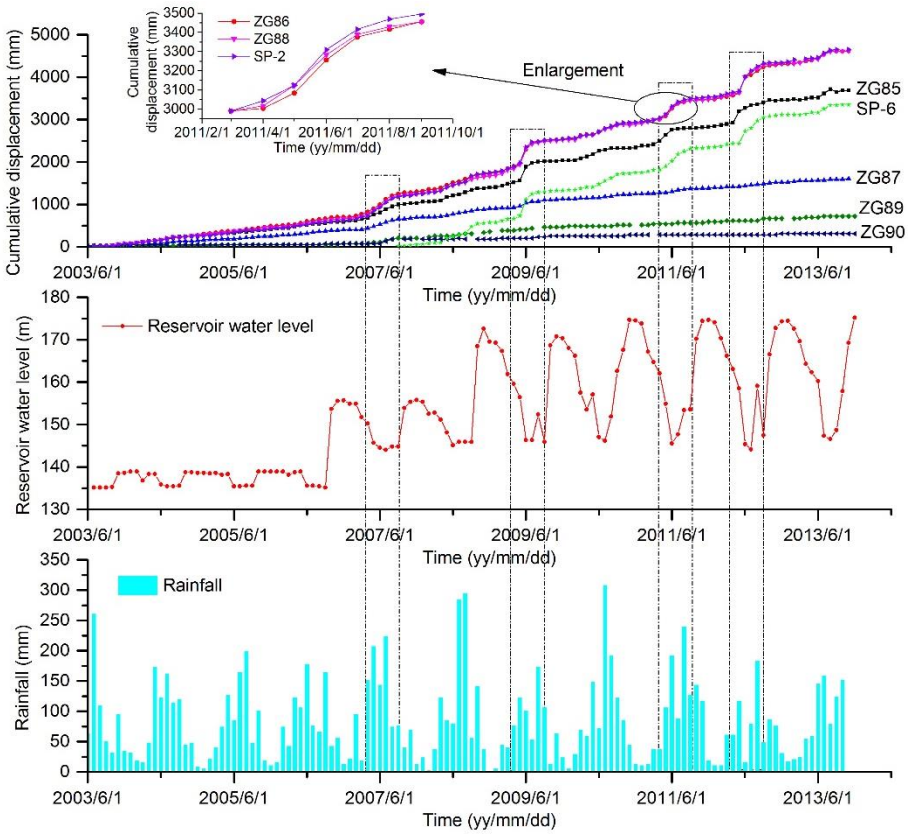



Fig. 6 The relationships between rainfall, reservoir water level and displacement

Many deformation or failure phenomena were observed in the Shuping landslide. In June 2003, a crack was generated in the middle part of the landslide on the outside of a local road, as shown in Fig. 7(a). In 2006, the reservoir water level increased to 156 m for the first time. Fig. 7(b) illustrates that the crack gradually extended to a width of 10 cm within 3 months of completing the road in April 2007. In August 2008, after a heavy storm occurred, deformation and tension cracks developed in the eastern portion of the landslide and impacted houses, as shown in Fig. 7(c). Since 2008, the reservoir water level has increased gradually to 172 m in October. In June 2009, the western portion of the landslide started cracking, with a maximum crack width of 20 cm and depth of 20-50 cm. In addition, several tension cracks formed at the eastern landslide boundary. The tension cracks in the eastern portion are shown in Fig. 7(d). In recent years, the cumulative deformation rate has remained low due to the relatively stable reservoir water level, which has fluctuated between 145 m and 175 m.

Therefore, the landslide deformation characteristics suggest that deformation in the western portion of the landslide is smaller than that in the eastern portion, and the Shuping landslide is affected by reservoir water level fluctuations and rainfall. When rainfall increases abruptly and the reservoir water level drops between April and August annually, the landslide becomes active, which increases landslide deformation. In other conditions, the landslide undergoes slow deformation at a constant speed.

In addition, groundwater, which is regarded as an active geologic agent, is one of the main factors that induces landslide instability. In the rising phase of reservoir water level, the groundwater level gradually increases, with a slight lag behind the increase in the reservoir water level. The groundwater remains high enough for ongoing movement to continue. Conversely, the groundwater level decreases in the declining phase of the reservoir water level. Moreover, the uplift pressure and seepage force of groundwater are dynamic processes that affect landslide stability. Therefore, groundwater influences displacement.

Overall, the reservoir water level, rainfall and groundwater are the major factors that influence the displacement of the Shuping landslide. The landslide displacement obviously increases when the reservoir water level decreases or when rainfall is heavy and continuous because the excess pore water pressure reduces the mean effective stress at the landslide shear surface making it more susceptible to movement.

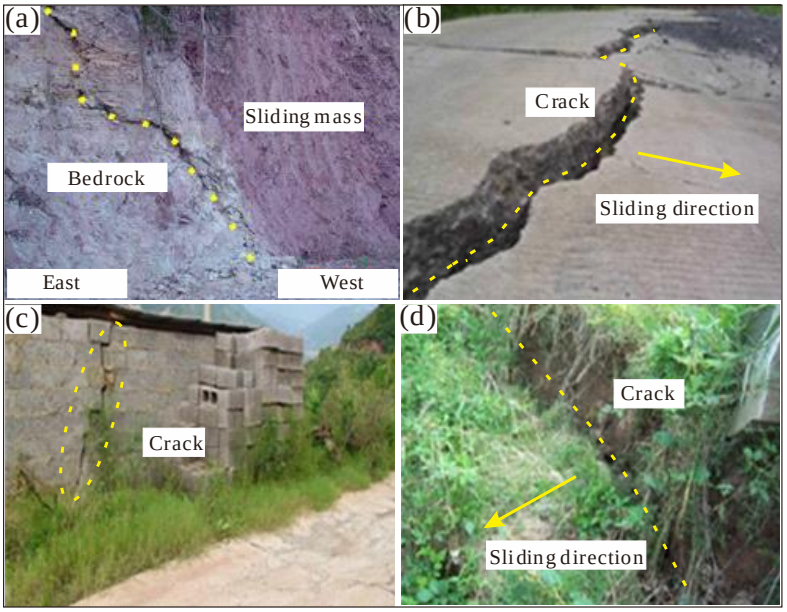



Fig. 7 Photographs of the ground cracks in the landslide (Ren et al., 2015): (a) crack in the middle of the landslide on the outside of the local road, (b) failure state of the local road, (c) wall cracking and subsidence in the eastern portion, and (d) the tension cracks in the eastern portion

During the period between June 2003 to June 2009, monitoring data show that the landslide deformation differences are manifested in the ground surface, and they display vertically distributed characteristics with elevation. In conclusion, the surface displacements below 200 m in elevation are larger than those above 200 m, and deformation is largest close to 175 m, which is the upper limit of the reservoir water level. This observation is due to the considerable influence of fluctuations in the reservoir water level on the landslide area below 200 m. The deep deformation of the landslide exhibited distinct differences at different depths, as shown in Fig. 8. Inclinometer monitoring holes QZK3 and QZK4, QZK1 and QZK2, which are located in the western portion of the landslide, exhibited small deformation and similar deformation trends. Thus, their lateral displacement curves are not presented, and only the curves of QZK3 and QZK4 are illustrated in this paper. The figures show that the sliding zones of QZK3 and QZK4 are located at elevations of 70 m and 30 m, respectively. Furthermore, the displacement change in the shallow sliding zones of both QZK3 and QZK4 is larger than that in the deep sliding zone.

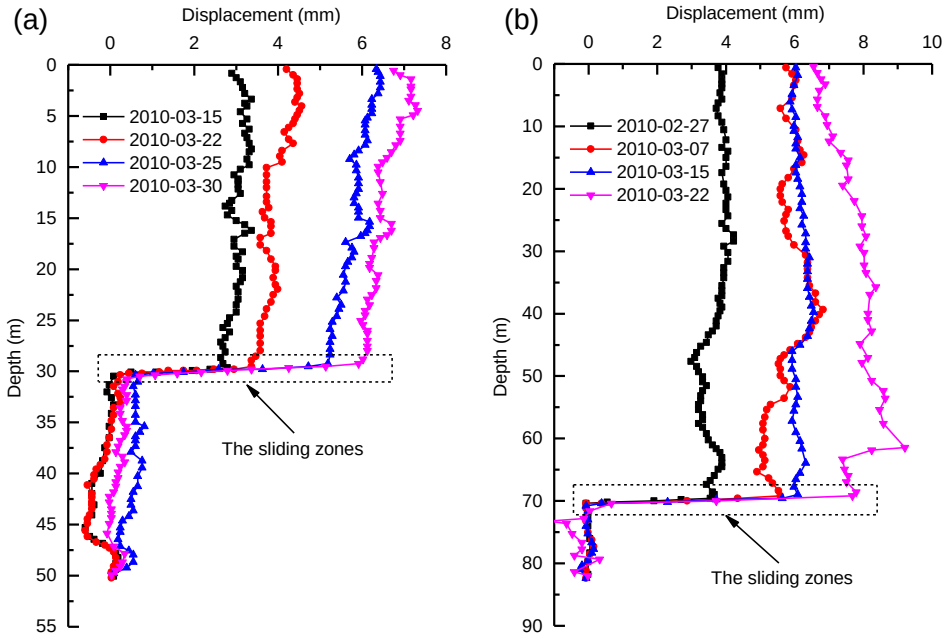



Fig. 8 Lateral displacements of Shuping landslide: (a) inclinometer monitoring hole QZK3 and (b) inclinometer monitoring hole

QZK4

## 4 Landslide displacement prediction

Based on the analysis of the deformation characteristics of Shuping landslide and the GA-LSSVM model above and due to the obvious nonlinear and episodic movement deformation characteristics of monitoring stations ZG85, ZG86 and ZG87, we select only these stations along longitudinal section A-B to verify and establish the prediction model. The model includes information regarding rainfall, the reservoir water level, human activities and the long-term behavior of Shuping landslide. Because the integrity of the data collected at monitoring points has an effect on the displacement prediction, the monitoring data from July 2003 to October 2013 are selected to explore landslide deformation. The data before October 2012 are used to train the GA-LSSVM model, and the data after October 2012 are used to test the model.

### 4.1 Prediction of the trend component displacement

Due to the scheduling period of the reservoir and the rainfall cycle, we choose 12 months as the moving average period. Because the curves of the trend component displacement versus time have quasi-linear and incremental characteristics, we use polynomial functions to fit these curves and provide the best-fitted results. The predicted and measured results of the trend component displacement at monitoring stations ZG85, ZG86 and ZG87 are shown in Figs. 9(a), 9(b) and 9(c), respectively. They indicate that the polynomial function provides good prediction performance for the trend component displacement and the fitted functions are expressed in Eqs. (13), (14) and (15).

$$p_t = -0.0015t^3 + 0.4744t^2 - 8.4975t + 128.83 \quad R^2=0.9980 \qquad (13)$$

$$p_t = -0.002t^3 + 0.604t^2 - 10.468t + 143.35 \quad R^2=0.9978 \qquad (14)$$

$$p_t = -0.0015t^3 + 0.3088t^2 - 2.7227t + 29.832 \quad R^2=0.9976 \qquad (15)$$

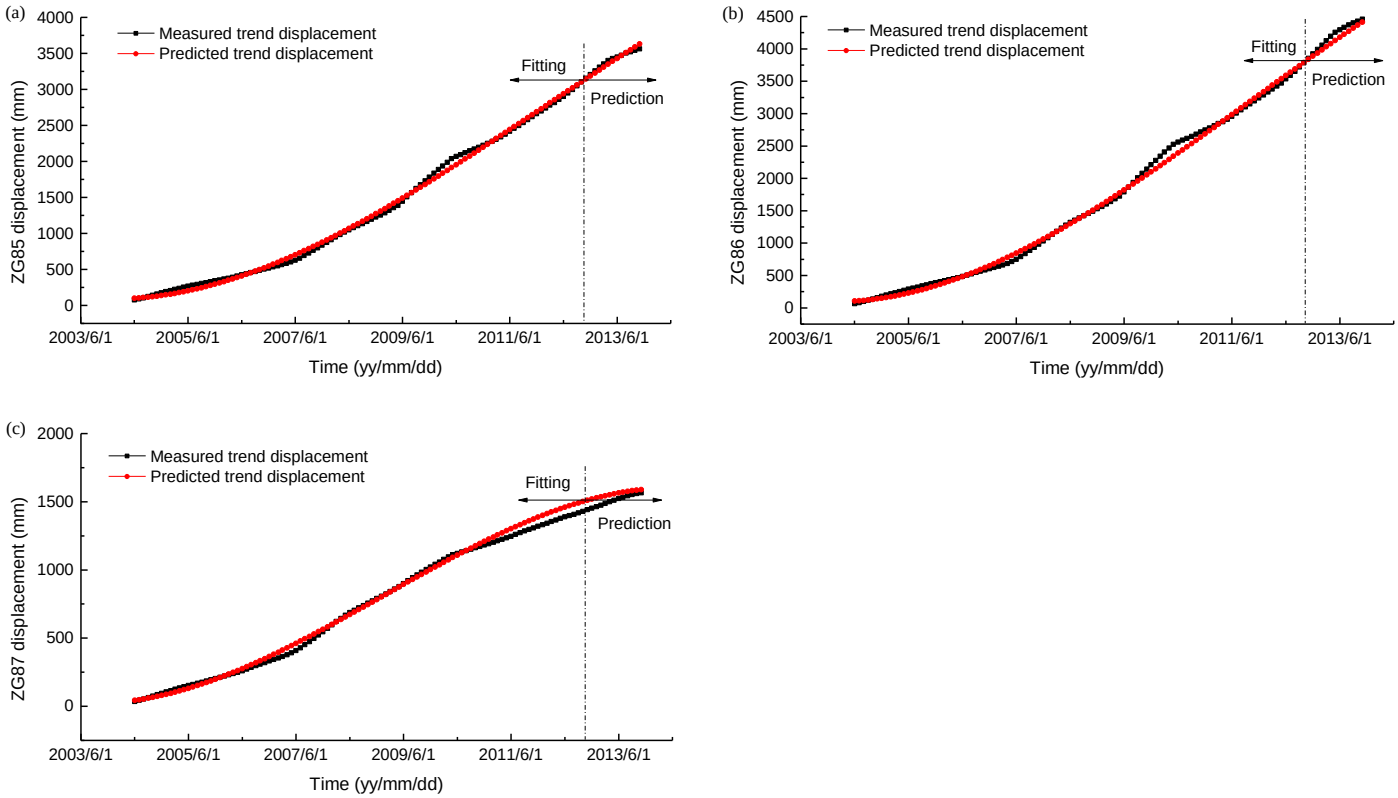



Fig. 9 Measured and predicted trend component displacement of Shuping landslide

### 4.2 The predicted periodic component displacement

The periodic component displacement is determined by subtracting the extracted trend component displacement from the cumulative displacement. The periodic displacement and the major influencing factors are illustrated in Figs. 10 and 11. The

variations in the periodic displacement are consistent with those in the influencing factors. The reservoir water level, rainfall and groundwater significantly influence the periodic displacement. For example, large periodic displacement can be observed in July 2009 and September 2012 when the landslide was affected by heavy rainfall and large variations in reservoir water level. Although the variation in reservoir water level was small before April 2007, the periodic displacement still exhibited small fluctuations due to the effects of rainfall and groundwater. This behavior could be explained in terms of stress changes within the landslide in that the rainfall events cause increased pore water pressures in the landslide shear zone which reduced the effective stress and increased instability. After April 2007, several distinct peaks can be observed in the periodic displacement-time curves during periods of decreasing reservoir water level. For example, the periodic displacement increased from May to July 2009 and from May to September 2012. However, when the reservoir water level increased from 145 m to 175 m, the periodic displacement gradually decreased. The main reason for the above conditions was that the rise of the reservoir water level increased the confining stress on the surface of the landslide and the hydrodynamic pressure, the direction of which was toward the interior of sliding body. Similarly, the lowering of the reservoir water level reduced the confining stress whilst pore water pressures were still high which would promote accelerated movement. The periodicity of the rainfall also affected the displacement rate. The periodic displacement increased with increasing rainfall and reached a peak value in summer, which reflects a certain lag.

At the head scarp of the landslide at an elevation of 181m, groundwater depth was measured by water level sensor within inclinometer monitoring hole QZK3. The change in groundwater depth exhibits considerable agreement with rainfall and reservoir water level fluctuations, with a slight lag observed for the latter. Due to the slight lag with the reservoir water level, groundwater increased the hydrodynamic pressure during periods when the reservoir water level decreased or remained stable, which resulted in continuous deformation of the landslide. Therefore, in the shallow groundwater zone, the periodic displacements measured at the three monitoring stations exhibited considerable fluctuations. In conclusion, the results in Figs. 10 and 11 indicate that the reservoir water level exerts the most influences on the displacement rate.

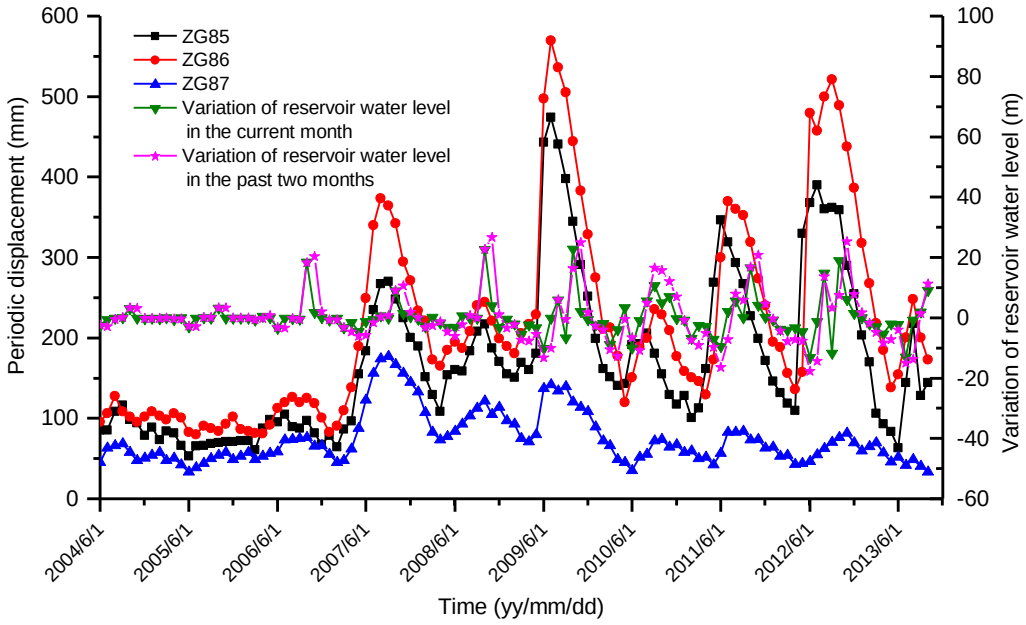



Fig. 10 The relationship between reservoir water level and the periodic displacement at GPS monitoring stations

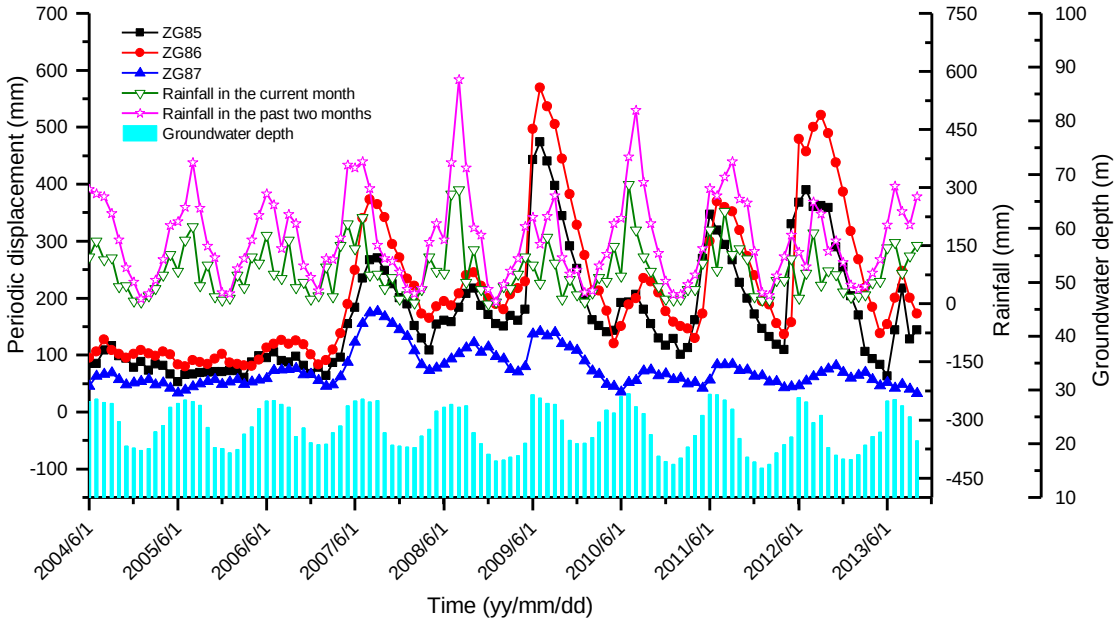



Fig. 11 The relationships between rainfall, groundwater depth and periodic displacement at GPS monitoring stations

The grey relational grade can represent the proximity degree between two series. If the trends in the two series are consistent or the degree of synchronous change is high, then the relational grade associated with system development is large. Otherwise, the relational grade is small. To remove the influence of dimensional data, data series must be normalized before calculating the relational grades, including the series of periodic displacement, rainfall and reservoir water level changes. The normalized formula can be expressed by Eq. (16):

$$\overline{y} = \frac{y - y_{\min}}{y_{\max} - y_{\min}} \qquad (16)$$

where $\overline{y}$ is the normalized value, $y$ is the original value, $y_{\max}$ is the maximum value of the data series, and $y_{\min}$ is the minimum value of the data series.

The grey relational coefficient of each data series and reference data series at each moment can be calculated as the following:

$$\gamma(y_0(k), y_i(k)) = \frac{\Delta\min + \rho\Delta\max}{\Delta_{oj}(k) + \rho\Delta\max} \qquad (17)$$

where $j$=1,2…$n$; $k$=1,2…$m$, $n$ is the number of data series items and $m$ is the number of parameters, $y_0(k)$ is the reference data series, $y_j(k)$ is the series after data preprocessing, $\Delta_{oj}(k) = \left\| y_0(k) - y_j(k) \right\|$ is the absolute value of the difference between $y_0(k)$ and $y_j(k)$, $\Delta\min = \min_{\forall j \in i} \min_{\forall k} \left\| y_0(k) - y_j(k) \right\|$ is the smallest value of $y_j(k)$, $\Delta\max = \max_{\forall j \in i} \max_{\forall k} \left\| y_0(k) - y_j(k) \right\|$ is the largest value of $y_j(k)$, $\rho$ is the distinguishing coefficient, $\rho \in [0,1]$. The smaller a value of $\rho$ is, the larger the distinguished ability is. $\rho = 0.5$ is generally used in the paper.

Then the average value of the grey relational coefficients is regarded as the grey relational grade (Tosun 2006). Thus, the grey relational grade is generated as follows:

$$\overline{\gamma_j} = \frac{1}{k}\sum_{i=1}^{m} \gamma_{ij} \qquad (18)$$

where $\overline{\gamma_j}$ is the grey relational grade for the *j*th data series.

Based on the grey relational analysis method, the relational grades between the influencing factors and the periodic displacements are shown in Table 1. We can use the large grey relational grades as the input variables in the GA-LSSVM model. When the relational grade is larger than 0.6, the influencing factor is closely correlated with the periodic displacement, which suggests that the selection of the influencing factor for predicting periodic displacement is reasonable (Wang 2003; Wang et al. 2004). Therefore, considering the characteristics of the periodic displacement and the relational grades between variables, the cumulative rainfall in the current month, the cumulative rainfall in the past two months, the reservoir water level, the variation in the reservoir water level in the current month, the variation in the reservoir water level in the past two months, and groundwater depth are selected as input variables. Moreover, the periodic component displacement is established as the output variable for use in the GA-LSSVM model.

Table 1 Relational grades between input variables and the periodic displacements

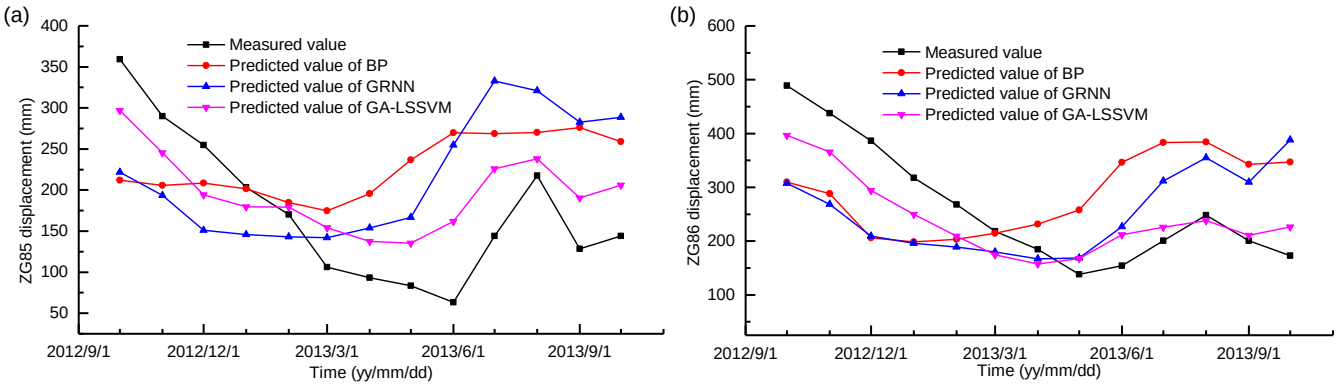
| Monitoring station | Relational grade | | | | | |
|---|---|---|---|---|---|---|
| | The cumulative rainfall in the current month | The cumulative rainfall in the past two months | The reservoir water level | The variation of The reservoir water level in the current month | The variation of The reservoir water level in the past two months | Groundwater depth |
| ZG85 | 0.700 | 0.705 | 0.763 | 0.797 | 0.768 | 0.718 |
| ZG86 | 0.682 | 0.691 | 0.756 | 0.794 | 0.770 | 0.714 |
| ZG87 | 0.692 | 0.705 | 0.724 | 0.794 | 0.780 | 0.720 |

The parameters of the LSSVM are optimized by the GA, including the best values of *C* and $\sigma$. Table 2 shows the optimal parameters of the LSSVM. The maximum generation threshold of the GA is 200, and the population number is 20. To validate the prediction ability of the GA-LSSVM model, we compare the results of generalized regression neural network (GRNN) and back propagation (BP) with two hidden layers with the result of the GA-LSSVM model. In this paper, the smoothing factor of the GRNN is 0.48, and there are 10 nodes in one of the hidden layers and 11 nodes in the other hidden layer of the BP.

Table 2 Optimal parameters of the LSSVM model

| Number | Monitoring station | *C* | $\sigma$ |
|---|---|---|---|
| 1 | ZG85 | 11.8234 | 6.4122 |
| 2 | ZG86 | 4.7346 | 8.0545 |
| 3 | ZG87 | 39.7819 | 5.7981 |

The prediction results of the periodic component displacement are shown in Fig. 12. The predicted values of the three prediction models and the measured values are consistent and illustrate similar trends. However, the predicted values obtained using the GA-LSSVM exhibit better agreement with observations than the other methods. Notably, the advantages of the model are clear from April 2013 to October 2013, as the periodic component displacement exhibited good agreement with the major influencing factors during a period of heavy rainfall and large fluctuations in the reservoir water level.



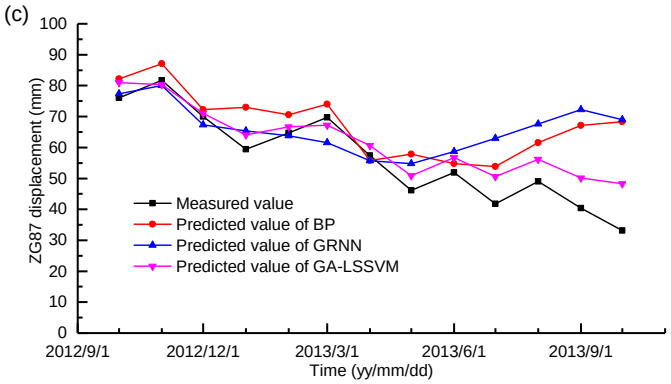



Fig. 12 Measured displacement and predicted periodic displacement of Shuping landslide

**4.3 Predicted cumulative displacement**

The predicted cumulative displacement is determined from the sum of the predicted trend displacement and the predicted periodic displacement. The predicted cumulative displacements and the measured values are presented in Table 3, Table 4 and Table 5 for monitoring station ZG85, ZG86 and ZG87, respectively. The results given in Table 3, Table 4 and Table 5 suggest that the GA-LSSVM model has better prediction performance than the GRNN model and the BP model, with a smaller relative error. Comparisons between the predicted values of cumulative displacement and measured values are shown in Fig. 13. The diagonal line shows the best prediction result in Fig. 13. The results are underestimated if the predicted values are located below the diagonal line, whereas the predicted values located above the line are overestimated. The predicted values from all the monitoring stations show good consistency with the measured values, as shown in Fig. 13.

Table 3 Comparison between the predicted values of cumulative displacement and measured values at monitoring station ZG85

| Time | Measured value (mm) | GA-LSSVM | | GRNN | | BP | |
|---|---|---|---|---|---|---|---|
| | | Predicted value (mm) | Relative error (%) | Predicted value (mm) | Relative error (%) | Predicted value (mm) | Relative error (%) |
| 2012/10/1 | 3460.208 | 3399.937 | 1.74 | 3324.829 | 3.91 | 3315.157 | 4.38 |
| 2012/11/1 | 3442.907 | 3389.608 | 1.55 | 3337.861 | 3.05 | 3349.827 | 2.78 |
| 2012/12/1 | 3460.208 | 3379.418 | 2.33 | 3336.503 | 3.58 | 3393.732 | 1.96 |
| 2013/1/1 | 3460.208 | 3406.014 | 1.57 | 3371.989 | 2.55 | 3427.727 | 0.95 |
| 2013/2/1 | 3477.509 | 3446.374 | 0.90 | 3410.133 | 1.94 | 3452.011 | 0.74 |
| 2013/3/1 | 3460.208 | 3462.169 | 0.06 | 3449.721 | 0.30 | 3482.668 | 0.64 |
| 2013/4/1 | 3494.81 | 3485.798 | 0.26 | 3502.356 | 0.22 | 3543.963 | 1.39 |
| 2013/5/1 | 3512.111 | 3524.423 | 0.35 | 3555.754 | 1.24 | 3625.738 | 3.13 |
| 2013/6/1 | 3512.111 | 3591.262 | 2.25 | 3684.274 | 4.90 | 3699.022 | 5.05 |
| 2013/7/1 | 3615.917 | 3695.444 | 2.20 | 3802.473 | 5.16 | 3738.225 | 3.27 |
| 2013/8/1 | 3719.723 | 3747.513 | 0.75 | 3830.496 | 2.98 | 3779.618 | 1.58 |
| 2013/9/1 | 3650.519 | 3740.002 | 2.45 | 3832.151 | 4.98 | 3825.664 | 4.58 |
| 2013/10/1 | 3685.121 | 3795.259 | 2.99 | 3877.587 | 5.22 | 3848.299 | 4.24 |

Table 4 Comparison between the predicted values of cumulative displacement and measured values at monitoring station ZG86

| Time | Measured value (mm) | GA-LSSVM | | GRNN | | BP | |
|---|---|---|---|---|---|---|---|
| | | Predicted value (mm) | Relative error (%) | Predicted value (mm) | Relative error (%) | Predicted value (mm) | Relative error (%) |
| 2012/10/1 | 4273.356 | 4183.984 | 2.09 | 4094.396 | 4.19 | 4096.849 | 4.13 |
| 2012/11/1 | 4290.657 | 4201.857 | 2.07 | 4104.93 | 4.33 | 4124.839 | 3.86 |
| 2012/12/1 | 4307.958 | 4149.796 | 3.67 | 4094.607 | 4.95 | 4091.602 | 5.02 |
| 2013/1/1 | 4307.958 | 4164.444 | 3.33 | 4130.77 | 4.11 | 4133.425 | 4.05 |
| 2013/2/1 | 4325.26 | 4192.775 | 3.06 | 4172.182 | 3.54 | 4186.816 | 3.20 |
| 2013/3/1 | 4342.561 | 4256.46 | 1.98 | 4212.082 | 3.00 | 4246.771 | 2.21 |
| 2013/4/1 | 4377.163 | 4317.892 | 1.35 | 4247.617 | 2.96 | 4312.109 | 1.49 |
| 2013/5/1 | 4394.464 | 4326.232 | 1.55 | 4297.626 | 2.20 | 4386.529 | 0.18 |
| 2013/6/1 | 4446.367 | 4388.693 | 1.30 | 4403.464 | 0.96 | 4523.094 | 1.73 |
| 2013/7/1 | 4532.872 | 4495.404 | 0.83 | 4535.948 | 0.07 | 4607.573 | 1.65 |
| 2013/8/1 | 4619.377 | 4609.902 | 0.21 | 4626.647 | 0.16 | 4656.543 | 0.80 |
| 2013/9/1 | 4602.076 | 4579.721 | 0.49 | 4628.676 | 0.58 | 4661.733 | 1.30 |
| 2013/10/1 | 4602.076 | 4592.204 | 0.21 | 4754.15 | 3.30 | 4713.128 | 2.41 |

Table 5 Comparison between the predicted values of cumulative displacement and measured values at monitoring station ZG87

| Time | Measured value (mm) | GA-LSSVM | | GRNN | | BP | |
|---|---|---|---|---|---|---|---|
| | | Predicted value (mm) | Relative error (%) | Predicted value (mm) | Relative error (%) | Predicted value (mm) | Relative error (%) |
| 2012/10/1 | 1505.19 | 1561.869 | 3.77 | 1578.221 | 4.85 | 1583.026 | 5.17 |
| 2012/11/1 | 1522.491 | 1580.602 | 3.82 | 1590.364 | 4.46 | 1597.352 | 4.92 |
| 2012/12/1 | 1522.491 | 1580.359 | 3.80 | 1586.605 | 4.21 | 1591.506 | 4.53 |
| 2013/1/1 | 1522.491 | 1581.923 | 3.90 | 1593.249 | 4.65 | 1600.855 | 5.15 |
| 2013/2/1 | 1539.792 | 1585.652 | 2.98 | 1599.822 | 3.90 | 1606.609 | 4.34 |
| 2013/3/1 | 1557.093 | 1600.959 | 2.82 | 1605.274 | 3.09 | 1617.769 | 3.90 |
| 2013/4/1 | 1557.093 | 1601.648 | 2.86 | 1606.713 | 3.19 | 1606.812 | 3.19 |
| 2013/5/1 | 1557.093 | 1608.744 | 3.32 | 1612.571 | 3.56 | 1615.702 | 3.76 |
| 2013/6/1 | 1574.394 | 1620.881 | 2.95 | 1622.897 | 3.08 | 1618.934 | 2.83 |
| 2013/7/1 | 1574.394 | 1620.703 | 2.94 | 1632.984 | 3.72 | 1623.904 | 3.14 |
| 2013/8/1 | 1591.696 | 1631.651 | 2.51 | 1643.08 | 3.23 | 1637.051 | 2.85 |
| 2013/9/1 | 1591.696 | 1630.511 | 2.44 | 1652.604 | 3.83 | 1647.566 | 3.51 |
| 2013/10/1 | 1591.696 | 1633.119 | 2.60 | 1653.808 | 3.90 | 1653.139 | 3.86 |

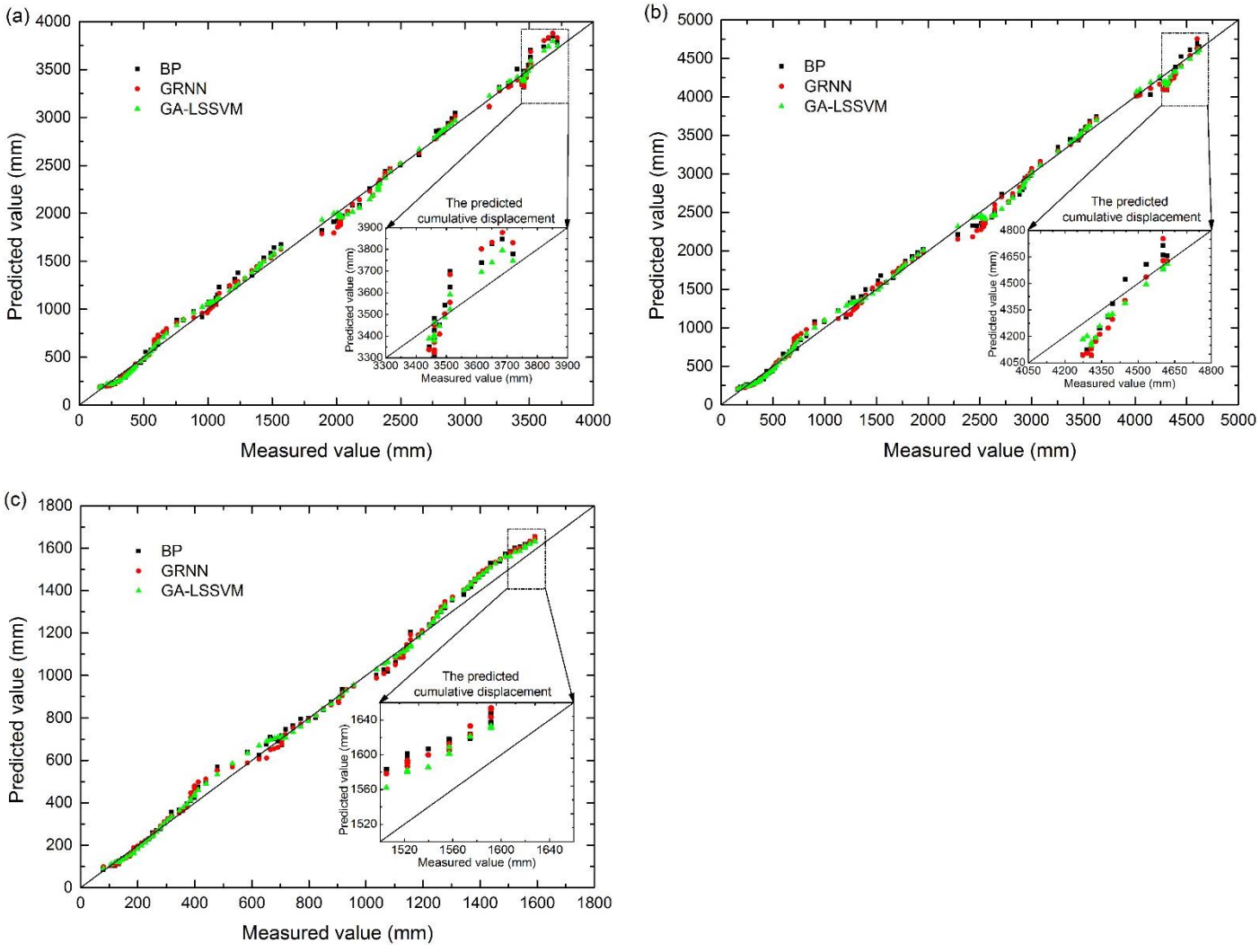



Fig. 13 Measured values versus predicted values of the cumulative displacement: (a) monitoring station ZG85, (b) monitoring station ZG86, and (c) monitoring station ZG87

## 5 Verification and error analyses

Three loss functions are used to assess the prediction performance and accuracy of the proposed model: the root mean square error (*RMSE*), mean absolute error (*MAE*), and mean absolute percentage error (*MAPE*). Then, the optimal parameters with minimum error are used to train the LSSVM model. The *RMSE*, *MAE* and *MAPE* formulas are as follows:

$$RMSE = \sqrt{\frac{1}{n}\sum_{i=1}^{n}(s_i - s_i^*)^2} \quad (19)$$

$$MAE = \frac{1}{n}\sum_{i=1}^{n}\left|s_i - s_i^*\right| \quad (20)$$

$$MAPE = \frac{1}{n}\sum_{i=1}^{n}\left|\frac{s_i - s_i^*}{s_i}\right| \quad (21)$$

where $s_i$ is the measured value, $s_i^*$ is the predicted value, and $n$ is the number of predicted values.

The performances of different models for landslide displacement prediction are assessed based on the *RMSE*, *MAE* and *MAPE*, as presented in Table 6. The prediction precision of the GA-LSSVM model based on time series analysis is better than that of the GRNN and the BP. Notably, the *RMSE*, *MAE* and *MAPE* values of the GA-LSSVM model were 63.4076, 56.6098 and 1.587% lower than those of the GRNN model, respectively, and 49.3696, 43.5537 and 1.225% lower than those of the BP model for monitoring station ZG85. The predicted results for monitoring stations ZG86 and ZG87 exhibited similar trends. According to the prediction results, the GA-LSSVM model has good deduction ability for landslide displacement prediction and can provide assistance in early risk assessment and landslide forecasting.

Table 6 Comparison of the performance of cumulative displacement prediction for the three models

| Model | *RMSE* (mm) | | | *MAE* (mm) | | | *MAPE* (%) | | |
|---|---|---|---|---|---|---|---|---|---|
| | ZG85 | ZG86 | ZG87 | ZG85 | ZG86 | ZG87 | ZG85 | ZG86 | ZG87 |
| GA-LSSVM | 62.4146 | 87.7215 | 49.0485 | 53.0048 | 74.0601 | 48.5392 | 1.492 | 1.703 | 3.131 |
| GRNN | 125.8222 | 134.6764 | 59.8173 | 109.6146 | 115.1067 | 59.2756 | 3.079 | 2.643 | 3.821 |
| BP | 111.7842 | 123.1948 | 62.0223 | 96.5585 | 107.6724 | 60.9701 | 2.717 | 2.464 | 3.935 |

## 6 Conclusion

Landslide displacement prediction is a major focus of contemporary landslide research. We use the deformation of a episodic movement landslide (Shuping landslide) as an example. According to time series analysis, the cumulative displacement is decomposed into a trend component displacement representing the trend of landslide deformation in the long term and a periodic component displacement that represents short-term deformation fluctuations. The trend displacement and periodic displacement are predicted using a polynomial function and the GA-LSSVM model, respectively. The LSSVM yields good fitting results in predicting the periodic displacement with the GA, which is utilized to determine the optimal parameters of the LSSVM. Based on our analysis of the deformation of Shuping landslide, the reservoir water level, rainfall and groundwater have major influences on the cumulative displacement. Therefore, based on the relational grades, we select six influential factors as the input variables. The predicted cumulative displacement is obtained from the sum of the predicted trend displacement and the predicted periodic displacement.

The GA-LSSVM model displays the highest accuracy, the smallest *RMSE* of 62.4146 mm, the smallest *MAE* of 53.0048 mm, and the smallest *MAPE* of 1.492% at monitoring station ZG85, while these three values are 87.7215 mm, 74.0601 mm and 1.703% at monitoring station ZG86 and 49.0485 mm, 48.5392 mm and 3.131% at monitoring station ZG87. The study results show that GA-LSSVM provides good performance for landslide displacement prediction, and the GA is appropriate for determining the optimal parameters used in the LSSVM model. Thus, the GA-LSSVM model can be effectively used to predict landslide displacement and reflect the corresponding relationships between the major influencing factors and the periodic component displacement.

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