

Dear Editor

According to your request I'm submitting a modified version of the manuscript. We have modified the text, trying to avoid similarity with our previously published papers.

Please consider that some duplicates are related to the description of terms of the equations and they can not be deleted either modified.

Best regards,

Michele D'Ambrosio and co-authors

Interactive comment on **“Regional physically based landslide early warning modelling: soil parameterization and validation of the results”** by Teresa Salvatici et al.

**Anonymous Referee #1      Received and published: 11 January 2018**

*I think this manuscript presents the application of the HIRESS code to forecast shallow landslides at the regional scale. Especially the geotechnical and hydrological input data were measured in 12 sites and then the spatial distribution of measured data was estimated by Monte Carlo simulation. Through the application of HIRESS code, it is possible to forecast the shallow landslide using rainfall data in the special area with regional scale. So I think it deserves to be published in NHESS after some minor problems are solved clearly. Some minor problems are as follows;*

AC: We thank the referee with his/her revision and fruitful comments.

**1:** *Firstly, I wonder how to consider the unsaturated soil parameters such as bubbling pressure in the HIRESS code. I think the unsaturated soil parameters were not considered in this manuscript. As you know the shallow landslide is induced by the rain infiltration into the ground and saturation of the surface soil layer. To analyze this phenomenon, the relationship between matric suction and water contents in the surface soil layer was considered in a view of unsaturated soil mechanism.*

AC: We thank the referee for the comment but we are not sure to have properly understood the comment. In particular, we are not sure if the comment wants to highlight that the unsaturated parameters were not considered in the analysis. If this is the key point we want to stress that the HIRESS model considers the effect of matric suction in unsaturated soils, taking into account the increase in strength and cohesion. The variation of matric suction based on volumetric water content, defined through the hydrological model, is modelled taking into account the parameters of the soil characteristic curves (the bubbling pressure, the pore size index distribution and the residual water content). Unfortunately, we have not defined the soil characteristic curve experimentally but the soil characteristic curves parameters were derived from literature values (Rawls et al., 1982) based on the soil types measured through laboratory analysis. We will revise the text providing a more clear and in-depth explanation on how the parameters of unsaturated soils have been taken into account in the analysis.

**2:** *Second, to make Thiessen's polygons for the rainfall data in a certain area, the rainfall data in study area as well as out of the study area especially around the study area should be used. But, in this study, the rainfall data in the only study area were used to make Thiessen's polygons. Also, the modification method of Thiessen's polygons should be verified.*

AC: To properly run the HIRESSS model we needed spatially distributed rainfall data; the most obvious approach could be the use of a geostatistical model to interpolate rainfall data (e.g. IDW or Kriging), but these approaches are not suitable for the study area, because of the morphology of the territory (small valleys surrounded by high mountains), that is not considered in these models. So, we decided to define a sort of "relevance area" of each rain gauge and the same rainfall value (for each hourly time step) has been assigned inside each area.

We used only rain gauges of the study area because we did not have other stations to be used in the definition of the Thiessen's polygons.

The modification of polygons has been carried out to take into account the morphology of the area and to avoid that data of some rain gauges could be considered in different river basins.

**3:** *Finally, in this manuscript, the final aim is to set-up the early warning system for shallow landslide with regional scale. But this manuscript focused on the application of the HIRESS code to the special area to forecast shallow landslide. Therefore, this part should be corrected and complemented to match up with the overall contents of the manuscript.*

AC: As discussed also in the introduction of the manuscript, warning systems for landslides can be designed and employed at different reference scales. In particular local systems for single slopes and regional systems. Usually the term regional refers to an area bigger than the single slope. Here below a list of selected references that report regional application of physically based models:

Baum, R., Savage, W., Godt, J., 2002. Trigrs: A FORTRAN program for transient rainfall infiltration and grid-based regional slope- stability analysis, Open-file Report, US Geol. Survey.

Baum, R.L., Godt, J.W., Savage, W.Z., 2010. Estimating the timing and location of shallow rainfall-induced landslides using a model for transient unsaturated infiltration. J Geophys Res 115:F03013.

Chen, H.X., Zhang, L.M., 2014. A physically-based distributed cell model for predicting regional rainfall-induced shallow slope failures. Engineering Geology doi:10.1016/j.enggeo.2014.04.011

Dietrich, W., Montgomery, D., 1998. Shalstab: a digital terrain model for mapping shallow landslide potential. NCASI (National Council for Air and Stream Improvement) Technical Report, February, 1998.

Rossi, G., Catani, F., Leoni, L., Segoni, S., Tofani, V., 2013. HIRESSS: a physically based slope stability simulator for HPC applications. Nat. Hazards Earth Syst. Sci., 13, pp. 151–166.

Salciarini, D., Fanelli, G., Tamagnini, C., 2017. A probabilistic model for rainfall-induced shallow landslide prediction at the regional scale, 386 Landslides, 14(5), 1731–1746.

We think that the term regional is appropriate and it can be left in the manuscript.



**Anonymous Referee #2      Received and published: 28 January 2018**

*The topic of the work meets the scope of the journal well. However, it is difficult for readers to recognize its contributions to the science community from its title, abstract and even the introduction part. The Introduction, Methodology and Discussion sections are not well-structured and pose difficult for readers to understand.*

AC: We would like to thank the referee for his/her careful revision and fruitful comments. We agree with the referee that the manuscript needs an in-depth revision especially concerning the structure and organization of the sessions. We are currently working in this direction and we are completely reorganization the contents of the Introduction, methodology and discussion.

*My specific concerns are listed below:*

**1)** *The introduction part fails to convey the current research gap and readers have difficulty to assess its scientific significance. It is unable to convince the readers why the authors carry out this work. It seems that authors want to share with the community some improvements by considering soil and vegetation parameters by using the existing model HIRESSS. I recommend the authors first detail the research question clearly, and then briefly describe their way to solve the problem*

AC: We thank the referee for the comments. We are rewriting the Introduction, trying to highlight better our key research questions and which are the main objectives of the research work. Our main objective is to test the application of an, already developed, physically-based model to forecast the occurrence of shallow landslides in a selected case study. Furthermore the work wants to highlight some improvements related to the soil parameters characterization and contribution of vegetation to slope stability. In order to be consistent between title and contents of the manuscript we propose to change the title

from: Regional physically based landslide early warning modelling: soil parameterisation and validation of the results.

to: Application of a physically-based model to forecast shallow landslides occurrence at regional scale.

**2)** *The Methodology part is mixed with Results. For example, lines 123-135 were measured results.*

AC: We agree with the referee and we are currently restructuring the text in order to separate methodology and results.

**3)** *The structure of the Methodology is not logical. I suggest the authors put an outline paragraph at the beginning of this section, in which they brief the logics of this section. “3.3 HIRESSS description” and “3.4 HIRESSS input data” should be placed in the beginning of the Methodology.*

AC: Again we agree with the referee. The methodological part has being revised in order to be more readable and clear.

**4)** *Although physically based landslide model is desirable, the input data is enormous and rigorous. The data of root cohesion and some of the soil values seem to be derived from existing literature review. Is it really proper to directly use these data in your study? You should justify this problem.*

AC: The physically based models require many hydrological and geotechnical parameters as input data. In many cases, for each geotechnical parameter, a constant value is used for the whole study area as averaged from in situ measurements or derived from literature data. In some studies, a limited degree of spatial variability is ensured using a certain value for distinct geological, lithological, or

engineering geological units, as derived from direct measurements or from existing databases and published data.

In this work we have tried to characterize as much as possible the soil covers from a hydrological and geotechnical point of view, through several direct in-situ and laboratory measurements. In particular the measured parameters are: effective cohesion, friction angle, dry unit weight, hydraulic conductivity effective porosity.

Some other parameters have not been measured, in particular we have not defined the soil characteristic curve experimentally but the soil characteristic curves parameters were derived from literature values based on the soil types measured through laboratory analysis.

At the same time the experimental evaluation of root cohesion is quite complicated and time demanding and we have chosen to define this value based on relevant literature for the different types of vegetation cover.

We will explain better this issue in the text and we will critically examine it in the discussion.

**5)** *Please detail the acquired time, spatial resolution and other characteristics of the DEM used in the model.*

AC: We will add this information in the text.

**6)** *The discussion part is poorly written. Authors should explain the results, compare with other's work, provide implications, acknowledge its limitations and echo the introduction part. I think this part should be significantly improved.*

AC: As already said before we are completely reorganizing the discussion session.

**Anonymous Referee #3      Received and published: 4 February 2018**

*The paper corresponds to the journal scope. In a general point of view: the paper is not very well structured, it is difficult for the reader to understand the message of the authors and to follow the text. The text lacks of consistency and some improvements are requested in order to publish the paper. Some recent references has to include and some sentences should be simplified. More precisely and point by point:*

AC: Dear Referee, Thanks for your detailed revision. We agree that the manuscript needs a general reorganization of the structure, with special reference to the methodology and discussion of the results. We are currently working in this direction and we are completely reorganization the contents of the introduction, methodology and discussion.

*1. The abstract and the introduction have to be rewritten. For instance, the problematic is not visible. The authors have to put the problem(s), the solution in general (with a state of the art) and after the contribution of their research. Clarify the introduction please.*

AC: We thank he referee for the comment. We are rewriting the Introduction, trying to highlight better our key research questions and which are the main objectives of the research work.

Our aim is to test the application of an already developed, physically based model to forecast the occurrence of shallow landslides in a selected case study in Italy. Furthermore the work wants to highlight some model improvements related to the soil parameters characterization and contribution of vegetation to slope stability

*2. The geographical description has to be modified. The description is not straightforward. In general you can start by the geological context with the lithology and the structure and after the landscape and the geomorphology of the area. After you follow by the weather and if you have information by land use.*

AC: We agree; we are modifying this part according to the referee comment.

*3. The methodology is not very well described, please revised it with a part about the HIRESS model, and after HIRESS data. The problem of root reinforcement can be put in the introduction or if you want absolutely speak about this topic, make a part called “ background”. Moreover, the part about data is few explained. Improve it please.*

AC: We agree, we are revisiting this part. The methodology will start with the description of the HIRESS model and then the input data. The problem of root reinforcement will be treated in the Introduction and then we will describe in the methodology how we have taken into account this parameter in our model.

4. *I think there is some lack of description about the root influence in your model and the way to obtain these information.*

AC: The problem of root reinforcement will be treated in the Introduction and then we will describe in the methodology how we have taken into account this parameter in our model.

5. *I think the monte carlo approach coupled with uncertainty is not new for landslide susceptibility assessment with PBM, there are some references to include in your text as Mergili et al., 2014 or Thiery et al., 2017 with r. slope. stability or ALICE tool used this approach to integrate the uncertainty of environmeent (geotechnical values). You have to mention these references in your text. doi:10.1016/j.geomorph.2013.10.008 or Thiery et al. : Thiery, Y., Vandromme, R., Maquaire, O., Berneradie, S., 2017. HYPERLINK [http://link.springer.com/chapter/10.1007/978-3-319-53498-5\\_104](http://link.springer.com/chapter/10.1007/978-3-319-53498-5_104)Land- slide susceptibility assessment by EPBM (Expert physically based model): strategy of calibration in complex environment. In: Mikoš, M., Tiwari, B., Yin, Y., Sassa, K. (Eds) Advancing Culture of Living with Landslides. Proceedings, Vol. 2: Advances in Landslide Science, Springer, 4th World Landslide Forum in Ljubljana, pp.917-926. [https://doi.org/10.1007/978-3-319-53498-5\\_104](https://doi.org/10.1007/978-3-319-53498-5_104). You can mention the last paper with TRIGRS :<https://doi.org/10.1007/s10346-017-0931-7> 6.*

AC: We thank the referee and we will include these references in the text.

6: *Finally, the discussion is not a discussion. In a scientific paper the discussion emphasize the results, the advantages of the method but also the drawbacks, the comparison with another approaches. In your text, there are any comments like that.*

*We suggest another structure of the text as follow: 1. Study sites 2. Background (if you choose this way) 3. Model: description, improvement and the strategy used (calibra- tion, etc) 4. Data used or created for your study 5. Results 6. Discussion 7. Conclusion.*

*One point not discussed in my first comments: Where is the relation with early warning system?It is a little explianed but it is not justify the term "early warning" in the tittle. I think you have to improve this topic in your text if you want to improve your it.*

AC: We thank the referee for the fruitful comment. We are completely reorganizing the text and consequently the structure of the manuscript.

7: *One point not discussed in my first comments: Where is the relation with early warning system?It is a little explianed but it is not justify the term "early warning" in the tittle. I think you have to improve this topic in your text if you want to improve your it.*

AC: In order to be consistent between title and contents of the manuscript we propose to change the title

From: Regional physically based landslide early warning modelling: soil parameterisation and validation of the results.

To: Application of a physically-based model to forecast shallow landslides occurrence at regional scale.

## Résumé des commentaires sur nhess-2017-425\_reviewer03.pdf

### Page : 1

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:06:43 please, simplify the text. one sentence is sufficient*

AC: Done

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:11:09*

*I think you can start the sentence by another term like: it is possible to define reliable alert levels by statistical analysis of failure probability.*

AC: This part has been removed.

### Page : 2

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:13:45 simplify the text please, the sentence is hard to understand in one read*

AC: Done

*Auteur : Sujet : Texte surligné Date : 04/02/2018 18:01:18*

AC: The introduction is now change in many parts as required.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:14:41 for shallow landslide ? for deep landslide? please re-precise*

AC: The introduction is now change in many parts as required.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:16:18*

*I think you can simplify the tex by one sentence. You hav etoo barrative sentence, please go to the subject straightaway*

AC: The introduction is now change in many parts as required.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:18:38*

*Which context do you speak ? vegetation mitigation is not always used by geotechnical office. in lot of cases the solution are not based on vegetation solutions reinforcement, wall, etc.....). please revise this part of the text.*

AC: The introduction is now change in many parts as required.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:20:19*

*ok, but it is a specific case. you study site is in this context ? if not maybe you can improve your introduction by giving the different context of vegetation solution and related context they are used.*

*AC: The introduction is now change in many parts as required.*

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:22:40*

*ok, but please put transition sentence. what is the relation between the text about root reinforcement and he used of HIRESS ? where is the problematic in this introduction ?*

*AC: The introduction is now change in many parts as required.*

### **Page : 3**

*Auteur : Sujet : Note Date : 28/01/2018 17:23:54*

*there is a lack of one problematic in the introduction. Please revise it.*

*AC: Tanks as we explained in the general comments, we proceeded to change the introduction in many parts, inserting the objectives of the work.*

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:25:50*

*ok, but what do you mean by high climatic variations? rainfall? others? please give some information*

*AC: The climate of the region is characterized by high variability strongly influenced by altitude (ranging from 400 m a.s.l of Dora Baltea's river floodplain to 4810 m a.s.l. of Mont Blanc), with a continental climate in the valleys floor and an Alpin climate at high altitudes.*

*Auteur : Sujet : Note Date : 28/01/2018 17:29:51*

*In a geographical description, the best is to start with geological settings which give the structure of the landscape, after the geomorphological processes are given in order to explain the landscape formation since the last glaciation. please revise your text.*

*AC: Thank you for the comments we modified the text as required: "From a geological point of view, the Valle d'Aosta is located NW with respect to the Insubrica Line, in particular, there are three systems of Europa chain: the Austroalpino, the Pennidiche and the Elvetico-Ultraelevato systems (De Giusti, 2004). Fig. 2 shows the lithological map of the study area obtained by reclassifying the geological units according to 11 lithological groups: landslides, calcareous schist, alluvial deposits, glacial deposits, colluvial deposits, glacier, granites, mica schists, green stone, black schists and serpentinites. In detail in the study area the main lithologies outcropping are metamorphic and intrusive rocks, in particular granites, metagranites, schists and serpentinite.*

The geomorphology of the region is characterized by steep slopes and valleys shaped by glaciers. The glacial modelling is shown in the U-shaped of Lys and Ayas valleys, and the erosive depositional forms found in the Ayas valley. The three valleys' watercourses, the Lys creek, the Evançon creek, and the Dora Baltea river, contributed to the glacial deposits modelling with the formation of alluvial fans. The climate of the region is characterized by high variability strongly influenced by altitude (ranging from 400 m a.s.l of Dora Baltea's river floodplain to 4810 m a.s.l. of Mont Blanc), with a continental climate in the valleys floor and an Alpin climate at high altitudes."

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:32:17*

*for landslides have you some references ? how do you know the processes, their typology etc... moreover more explanations about shallow landslides (this is the object of the early warning system you propose) should be welcome. please detail landslides phenomenon.*

AC: The informations about landslide have been taken from Catasto dei Dissesti Regionale –Val d'Aosta, <http://catastodissesti.partout.it>, we added some information about the shallow landslide considered: "The slope steepness, together with mean annual precipitation of 800-900 mm are the main landslide triggering factors. These features lead the study area to be prone to landsliding, in particular rock falls, deep seated gravitational slope deformations (DSGSD), rocks avalanches, debris avalanches, debris flows, and debris slides (Catasto dei Dissesti Regionale – form Val d'Aosta Regional Authorities). In this work we model the triggering conditions of shallow landslides, i.e. soil slips and translational slides and we do not take into account the other types of movement."

**Page : 4**

*Auteur : Sujet : Note Date : 28/01/2018 17:36:37*

*I don't understand how you have structured the text. I think you have to give the detail of each formations before, which formation have been investigated?*

AC: We change the structure of the text as suggested, the methodology will start with the description of the HIRESSS model and then the input data preparation.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:40:17*

*You chose the slope deposits point to investigate by analysis of DTM?*

*I am very surprised, for me the field survey, observations is the first way to chose good locations.*

*Have you make a detail geomorphological analysis of the study sites? This is the first step to conduct a slope instability analysis.*

AC: We wanted know some field informatinos about the properties of soil deposits and so we chose some survey points based on geographic, lithological information and on landslide map. When we were on field some accessible areas were private and therefore it was not possible to analyze them.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:42:05*

*you can simplify. Please make one reference about the protocol. if the reader wants more information, he can read the protocol on another paper.*

AC: Done, we simplify the text and insert some protocol reference.

## **Page : 5**

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:45:12*

*you have to reduce this part, lot of thing can be read in litterature. Simplify the text.*

AC: Done thank you for the comment The root reinforcement is insert as static data in the section 3.2 HIRESSS Input data preparation.

## **Page : 6**

*Auteur : Sujet : Texte surligné Date : 28/01/2018 17:47:36*

*simplify the text about roots etc ...*

AC: Done

## **Page : 7**

*Auteur : Sujet : Texte surligné Date : 04/02/2018 18:00:48*

*the approach is not new, Mergili et al., 2014 or Thiery et al., 2017 with r. slope. stability or ALICE tool used this approach to integrate the uncertainty of environmeent (geotechnical values). You have to mention these references in your text.*

*doi:10.1016/j.geomorph.2013.10.008*

*Thiery et al.*

*Thiery, Y., Vandromme, R., Maquaire, O., Berneradie, S., 2017. HYPERLINK "http://link.springer.com/ chapter/10.1007/978-3-319-53498-5\_104" Landslide susceptibility assessment by EPBM (Expert physically based model): strategy of calibration in complex environment. In: Mikoš, M., Tiwari, B., Yin, Y., Sassa, K. (Eds) Advancing Culture of Living with Landslides. Proceedings, Vol. 2: Advances in Landslide Science, Springer, 4th World Landslide Forum in Ljubljana, pp.917-926. https://doi. org/10.1007/978-3-319-53498-5\_104*

*you can mention the last paper with TRIGRS :https://doi.org/10.1007/s10346-017-0931-7*

AC: The use of Monte Carlo Simulation inside the HIRESSS code is not new, it is just explained in the work of Rossi et al.(2013), we also include the suggested reference in the text.

## **Page : 8**

*Auteur : Sujet : Note Date : 28/01/2018 18:01:15*

*I think you have a problem of structure in your text. I think it is better to present 1. study sites*



*2 Model, improvement and the strategy used (calibration etc)*

*3 data used or created for your study*

*4 results*

*The text will be more clear and understandable*

AC: Thank you for the comment, this part is also recommended by the other reviewers we have provided to change the structure of the paper as suggested.

*Auteur : Sujet : Texte surligné Date : 28/01/2018 18:03:21*

*this sentence has to be in another part related to the problematic of environmental lack of data or problems.*

AC: this sentence about validation is now in the discussion of the model results.

## **Page : 9**

*Auteur : Sujet : Note Date : 04/02/2018 18:01:50*

*the discussion is poor, the main goal of a discussion is to criticize results, methodology and have an objective vision of the research. I think the discussion has to be improved by authors.*

*I don't see the real contribution of the study. Hard work was made, but the text does not reflect this work. You have to improve your text.*

AC: We will explain better this issue in the text and we will critically examine it in the discussion. We completely reorganizing the discussion section.

# Application of a physically-based model to forecast shallow landslides at regional scale

Teresa Salvatici<sup>1</sup>, Veronica Tofani<sup>1</sup>, Guglielmo Rossi<sup>1</sup>, Michele D'Ambrosio<sup>1</sup>, Carlo Tacconi Stefanelli<sup>1</sup>, Elena Benedetta Masi<sup>1</sup>, Ascanio Rosi<sup>1</sup>, Veronica Pazzi<sup>1</sup>, Pietro Vannoci<sup>1</sup>, Miriana Petrolo<sup>1</sup>, Filippo Catani<sup>1</sup>, Sara Ratto<sup>2</sup>, Hervé Stevenin<sup>2</sup> and Nicola Casagli<sup>1</sup>

<sup>1</sup>Department of Earth Sciences, University of Firenze, Firenze, 50121, Italy

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Correspondence to: Michele D'Ambrosio (michele.dambrosio@unifi.it)

Abstract.

In this work, we apply a physically-based model, namely the HIRESSS (High REsolution Stability Simulator) model, to forecast the occurrence of shallow landslides at regional scale. HIRESSS is a physically based distributed slope stability simulator for analysing shallow landslide triggering conditions during a rainfall event. The software is made of two parts: hydrological and geotechnical. The hydrological model is based on an analytical solution of an approximated form of the Richards equation while the geotechnical stability model is based on an infinite slope model that takes into account the unsaturated soil condition. The test area is a portion of the Valle d'Aosta region, located in North-West Alpine mountain chain. The geomorphology of the region is characterized by steep slopes with elevations ranging from 400 m a.s.l. of Dora Baltea's river floodplain to 4810 m a.s.l. of Mont Blanc. In the study area, the mean annual precipitation is about 800-900 mm. These features lead to the territory to be very prone to landslides, mainly shallow rapid landslides and rock falls. In order to apply the model and to increase its reliability, an in-depth study of the geotechnical and hydrological properties of hillslopes controlling shallow landslides formation was conducted. In particular, two campaigns of on site measurements and laboratory experiments were performed with 12 survey points. The data collected contributes to generate input map of parameters for HIRESSS model. In order to consider the effect of vegetation on slope stability, the soil reinforcement due to the presence of roots has been also taken into account based on vegetation maps and literature values of root cohesion. The model was applied in back analysis on two past events that have affected Valle d'Aosta region between 2008 and 2009, triggering several fast shallow landslides. The validation of the results, carried out using a database of past landslides, has provided good results and a good prediction accuracy of the HIRESSS model both from temporal and spatial point of view.

## 1 Introduction

Landslide prediction at regional scale can be performed following two approaches: a) rainfall thresholds based on statistical analysis of rainfall and landslides and b) physically-based deterministic models. While the first approach is currently extensively used at regional scale (Aleotti, 2004; Cannon et al., 2011; Martelloni et al., 2012; Rosi et al., 2012; Lagomarsino et al., 2013), the latter is more frequently applied at slope or catchment scale (Dietrich and Montgomery 1998; Pack et al. 2001; Baum et al. 2002, 2010; Lu and Godt 2008; Simoni et al. 2008; Ren et al. 2010; Arnone et al. 2011; Salciarini et al., 2012; Park et al., 2013; Rossi et al. 2013; Salciarini et al. 2017). The poor knowledge of hydrological and geotechnical parameters spatial distribution, caused by the extreme heterogeneity and inherent variability of soil at large scale (Mercogliano et al., 2013; Tofani et al., 2017), mainly avoid the physically-based model

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**Eliminato:** Regional

**Eliminato:**

**Eliminato:** for

**Eliminato:** landslide early warning modelling: soil parameterisation and validation of the

**Eliminato:** of

**Eliminato:** results

**Eliminato:** The final aim is the set-up of an early warning system at regional scale for shallow landslides.

**Eliminato:** in real time and in large areas using parallel computational techniques.

**Eliminato:** can run in real-time by assimilating weather data and uses Monte Carlo simulation techniques to manage

**Eliminato:** and hydrological input parameters.

**Eliminato:** a high hydrogeological hazard in

**Eliminato:** whole

**Eliminato:** , as mass movements interest the 70% of the municipality areas (

**Eliminato:** ).

**Eliminato:** take into account

**Eliminato:** contribution of the root cohesion

**Eliminato:** the

**Eliminato:** map

**Eliminato:** .

**Eliminato:** A statistical analysis of the HIRESSS outputs in terms of failure probability has been carried out in order to define reliable alert levels for regional landslide early warning systems.

**Eliminato:** A landslide early warning system is defined as the set of capacities needed to generate and disseminate timely and meaningful warning information to enable individuals, communities and organizations threatened by hazards to prepare and act appropriately and in sufficient time to reduce the possibility of harm or loss (UNISDR, 2009). Warning systems for landslides can be designed and employed at different reference scales. Two categories of early warning systems can be defined on the basis of their scale of analysis: local systems for single slopes (Intrieri et al., 2013) and regional systems. Regional early warning systems for shallow landslides can be developed

**Eliminato:** This is because the

78 application at regional scale. On the other hand, physically-based models allow to predict spatially and temporally the  
 79 occurrence of landslides with high accuracy producing accurate hazard maps that can be of help for landslide risk  
 80 assessment and management.

81 In this work, we apply the physically based model, named HIRESSS (Rossi et al., 2013) in Eastern part of Valle d'Aosta  
 82 region (Italy), in North-West Alpine mountain chain in order to test the capacity of the model to forecast the occurrence  
 83 of shallow landslides at regional scale. In particular, the objectives of the work are: i) to properly characterise the  
 84 geotechnical and hydrological parameters of the soil to feed the HIRESSS model and to spatialize this punctual  
 85 information in order to have spatially-continuous maps of the model input data ii) to test the HIRESSS code for two  
 86 selected rainfall events that have triggered several shallow landslides and to validate the model results. HIRESSS is a  
 87 physically based distributed slope stability simulator for analysing shallow landslide triggering conditions in real time  
 88 and in large areas using parallel computational techniques. In the area selected, an in-depth study of the geotechnical and  
 89 hydrological properties of hillslopes controlling shallow landslides formation was conducted, performing two campaigns  
 90 (12 survey points) of in-situ measurements and laboratory tests. Furthermore, the HIRESSS model has been modified to  
 91 take into account the effect of the root reinforcement to the stability of slopes based on plant species distribution, and  
 92 literature values of root cohesion.

- 93 ▲
- 94
- 95 •
- 96 •

## 97 2 Study area and rainfall events

98 The study area, called alert Zone B by the regional civil protection authorities, is located in eastern part of Valle d'Aosta  
 99 region, in North-West Alpine mountain chain (Fig. 1). The area is characterized by three main valleys: Champorcher  
 100 valley, Gressoney or Lys valley, and Ayas valley. The first is located on the right side of Dora Baltea water catchment,  
 101 and represents the southern part of the study area. The second and third valleys show N-S orientation, and they are  
 102 delimited to north by Monte Rosa massif (4527 m a.s.l) and to south by Dora Baltea river.

103 From a geological point of view, the Valle d'Aosta is located NW with respect to the Insubrica Line, in particular, there  
 104 are three systems of Europa chain: the Austroalpino, the Pennidiche, and the Elvetico-Ultraelevato systems (De Giusti,  
 105 2004). Fig. 2 shows the lithological map of the study area obtained by reclassifying the geological units according to 11  
 106 lithological groups: landslides, calcareous schist, alluvial deposits, glacial deposits, colluvial deposits, glacier, granites,  
 107 mica schists, green stone, black schists and serpentinites. In detail in the study area the main lithologies outcropping are  
 108 metamorphic and intrusive rocks, in particular granites, metagranites, schists and serpentinite.

109 The geomorphology of the region is characterized by steep slopes and valleys shaped by glaciers. The glacial modelling  
 110 is shown in the U-shaped of Lys and Ayas valleys, and the erosive depositional forms found in the Ayas valley. The three  
 111 valleys' watercourses, the Lys creek, the Evançon creek, and the Dora Baltea river, contributed to the glacial deposits  
 112 modelling with the formation of alluvial fans. The climate of the region is characterized by high variability strongly  
 113 influenced by altitude (ranging from 400 m a.s.l of Dora Baltea's river floodplain to 4810 m a.s.l. of Mont Blanc), with a  
 114 continental climate in the valleys floor and an Alpin climate at high altitudes.

115 The slope steepness, together with mean annual precipitation of 800-900 mm are the main landslide triggering factors.  
 116 These features lead the study area to be prone to landsliding, in particular rock falls, deep seated gravitational slope

Eliminato: ... [1]

Eliminato:

**Eliminato:** modelling the effect of vegetation in terms of roots reinforcement has to be taken into account on slopes stability since it plays a crucial role (Gray and Magahan, 1981). Mainly through the root systems, in fact, vegetation strongly affects the mechanical and hydrological soil behaviour, and in particularly the shallow landslides triggering processes. Except for particular contexts, the vegetation constitutes a mitigating element for the instability (Chirico et al. 2013). The stabilizing action of the vegetal communities in the slopes vadose zone is mainly due to reinforcement of the soil by the root network (increase of the tensile strength) (Gray and Sotir, 1996; Vergani et al., 2017).

Eliminato: the vegetation map

**Spostato in giù [1]:** The HIRESSS model simulated two past events, one in 2008 and one in 2009, and the validation of the model performance was carried out comparing the results with the landslide regional database. In particular: 24 - 31 May 2008: on 28 and 29 May 2008 intense and persistent rainfall was recorded across the Valle d'Aosta region with a total precipitation in the study area of about 250 mm causing flooding, debris flows and rockfalls. 25 - 28 April 2009: from 26 April to 28 April 2009 heavy rainfall affected the south-eastern part of the Valle d'Aosta region, with the highest precipitation recorded at the Lillianes Granges station of about 268 mm. This precipitation triggered several landslides.

**Eliminato:** Eventually, a discussion on how the model results can be analysed in order to set up an early warning system is provided.

Eliminato: represent

**Eliminato:** The geomorphology of the region is characterized by steep slopes, high climatic and altitude (ranging from 400 m a.s.l of Dora Baltea's river floodplain to 4810 m a.s.l. of Mont Blanc) variability. From a geomorphologic point of view, valleys shaped by glaciers characterize the territory

**Spostato in giù [2]:** The glacial modelling is shown in the U-shaped of Lys and Ayas valleys, and the erosive depositional forms found in the Ayas valley. The three valleys' watercourses, the Lys creek, the Evançon creek, and the Dora Baltea river, contributed to the glacial deposits modelling with the formation of alluvial fans.

Eliminato:

Eliminato:

Eliminato: (alert Zone B)

Eliminato: 8

Eliminato: group

Eliminato: Calcareous schist, Granites, Mica

Eliminato: Pietre Verdi.

**Spostato (inserimento) [2]**

**Formattato:** Car. predefinito paragrafo

**Eliminato:** to a high hydrogeological hazard in the whole territory

**Eliminato:** mass movements interest the 70% of the municipality areas, as:

168 deformations (DSGSD), rocks avalanches, debris avalanches, debris flows, and debris slides (Catasto dei Dissesti  
 169 Regionale – form Val d'Aosta Regional Authorities). In this work we model the triggering conditions of shallow  
 170 landslides, i.e. soil slips and translational slides and we do not take into account the other types of movement.  
 171 The HIRESSS model simulated two past events, one in 2008 and one in 2009, and the validation of the model performance  
 172 was carried out comparing the results with the landslide regional database.  
 173 In particular:  
 174 • 24 - 31 May 2008: on 28 and 29 May 2008 intense and persistent rainfall was recorded across the Valle d'Aosta  
 175 region with a total precipitation in the study area of about 250 mm causing flooding, debris flows and rockfalls.  
 176 • 25 - 28 April 2009: from 26 April to 28 April 2009 heavy rainfall affected the south-eastern part of the Valle  
 177 d'Aosta region, with the highest precipitation recorded at the Lillianes Granges station of about 268 mm. This  
 178 precipitation triggered several landslides.  
 179

**Eliminato:** Deep Seated Gravitational Slope Deformations

**Eliminato:** flow

**Eliminato:** slide.

**Spostato (inserimento) [1]**

### 180 3 Methodology

#### 181 3.1 HIRESSS description

182 The physically-based distributed slope stability simulator HIRESSS (Rossi et al., 2013) is a model developed to analyse  
 183 shallow landslide triggering conditions on large scale at high spatial and temporal resolution using parallel calculation  
 184 method. Two parts compose the model: hydrological and geotechnical (Rossi et al., 2013). The hydrological part is based  
 185 on a dynamical input of the rainfall data which are used to calculate the pressure head and provide it to the geotechnical  
 186 stability model. The hydrological model is initiated as a modelled form of hydraulic diffusivity, using an analytical  
 187 solution of an approximated form of the Richards equation under the wet condition (Richards, 1931). The equation  
 188 solution allows us to calculate the pressure head variation ( $h$ ), depending on time ( $t$ ) and depth of the soil ( $Z$ ). The solutions  
 189 are obtained by imposing some boundary conditions as described by Rossi et al. (2013).  
 190 The geotechnical stability model is based on an infinite slope stability model. The model considers the effect of matric  
 191 suction in unsaturated soils, taking into account the increase in strength and cohesion. The stability of slope at different  
 192 depths ( $Z$  values) is computed since the hydrological model calculates the pressure head at different depths. The variation  
 193 of soil mass caused by water infiltration on partially saturated soil is also modelled. The original FS equations (Rossi et  
 194 al., 2013) were modified taking into account the effect of root reinforcement ( $c_r$ ) as an increase of soil cohesion ( $c'$ )  
 195 according to the Eq. 1:

**Eliminato:** 3.1 Soil Geotechnical and hydrological characterization

... [2]

**Spostato (inserimento) [3]**

$$196 c_{tot} = c' + c_r$$

197 (1)

**Spostato (inserimento) [4]**

198 Regarding the geotechnical influence of roots on the soil strength, roots seem to affect the cohesion parameter only, while  
 199 the friction angle would be poorly or not at all interested by reinforcement (Waldron and Dakessian, 1981; Gray and  
 200 Ohashi 1983; Operstein and Frydaman, 2000; Giadrossich et al., 2010). Therefore, is necessary to consider the root  
 201 cohesion in calculating FS and consequently in applying HIRESSS model.

**Spostato (inserimento) [5]**

**Formattato:** Colore carattere: Testo 1

202 The root reinforcement (or root cohesion) can be considered equal to (Eq. 2):

**Spostato (inserimento) [6]**

**Formattato:** Colore carattere: Testo 1

$$203 c_r = kT_r(A_r/A)$$

**Spostato (inserimento) [7]**

**Formattato:** Colore carattere: Testo 1

$$204 (2)$$

where  $T_r$  is the root failure strength (tensile, frictional, or compressive) of roots per unit area of soil.  $A_r/A$  the root area ratio (proportion of area occupied by roots per unit area of soil),  $k$  a coefficient dependent on the effective soil friction angle and the orientation of roots. The measure of  $c_r$  varies with vegetal species, within a single species depends on how plants respond to environmental characteristics and fluctuations.

Spostato (inserimento) [8]

Formattato: Colore carattere: Testo 1

The new equation of FS at unsaturated conditions is therefore (Eq. 3):

Spostato (inserimento) [9]

Spostato (inserimento) [10]

$$FS = \frac{\tan \varphi}{\tan \alpha} + \frac{c_{tot}}{\gamma_d \gamma \sin \alpha} + \frac{\gamma_w h \tan \varphi \left\{ 1 + (h_b^{-1} |h|)^{\lambda+1} \right\}^{\frac{\lambda}{\lambda+1}}}{\gamma_d \gamma \sin \alpha} \quad (3)$$

where  $\varphi$  is the friction angle,  $\alpha$  is the slope angle,  $\gamma_d$  is the dry soil unit weight,  $\gamma$  is the depth,  $\gamma_w$  is the water unit weight,  $h$  is the pressure head,  $h_b$  is the bubbling pressure, and  $\lambda$  is the pore size index distribution. In saturated condition the equation of FS (Rossi et al., 2013) becomes (Eq. 4):

Spostato (inserimento) [11]

Spostato (inserimento) [12]

$$FS = \frac{\tan \varphi}{\tan \alpha} + \frac{c_{tot}}{(\gamma_d(\gamma - h) + \gamma_{sat} h) \sin \alpha} - \frac{\gamma_w h \tan \varphi}{(\gamma_d(\gamma - h) + \gamma_{sat} h) \tan \alpha} \quad (4)$$

where  $\gamma_{sat}$  is the saturated soil unit weight.

Spostato (inserimento) [13]

One of the major problems, associated with the deterministic approach employed on a large scale, is the uncertainty of the static input parameters or geotechnical parameters of the soil. The method used for the estimation of parameters spatial variability is the Monte Carlo Simulation. The Monte Carlo simulation achieves a probability distribution of input parameters providing results in terms of slope failure probability (Thiery et al. 2017). The developed software uses the computational power offered by multicore and multiprocessor hardware, from modern workstations to supercomputing facilities (HPC), to achieve the simulation in reasonable runtimes, compatible with civil protection real time monitoring (Rossi et al. 2013). The HIRESSS model loads spatially distributed data arranged as 12 input raster maps and the maps of rainfall intensity. These input raster maps are: slope gradient; effective cohesion ( $c'$ ); root cohesion ( $c_r$ ); friction angle ( $\varphi'$ ); dry unit weight ( $\gamma_d$ ); soil thickness; hydraulic conductivity ( $k_s$ ); initial soil saturation ( $S$ ); pore size index ( $I$ ); bubbling pressure ( $h_b$ ); effective porosity ( $n$ ); and residual water content ( $q_r$ ), and rainfall intensity.

Spostato (inserimento) [14]

### 3.2 HIRESSS input data preparation

The input parameters can be divided in two classes: the static data and the dynamical data. Static data are geotechnical and morphological parameters while the dynamical data is represented by the hourly rainfall intensity. Static data are read only once at the beginning of the simulation while dynamical inputs are continuously updated.

The HIRESSS input are in raster, therefore point data and parameters have to be adequately spatially distributed. In this application the spatial resolution was 10 m.

#### Static data

The slope gradient was calculated from the DEM (Digital Elevation Model- 2006). Effective cohesion, friction angle, hydraulic conductivity, effective porosity and dry unit weight, were obtained, spatializing according to lithology, the soil punctual parameters derived from the in situ and laboratory geotechnical tests and analysis.

In particular, the properties of slope deposits were determined by in situ and laboratory measurements (Bicocchi et al., 2016; Tofani et al., 2017) at 12 survey points. To carry out the in situ tests the survey points were selected following these

248 characteristics: i) physiography, ii) landslides occurrence, and iii) geo-lithology (Fig. 2). Regarding the first point, a high-  
 249 resolution DEM (from Val d'Aosta Regional Authorities) ~~together with a careful first surveys were~~ used to locate the  
 250 most suitable slopes. The surveys took place in two sessions, the first one in August 2016, and the second one in  
 251 September 2016. The following analyses were conducted:

Eliminato: was

- 252 • registration of geographical position using a GPS, ~~and photographic documentation of the site characteristics~~  
 253 (morphology and vegetation);
- 254 • in situ measurement of saturated hydraulic conductivity ( $k_s$ ) by means of the constant-head well permeameter  
 255 Amoozegar;
- 256 • sampling of an aliquot (~2 kg each) of the material for laboratory tests, including grain size distributions, index  
 257 properties, Atterberg limits and direct shear tests.

Eliminato:

258 The permeability in-situ measurements and the soil samplings were made at depth ranging from 0.4 to 0.6 m below the  
 259 ground level. The evaluation of the  $k_s$  (saturated hydraulic conductivity or permeability) was made with the *Amoozegar*  
 260 permeameter (Amoozegar, 1989). The measurement was obtained by observing the amount of water required to maintain  
 261 a constant volume of water into the hole. In situ measurements are then applied into the Glover solution (Amoozegar,  
 262 1989), which calculates the saturated permeability of the soils. The  $k_s$  is a very useful parameter not only for slope stability  
 263 modelling but also for many other hydrological problems (groundwater, surface water runoff and sub-surface, flow  
 264 calculation of water courses).

Eliminato: Eq. 1),

Eliminato: -

[3]

265 In addition, the in situ collected samples were examined in the laboratory to define a wide range of parameters to  
 266 characterize more extensively the deposits. In particular, the following tests were performed in order to classify the  
 267 analysed soils:

- 268 • grain size distribution (determination of granulometric curve for sieving and settling following ASTM  
 269 recommendations), and classification of soils (according to AGI and USCS classification, Wagner, 1957);
- 270 • determination of the main index properties (porosity, relationships of phases, natural water content  $w_n$ , natural  
 271 and dry unit weight  $\gamma$  and  $\gamma_d$ ) following the ASTM recommendations;
- 272 • determination of Atterberg limits (liquid limit LL, plastic limit PL, and plasticity index PI);
- 273 • direct shear test on selected samples.

274 ~~Soil thickness was calculated by the GIST model (Catani et al., 2010; Del Soldato et al., 2016). Soil characteristic curves~~  
 275 ~~parameters (pore size index, bubbling pressure, and residual water content) were derived from literature values (Rawls et~~  
 276 ~~al., 1982).~~

Eliminato: Based on the result obtained from the

Spostato (inserimento) [15]

277 ~~Root cohesion variations in the area (at the soil depth chosen for the physical modelling with HIRESSS) were obtained~~  
 278 ~~firstly, identifying the plant species and determining their distribution from in situ observations and vegetational maps~~  
 279 ~~(Carta delle serie di vegetazione d'Italia, Italian Ministry of the Environment and Protection of Land and Sea). Then, the~~  
 280 ~~measure of cohesion due to the presence of roots was assigned to each subarea according to the dominant plant species~~  
 281 ~~and literature root cohesion for that species (Bischetti, 2009; Burylo et al., 2010; Vergani et al., 2013) that were calculated~~  
 282 ~~considering the Fiber Bundle Model (Pollen et al., 2004). The measure of  $c_r$  varies with vegetal species, within a single~~  
 283 ~~species depends on how plants respond to environmental characteristics and fluctuations, so map of root cohesion~~  
 284 ~~variations obtained as mentioned is a simplification of reality. This is a necessary simplification as the known methods~~  
 285 ~~to evaluate root cohesion variations are not suitable for wide areas and acceptable measurement times.~~

Spostato (inserimento) [16]

286 ~~The last static input data, in this case of study, is the exposure rock mask. This was defined considering the lithological~~  
 287 ~~and land use maps, so that HIRESSS model avoided the simulation on steep rock slopes areas.~~

294 The geotechnical properties and root cohesion of the soils have been spatialized with respect to a lithological  
 295 classification.  
 296 For each lithological class and plant species the mean value has been selected in order to obtain the HIRESSS input raster  
 297 parameters.

#### 299 Dynamic data

300 In the study area, the rainfall hourly data from 27 pluviometers were available, therefore it was necessary to spatially  
 301 distribute them to generate 10x10 m cell size input raster to ensure the correct program operation. The rainfall data were  
 302 elaborated applying the Thiessen's polygon methodology (Rhynsburger, 1973) modified to take into account the elevation.  
 303 Thiessen's polygon methodology, in fact, allows us to divide a planar space in some regions, and to assign the regions to  
 304 the nearest point feature. This approach defines an area around a point, where every location is nearer to this point than  
 305 to all the others. Thiessen's polygon methodology do not consider the morphology of the area, so the alert Zone B was  
 306 divided in three catchment areas and the polygons were calculated for each rain gauges considering the reference  
 307 catchment basin (Fig. 3).

#### 308 4 Results

309 The results of the geotechnical and hydrological characterization of the soils of the 12 survey points are shown in Table  
 310 1 for all survey sites.

311 The results of granulometric tests shown that the analysed soils are predominantly sands with silty gravel (Fig. 4 and  
 312 Table 1). Regarding the index properties, the natural soil water content values were predominantly about 20% by weight,  
 313 with a maximum and minimum values of 5.1% and 26.2%, respectively. These values reflect their different ability to hold  
 314 water in their voids. The measured natural unit weight ( $\gamma$ ) was variable between 15.3 kN/m<sup>3</sup> and 21.7 kN/m<sup>3</sup>, depending  
 315 not only on the different grain size distribution but also by different thickening and consolidation states. Regarding  
 316 saturated unit weight ( $\gamma_{sat}$ ) the measured values range between 18.2 kN/m<sup>3</sup> and 21.5 kN/m<sup>3</sup> (Table 1).

317 The Atterberg limits (LL and PL) were measured on samples with a sufficient passing fraction (> 30% by weight) through  
 318 40 ASTM (0.425 mm) sieve. For sandy prevalent samples, LL values are predominantly around 40% of water content (%  
 319 by weight), while the PL is around 30% (Table 1).

320 The effective friction angle varies between a minimum of 25.6° and a maximum of 34.3°, while the effective cohesion  
 321 ranges from a minimum of 0.0 kPa to a maximum of 9.3 kPa. Consistent with the presence of sandy soils, the saturated  
 322 permeability values were around a medium-high value of 10<sup>-6</sup> m/s. The minimum and maximum values were found  
 323 between 1.36·10<sup>-7</sup> m/s and 1.54·10<sup>-5</sup> m/s. Considering the poor variability of samples, the permeability values were  
 324 relatively homogeneous and in accordance with the values reported in the literature (Table 1).

325 Most commonly used models to quantify rooted soils strength are based on a Mohr-Coulomb failure criterion for  
 326 unsaturated soil in which a term representing root reinforcement is added (Eq. 2):

$$327 \tau = c' + (\mu_a - \mu_w) \tan \varphi_b + (\sigma - \mu_a) \tan \varphi' + c_r$$

328 (2)

329 where  $\tau$  is the soil-shearing resistance,  $c'$  effective cohesion,  $\mu_a$  the pore-air pressure,  $\mu_w$  the pore-water pressure,  $\varphi_b$  the  
 330 angle describing the increase in shear strength due to an increase in matric suction ( $\mu_a - \mu_w$ ),  $\sigma$  the normal stress on the  
 331 shear plane,  $\varphi'$  the effective soil friction angle, and  $c_r$  the increase in shear strength due to roots.

Spostato (inserimento) [17]

Spostato (inserimento) [18]

Formattato: Colore carattere: Testo 1

Eliminato: ,

Eliminato: 3

Eliminato: -

Eliminato: of

Eliminato: 3.2. Evaluation of root reinforcement - ... [4]

Spostato in su [5]: , roots seem to affect the cohesion parameter only, while the friction angle would be poorly or not at all interested by reinforcement (Waldron and Dakessian, 1981; Gray and Ohashi 1983; Operstein and Frydman, 2000; Giadrossich et al., 2010).

Formattato: Colore carattere: Testo 1

Spostato in su [6]: The root reinforcement (or root cohesion) can be considered equal to (Eq.

Formattato: Colore carattere: Testo 1

Eliminato: 3

Spostato in su [7]: ):

$c_r = kT_r(A_r/A)$

Formattato: Colore carattere: Testo 1



348  $c_r = kT_r(A_r/A)$  The additional cohesion induced by roots assumes different values not only depending on plant species  
 349 and environmental characteristics, but also on depth of soil, as roots diameter and density vary with latter. Because of  
 350 such evidence, studies on roots cohesion of different species report values as function of depth of soil. In the area of the  
 351 case study, soils have thinner thickness than those ones in which these studies are carried out. In such thin soils, root  
 352 systems organize their growth depending on available space not reaching the same depth of roots of thick soils.  
 353 Consequently, in this context root cohesion of species at the different depth is dissimilar related to literature values.  
 354 Considering this, map for variation of root cohesion is processed taking for each species the minimum cohesion (among  
 355 those specified for each species at the different depth) reported in literature. By doing this, contribution of vegetation to  
 356 stability of slopes is considered in FS calculate and at the same time, it is avoided an overestimate of root cohesion.  
 357 In the area, root cohesion defined as mentioned above ranges from a minimum of 0.0 kPa (mainly in the outcrop area) to  
 358 maximum of 8.9 kPa (area occupied by mountain maple on the left bank of river Dora Baltea).

359

360

361

362  $c_{tot} = c' + c_r$

363 
$$FS = \frac{\tan \varphi}{\tan \alpha} + \frac{c_{tot}}{\gamma_d \gamma \sin \alpha} + \frac{\gamma_w h \tan \varphi \left( 1 + (h_b^{-1} |h|)^{\lambda+1} \right)^{\frac{\lambda}{\lambda+1}} - 1}{\gamma_d \gamma \sin \alpha}$$

364 
$$FS = \frac{\tan \varphi}{\tan \alpha} + \frac{c_{tot}}{(\gamma_d (\gamma - h) + \gamma_{sat} h) \sin \alpha} - \frac{\gamma_w h \tan \varphi}{(\gamma_d (\gamma - h) + \gamma_{sat} h) \tan \alpha}$$

365

366

367 In Table 2, the mean values of each input parameters respect to lithological class were reported.

368 The pore size index, bubbling pressure and residual water content are constant in whole area of: 0,322 (-); 0,1466 m and  
 369 0,041 (-), respectively.

370 The distributed soil parameters maps are shown in Fig. 5.

371 The results of rainfall data elaborated using Thiessen's polygon methodology are 192 and 96 rainfall hourly maps for the  
 372 2008 and 2009 event, respectively. In Fig. 6 are reported the cumulative maps of each event.

373 All these data have been inserted in the HIRESSS model to obtain day-by-day maps of landslide occurrence probability.

374 The main characteristics of simulation are showed in Table 3. Before to discuss the model results is necessary to check  
 375 false positive for both the simulated events, the first day of simulation, characterized by the absence of rainfall, was  
 376 analysed. The results showed that those pixels with a high landslide occurrence probability are unstable because of  
 377 morphometric reasons, predominantly high slope angles. To remove these false positive, a numeric mask was applied.  
 378 Using the GIS software commands, it was possible to calculate the number of pixels of the first simulation day with a  
 379 trigger probability value greater than 80% and delete them (Fig. 7). The mask was then applied to the rest of landslide  
 380 occurrence probability maps.

381

382 The resulting maps for each days of the simulated

383 events are shown in the Fig. 8 and Fig. 9.

384

Spostato in su [8]:	where Tr is the root failure strength (t ... [5]
Eliminato:	... (3) ...
Eliminato:	... [7]
Formattato	... [6]
Eliminato:	... to maximum of 8.9 kPa (in the ... rea occupie ... [8]
Eliminato:	3.3
Spostato in su [3]:	HIRESSS description ... [9]
Eliminato:	4
Spostato in su [4]:	... [10]
Eliminato:	... (4) ...
Spostato in su [9]:	The new equation of FS at unsaturate ... [11]
Eliminato:	5
Spostato in su [10]:	... [12]
Eliminato:	5) ...
Spostato in su [11]:	where $\varphi$ is the friction angle, $\alpha$ is t ... [13]
Eliminato:	6
Spostato in su [12]:	... [14]
Eliminato:	6) ...
Spostato in su [13]:	where $\gamma_{sat}$ is the saturated soil unit v ... [15]
Eliminato:	...
Spostato in su [14]:	The developed software uses the ... [16]
Eliminato:	...
Spostato in su [15]:	was calculated by the GIST model ... [18]
Eliminato:	1982) and they are constant in whole area. Roof ... [19]
Spostato in su [16]:	2010; Vergani et al., 2013) that we ... [20]
Eliminato:	The initial soil saturation was empirical defined ... [21]
Spostato in su [17]:	In the study area, the rainfall hourly ... [22]
Eliminato:	5
Spostato in su [18]:	... [23]
Formattato	... [24]
Eliminato:	The HIRESSS model provide... day-by-day a ... [25]
Spostato in giù [19]:	validation is necessary to have ... [26]
Formattato	... [27]
Eliminato:	event (24 - 31 May 2008) are shown in Fig. 7. ... [28]
Spostato in giù [20]:	The rainfall intensity increased f ... [29]
Eliminato:	7
Spostato in giù [21]:	b, c). In the following days rainfa ... [30]
Eliminato:	7d). ... [31]
Formattato	... [32]
Spostato in giù [22]:	a, b), because of the low rainfall in ... [33]
Eliminato:	8 c, d). ...
Spostato in giù [23]:	The temporal validation was also ... [34]
Formattato	... [35]
Eliminato:	a, b). As it could be expected, the results show ... [36]
Spostato in giù [24]:	Spatial validation was performed ... [37]
Eliminato:	Figure 10
Spostato in giù [25]:	shows an example of landslide ev ... [38]
Eliminato:	10 a
Spostato in giù [26]:	and b it is possible to note that in ... [39]
Eliminato:	can not simulated by the HIRESSS model
Formattato	... [40]



811 A back analysis was carried out to evaluate the model performance from a temporal and spatial point of view. To perform  
 812 a solid validation is necessary to have information on spatial and temporal location of landslides. In particular, the time  
 813 of occurrence is very rarely known with hourly precision, and usually landslides are related to a rainstorm, without any  
 814 more precise information on time of occurrence (Rossi et al., 2013). Concerning the spatial landslides locations, in many  
 815 cases they are included in the database only as points without any information on the area involved. In our database,  
 816 provided by the local authorities, landslides are points with information on the day of occurrence.  
 817 In general, for both events temporal validation shows that the daily highest probability of occurrence, computed by  
 818 HIRESSS, correspond with the days with real landslide occurrence and with the most intense precipitation.  
 819 The results of the first simulated event (24 - 31 May 2008) are shown in Fig. 8. The failure probability in the whole area  
 820 is negligible for the first four days (from 24 to 27 May 2008) (Fig. 8a). The rainfall intensity increased since 27 May,  
 821 reaching the highest value on 29 May, when the precipitation value was around 100 mm in the eastern sector of study  
 822 area.  
 823 The HIRESSS model well simulate this passage: the 28 May and 29 May 2008 landslide occurrence probability maps  
 824 show a considerable increase of the probability of failure with maximum values around 90% at the East of alert Zone B  
 825 (Fig. 8 b, c). In the following days rainfall intensity decreases, and also the probability slowly decreases, being anyway  
 826 still high on 30 May 2008. Landslides reported in the database are dated 30 May and 31 May 2008 (Fig. 8d).  
 827 Concerning the second event (25 - 28 April 2009) landslide occurrence probability is negligible for the first two days (25  
 828 and 26 April 2009) in the whole area (Fig. 9 a, b), because of the low rainfall intensity. From 27 April 2009 rainfalls  
 829 become more intense, especially in the southeast sector of the region, where the cumulated rainfall average was about  
 830 151 mm. This event led to many landslides triggered during these days (as reported in the database). Also the probability  
 831 maps show high values during these days (Fig. 9 c, d).  
 832 In Table 4 the results over 75% of slope failure probability for both events are highlighted and confirm the correct  
 833 temporal occurrence of landslides.  
 834 The temporal validation was also carried out considering daily cumulative rainfall compared to the landslide failure  
 835 probability. In particular, a median of landslide occurrence probability was calculated for four pluviometric areas  
 836 identified by Thiessen's polygons methodology, modified according to limits of river basins, both for the event of May  
 837 2008 and for the April 2009 event (Fig. 10 a, b). As it could be expected, the results show that when the highest rainfall  
 838 intensity is measured, the highest probability of occurrence is computed for the all areas and for both events.  
 839  
 840 Spatial validation was performed following a pixel by pixel method: this method is the most complex since it consists in  
 841 comparing the probability of instability of each pixel with the pixels involved in the actual event that occurred. This  
 842 validation implies a great deal of uncertainty in the results since the reports of landslide events may have errors on the  
 843 precise spatial location and on the size of the phenomenon. To overcome this problem and taking into account probable  
 844 errors caused by the actual spatial location in the database, an area of 1 km<sup>2</sup> (called influence area) around the point of  
 845 the landslide were considered in the validation analysis. Inside the influence area, pixels that have the 75% of probability  
 846 of failure were considered instable.  
 847 Figure 11 shows an example of landslide event occurred in the Arnad municipality on 30 May 2008. The model computes  
 848 a low failure probability on 24 May 2008 and an increase of probability on 30 May 2008. In Fig. 11a and b it is possible  
 849 to note that inside the red circle the red and yellow area increase on 30 May with respect to 24 May. In this case, the  
 850 model is able to identify correctly such movement. To better highlight this validation, Figure 10c shows the number of

**Eliminato:** The final aim of the physically-based modelling for landslide prediction is to set-up an early warning system at regional scale based on the model output. The validation of the results performed in the previous section showed that the HIRESSS model performs good results with good prediction capacity both from a spatial and temporal point of view. In this work the HIRESSS model computes the daily probability of occurrence with a spatial resolution of 10 m. In order to become an active and proficient early warning system it is necessary to define a method for the interpretation of the probabilistic results (e.g., definition of probability values corresponding to alert thresholds). Furthermore, in order to have more usable results especially for public administration and civil protection authorities it is necessary to possibly aggregate the model outputs temporally and spatially. ... [41]

Spostato (inserimento) [19]

Spostato (inserimento) [20]

Spostato (inserimento) [21]

Spostato (inserimento) [22]

Spostato (inserimento) [23]

Spostato (inserimento) [24]

Spostato (inserimento) [25]

Spostato (inserimento) [26]

866 pixels above 75% of probability calculated by the model, within the circular area of about 1 km<sup>2</sup> around the all landslides  
867 occurred during the event of 2008. For some of the reported landslide events, the number of pixels above 75% increases  
868 on 30 May 2008, only in case of the Champdepraz and Montjovet 2 events the probability does not increase. This may be  
869 caused by the low precision of location of the reported landslide, and maybe because some of the real landslides reported  
870 are other types of movements (rockfalls, rotational slides) that cannot simulated by the HIRESSS model.

## 871 6 Conclusion

872 The HIRESSS code (a physically-based distributed slope stability simulator for analysing shallow landslide triggering  
873 conditions in real time and in large areas) was applied to the eastern sector of Valle d'Aosta region in order to test its  
874 capability to forecast shallow landslides at regional scale. The model was applied in back analysis to two past rainfall  
875 events that have triggered in the study areas several shallow landslides between 2008 and 2009. In order to run the model  
876 and to increase its reliability, an in-depth study of the geotechnical and hydrological properties of hillslopes controlling  
877 shallow landslides formation was conducted. In particular, two campaigns of on site measurements and laboratory  
878 experiments were performed with 12 survey points. The data collected contributes to generate input map of parameters  
879 for HIRESSS model according to lithological classes. The effect of vegetation on slope stability in terms of root  
880 reinforcement has been also taken into account based on the plant species distribution and literature values of root  
881 cohesion to product a map of root reinforcement of the study area. The outcomes of the model are daily failure probability  
882 maps with a spatial resolution of 10 m. To evaluate the model performance both temporal and spatial validation were  
883 carried out, and in general for both the simulated events the computed highest daily probability of occurrence corresponds  
884 to the days and the areas of real landslides.

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**Spostato in giù [27]:** The outcomes of the model are daily failure probability maps with a spatial resolution of 10 m.

**Eliminato:** .

**Eliminato:** vegetation map

**Eliminato:** producing a map

**Eliminato:** .

**Spostato (inserimento) [27]**

**Formattato:** Colore carattere: Automatico

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... [43]

**Table 1.** Geotechnical properties of survey points (grain size distribution, Atterberg limits, index properties, permeability and shear strength parameters).

SITE	SOIL TYPE	G %	S %	M %	C %	LL (%)	PL (%)	PI (%)	USCS	$\gamma$ ( $kN\ m^{-3}$ )	$\gamma_d$ ( $kN\ m^{-3}$ )	$\gamma_{sat}$ ( $kN\ m^{-3}$ )	n (%)	w (%)	$k_s$ ( $m\ s^{-1}$ )	$k_{sc}$ ( $m\ s^{-1}$ )	$\phi'$ lab ( $^{\circ}$ )	c' ( $kPa$ )
Site 1	Sand with silty gravel	27.8	45.2	23.4	3.6	36	25	11	SM	16.7	13.7	18.3	47.3	11.3	/	2.52E-06	25.6	1.0
Site 2	Sand with gravelly silt	19.4	50.5	29.0	1.1	38	25	14	SC	19.1	14.5	18.8	44.3	11.4	2.71E-06	1.48E-06	34.3	1.5
Site 3	Sand with gravel and silt	26.9	45.2	26.8	1.1	/	/	/	/	/	/	/	/	/	/	8.89E-07	/	/
Site 4	Sand with gravelly silt	18.8	40.4	39.2	1.6	38	27	11	SM	19.5	14.8	19.0	43.2	10.7	1.36E-07	4.51E-07	34.3	0.0
Site 5	Sand with gravel and silt	31.0	43.1	25.7	0.2	47	36	11	SM	18.4	14.0	18.5	46.3	11.0	/	2.44E-06	25.7	9.3
Site 6	Sand with poorly silty gravel	28.5	57.5	13.9	0.1	52	38	13	SM	18.7	13.5	18.2	47.9	20.0	/	8.27E-06	30.2	4.4
Site 7	Sand with silty gravel	37.0	42.6	17.9	2.5	40	32	8	SM	20.3	15.5	19.5	40.4	26.2	5.18E-06	2.97E-06	28.2	3.4
Site 8	Sandy silty gravel	58.1	24.6	16.0	1.3	43	28	16	GM	17.2	15.7	19.6	39.6	9.4	/	3.76E-06	30.1	8.1
Site 9	Gravelly silty sand	18.7	55.1	24.4	1.8	46	36	10	SM	20.1	18.7	21.5	27.9	8.1	2.41E-06	1.73E-06	33.9	0.6
Site 10	Sand with gravelly silt	21.9	52.0	25.1	1	46	37	8	SM	18.4	16.0	19.8	38.6	15.5	/	2.10E-06	30.3	1.5
Site 11	Gravelly silty sand	24.3	51.4	21.2	3.1	31	25	7	SM	21.7	18.0	21.2	31.9	20.5	4.03E-06	3.05E-06	29.8	2.0
Site 12	Gravel with poorly silty sand	55.2	32.2	12.2	0.4	55	45	10	SM	15.3	14.6	18.9	43.9	5.1	1.54E-05	8.25E-06	30.2	1.6
MEAN		30.63	44.98	22.9	1.48	42.91	32.18	10.82		18.67	15.36	19.39	41.03	13.56	4.98E-06	3.16E-06	30.24	3.04
MEDIAN		27.35	45.2	23.9	1.2	43	32	11		18.7	14.8	19.0	43.2	11.3	3.37E-06	2.48E-06	30.2	1.6
STD.DEV		13.31	9.48	7.41	1.11	7.15	6.71	2.71	/	1.80	1.68	1.10	6.34	6.30	5.38E-06	2.56E-06	3.05	3.07
MAX		58.1	57.5	39.2	3.6	55	45	16		21.7	18.7	21.5	47.9	26.2	1.54E-05	8.27E-06	34.3	9.3
MIN		18.7	24.6	12.2	0.1	31	25	7		15.3	13.5	18.2	27.9	5.1	1.36E-07	4.51E-07	25.6	0

Tabella formattata

Table 2. Spatialized geotechnical parameters of each lithological class as input for HIRESSS model.

Lithological classes	Soil Type	$\phi'$ lab (°)	$c'$ (Pa)	$\gamma_s$ (kN m <sup>-3</sup> )	$n$ (%)	$k_s$ (m s <sup>-1</sup> )	$h_s$	$q_r$	$l$
Calcareous schist	Sand with gravelly silt	31	1000	16.5	39	1.1E-05	0.1466	0.041	0.322
Alluvial deposits	Sand with gravel and silt	26	1000	14.0	46	3.0E-06	0.1466	0.041	0.322
Glacial deposits	Sand with silty gravel	31	1000	15.3	41	2.7E-06	0.1466	0.041	0.322
Colluvial deposits	Sand with silty gravel	25	1000	13.7	47	2.5E-06	0.1466	0.041	0.322
Granites	Sandy gravel	30	1000	17.6	32	4.0E-06	0.1466	0.041	0.322
Mica schists	Sandy silty gravel	30	1000	17.7	32	6.0E-06	0.1466	0.041	0.322
Green stone	Gravel with silty sand	32	1000	16.3	37	4.6E-06	0.1466	0.041	0.322

Eliminato: Geotechnical

Tabella formattata

Table 3. Main characteristics of the simulation.

	2008 event	2009 event
Spatial resolution	10 m	10 m
Time step	1h	1h
Rainfall hours	192	96

5 Table 4. Hiresss results over 75% of slope failure probability for two events.

Event 2008	N. Pixel	Total %	Pixel area (km <sup>2</sup> )
24/05/2008	62344	1	6
25/05/2008	21295	0	2
26/05/2008	84256	1	8
27/05/2008	95220	1	10
28/05/2008	15364	0	2
29/05/2008	243137	3	24
30/05/2008	79437	1	8
31/05/2008	7110	0	1
Event 2009	N. Pixel	Total %	Pixel area (km <sup>2</sup> )
25/04/2009	0	0	0
26/04/2009	52644	1	5
27/04/2009	326826	4	33
28/04/2009	56599	1	6

Eliminato: 3. The number

Eliminato: landslides for each

Eliminato: class

Eliminato: Failure probability

Tabella formattata

Eliminato: 29 May

Formattato: Colore carattere: Nero, Italiano

Eliminato: 27 April 2009

Eliminato: Low

Eliminato: 5

Eliminato: 4

Eliminato: Medium

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Eliminato: 6

Tabella formattata

Eliminato: High

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Eliminato: 1

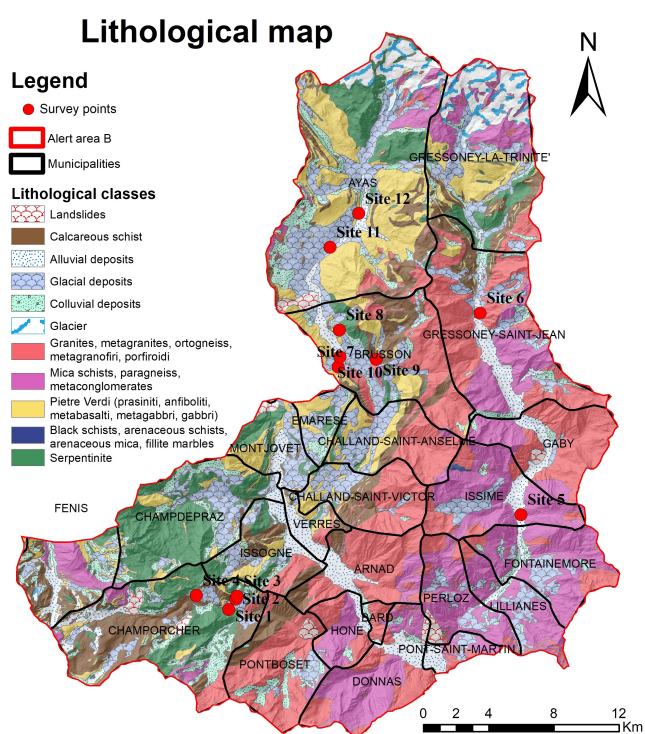
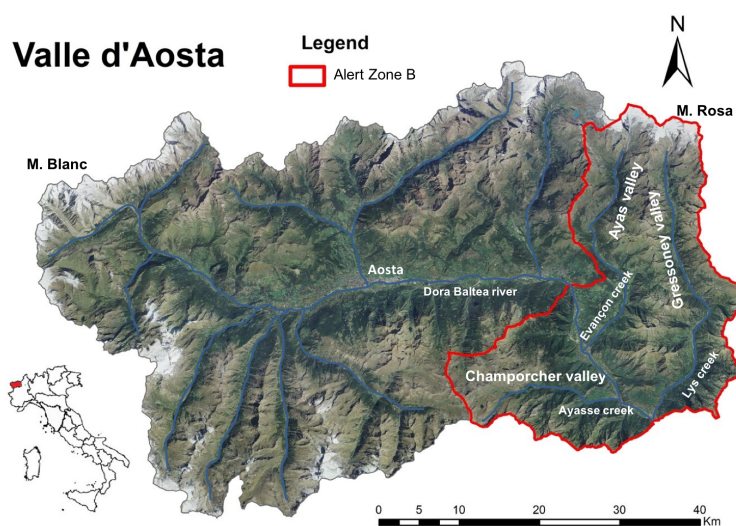




Figure 2. Spatial distribution of survey points compared to the geo-lithology.

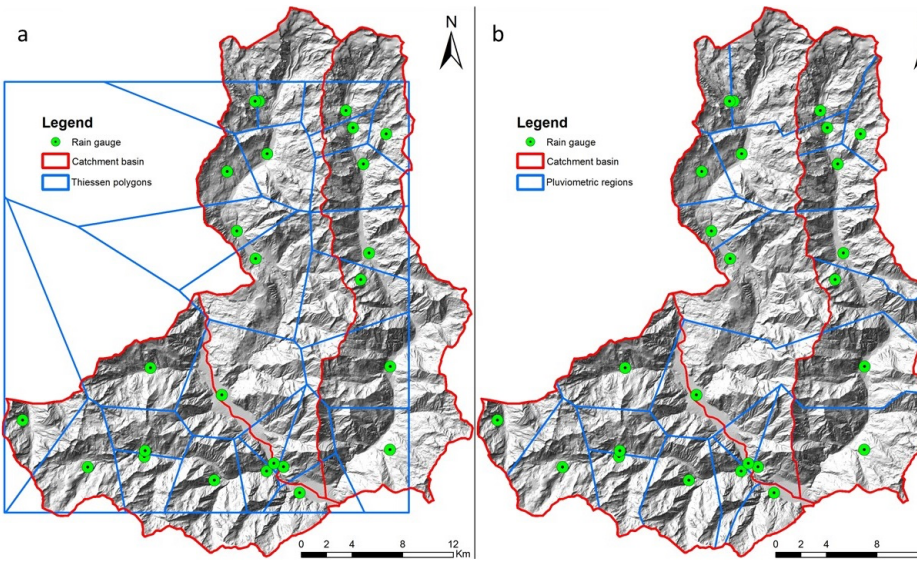


Figure 3. Comparison of Thiessen's polygons methodology a) simple b) modified according to the catchment basins boundaries.

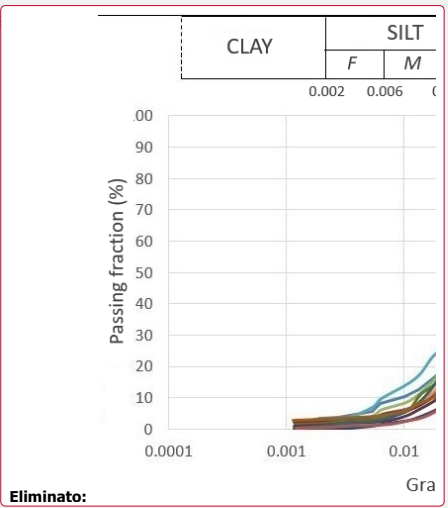
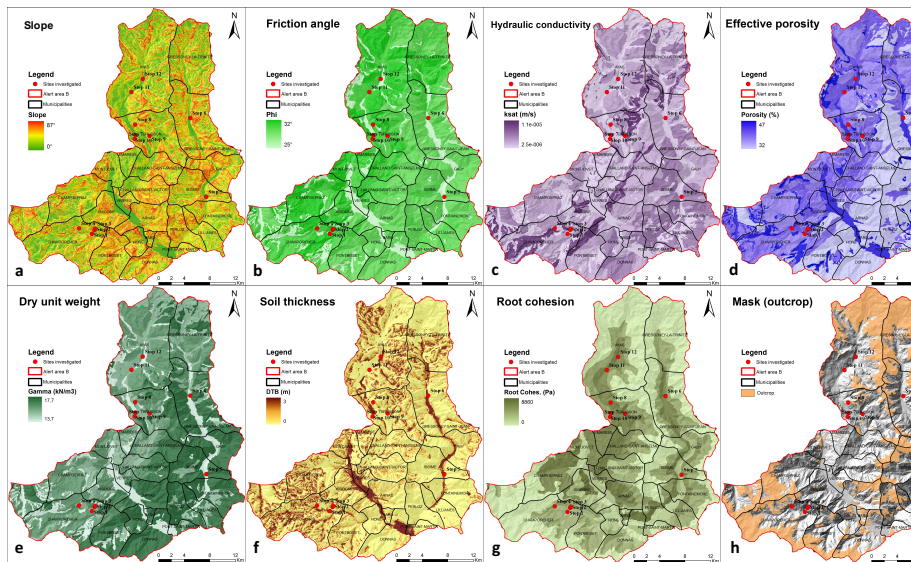
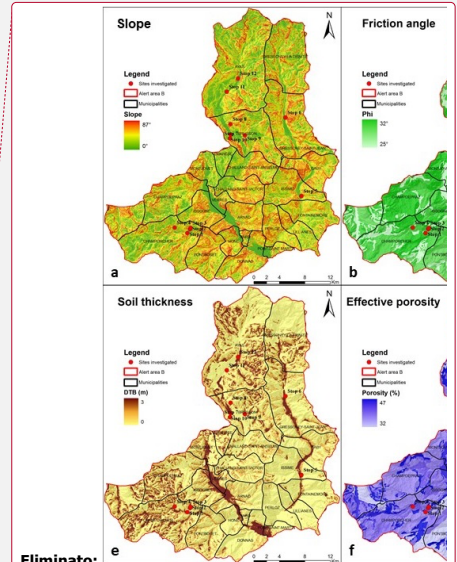


Figure 4. Grain size distributions of soil samples.



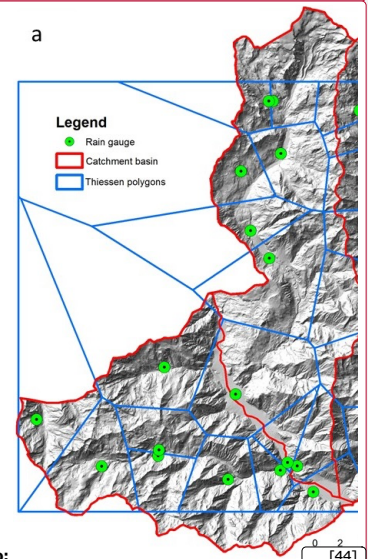


**Figure 5.** Static input parameters for HIRESSS model: a) slope gradient; b) friction angle; c) Hydraulic conductivity; d) effective porosity; e) dry unit weight; f) soil thickness; g) root cohesion; and h) exposure rock mask.

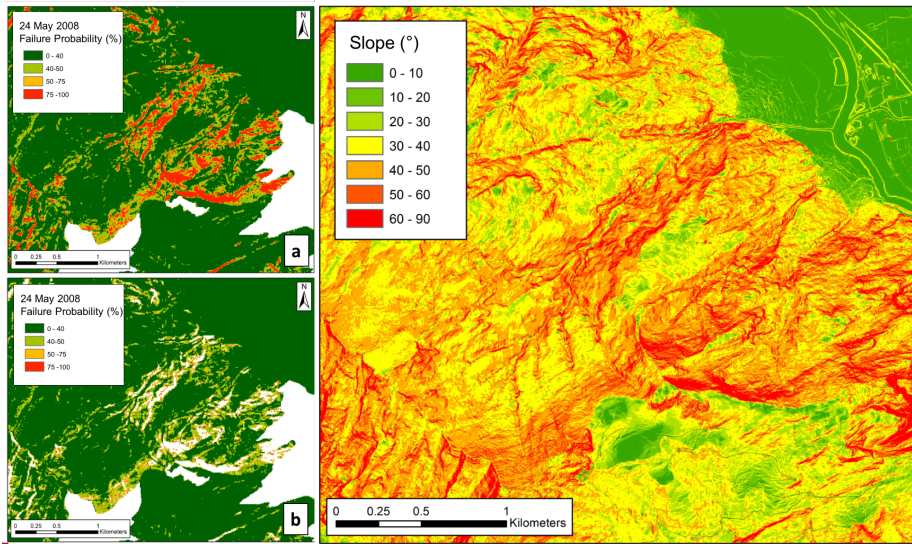


**Figure 6.** Cumulated rainfall maps for two events.

**Eliminato:**  
**Eliminato:** 4  
**Eliminato:** root cohesion; c)  
**Eliminato:** d  
**Eliminato:** e) soil thickness; f)  
**Eliminato:** g  
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**Formattato:** Spazio Dopo: 0 pt



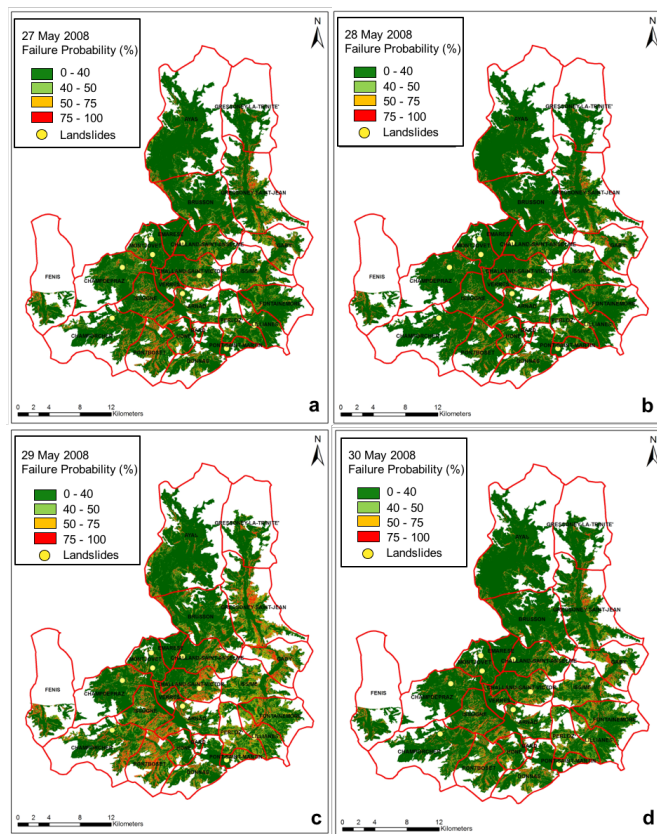
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**Spostato in su [29]:** . Comparison of Thiessen's polygons methodology a) simple b) modified according to the catchment basins boundaries. .



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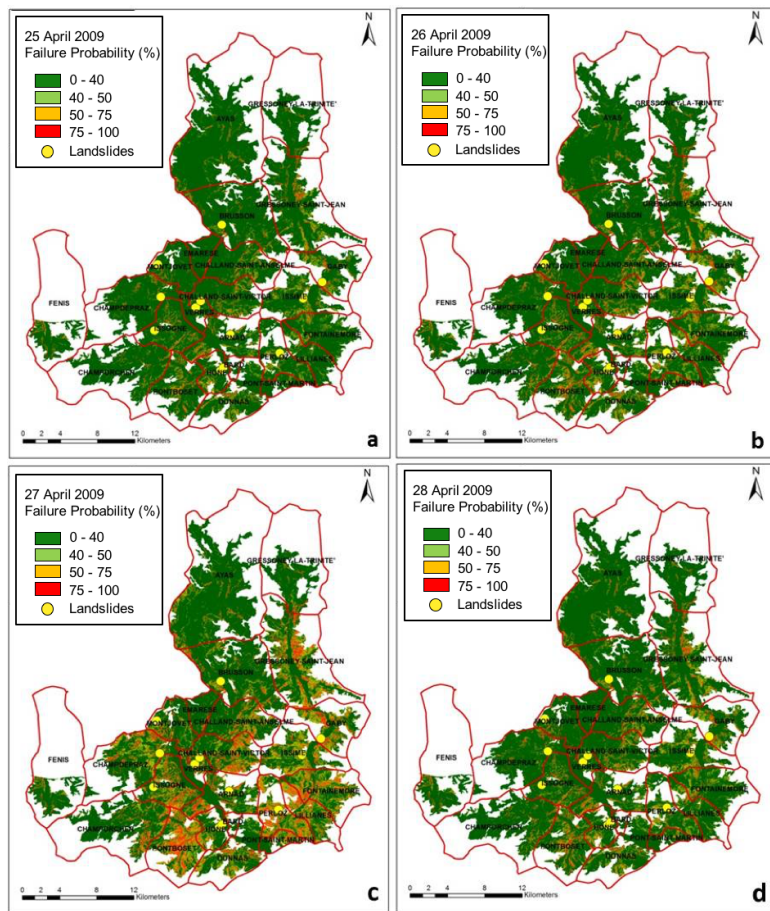
**Figure 7.** Example of numerical mask to remove the false positive of the first event simulated, between 24-31 May 2008, a) the HIRESSES result of the first day of simulation with false positive pixels, b) the probability map after the numerical mask implementation, c) the slope map shows that the pixels with high probability of landslide occurrence are located where the slope is higher than 60%.

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**Figure 8.** HIRESSS landslide probability maps of simulate event of 24-31 May 2008 and reporting landslide during this event focused on the four critical days, a) 27 May 2008, b) 28 May 2008, c) 29 May 2008, and d) 30 May 2008.

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**Figure 9.** HIRESSES landslide probability maps of simulate event between 25 - 28 April 2009 and reporting landslide during this event, a) 25 April 2009, b) 26 April 2009, c) 27 April 2009 and d) 28 April, 2009.

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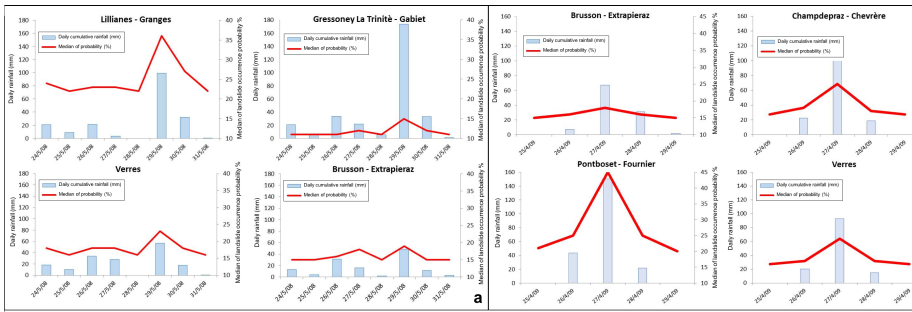


Figure 10. Correlation graphs between the daily cumulative rainfall and the median of landslide occurrence probability for both events.

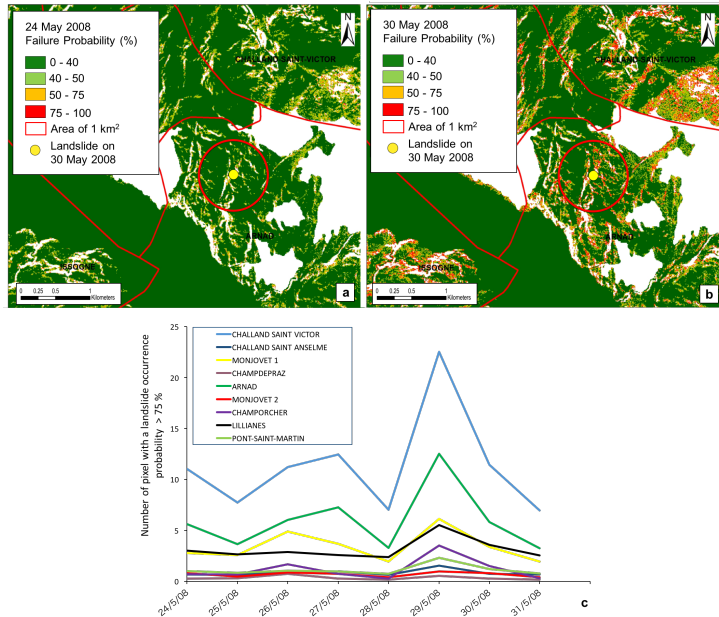
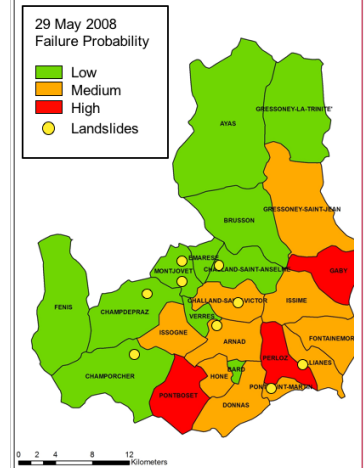
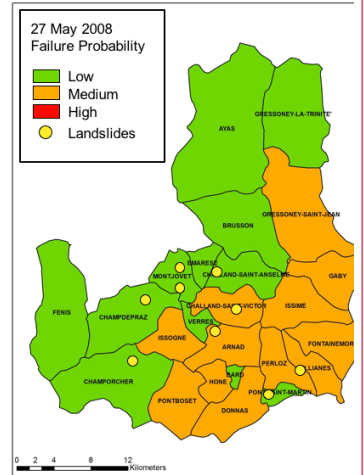


Figure 11. An example of landslide event happened in the Arnad municipality compared to landslide occurrence probability map, a) before and b) after rainfall event. c) Number of pixels above 75% of probability calculated by the model for all the landslides triggered during the event in the study area.

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### 3.1 Soil Geotechnical and hydrological characterization

The

:

$$k_s = \frac{Q \left[ \sinh^{-1} \left( \frac{h}{r} \right) - \left( \frac{r^2}{h^2} + 1 \right)^{\frac{1}{2}} + \frac{r}{h} \right]}{2\pi h^2} \quad (1)$$

where  $Q$  is the steady-state rate of water flow from the permeameter into the auger hole,  $h$  is the water depth in the borehole (constant), and  $r$  is the borehole radius.

### 3.2. Evaluation of root reinforcement

Root reinforcement is due to root tensile strength that is usually greater than the tensile strength of soil. Conversely, soil has a greater strength to compression, therefore the overall effect is a strengthened matrix soil, in which stresses are relocated from sediments to roots (Greenway, 1987). Consequently, the strength of rooted soil results from sediments nature (cohesion and friction angle), root strength and strength of soil-roots bonds (Waldron, 1977; Waldron and Dakessian, 1981; Ennos, 1990). Regarding strength parameters

where  $T_r$  is the root failure strength (tensile, frictional, or compressive) of roots per unit area of soil,  $A_r/A$  the root area ratio (proportion of area occupied by roots per unit area of soil),  $k$  a coefficient dependent on the effective soil friction angle and the orientation of roots. The measure of  $cr$  varies with vegetal species, within a single species depends on how plants respond to environmental characteristics and fluctuations.

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In view of all that has been mentioned so far, it is necessary to consider the root cohesion in calculating FS and consequently in applying HIRESSS model.

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### HIRESSS description

The physically-based distributed slope stability simulator HIRESSS (Rossi et al., 2013) is a model developed to analyse shallow landslide triggering conditions on large scale at high spatial and temporal resolution using parallel calculation method. Two parts compose the model: hydrological and geotechnical (Rossi et al., 2013). The hydrological part is based on a dynamical input of the rainfall data which are used to calculate the pressure head and provide it to the geotechnical stability model. The hydrological model is initiated as a modelled form of hydraulic diffusivity, using an analytical solution of an approximated form of the Richards equation under the wet condition (Richards, 1931). The equation solution allows us to calculate the pressure head variation ( $h$ ), depending on time ( $t$ ) and depth of the soil ( $Z$ ). The solutions are obtained by imposing some boundary conditions as described by Rossi et al. (2013).

The geotechnical stability model is based on an infinite slope stability model. The model considers the effect of matric suction in unsaturated soils, taking into account the increase in strength and cohesion. The stability of slope at different depths ( $Z$  values) is computed since the hydrological model calculates the pressure head at different depths. The variation of soil mass caused by water infiltration on partially saturated soil is also modelled. The original FS equations (Rossi et al., 2013) were modified taking into account the effect of root reinforcement ( $c_r$ ) as an increase of soil cohesion ( $c'$ ) according to the Eq.

:

$$c_{tot} = c' + c_r$$

The new equation of FS at unsaturated conditions is therefore (Eq.

):

$$FS = \frac{\tan \varphi}{\tan \alpha} + \frac{c_{tot}}{\gamma_d y \sin \alpha} + \frac{\gamma_w h \tan \varphi \left[ 1 + (h_b^{-1} |h|)^{\lambda+1} \right]^{\frac{\lambda}{\lambda+1}} - 1}{\gamma_d y \sin \alpha} \quad ($$

where  $\varphi$  is the friction angle,  $\alpha$  is the slope angle,  $\gamma_d$  is the dry soil unit weight,  $y$  is the depth,  $\gamma_w$  is the water unit weight,  $h$  is the pressure head,  $h_b$  is the bubbling pressure, and  $\lambda$  is the pore size index distribution. In saturated condition the equation of FS (Rossi et al., 2013) becomes (Eq.

):

$$FS = \frac{\tan \varphi}{\tan \alpha} + \frac{c_{tot}}{(\gamma_d(y-h) + \gamma_{sat}h) \sin \alpha} - \frac{\gamma_w h \tan \varphi}{(\gamma_d(y-h) + \gamma_{sat}h) \tan \alpha} \quad ($$

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where  $\gamma_{sat}$  is the saturated soil unit weight.

One of the major problems, associated with the deterministic approach employed on a large scale, is the uncertainty of the static input parameters or geotechnical parameters of the soil. The method used for the estimation of parameters spatial variability is the Monte Carlo Simulation. The Monte Carlo simulation achieves a probability distribution of input parameters providing results in terms of slope failure probability

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The developed software uses the computational power offered by multicore and multiprocessor hardware, from modern workstations to supercomputing facilities (HPC), to achieve the simulation in reasonable runtimes, compatible with civil protection real time monitoring (Rossi et al. 2013).

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### 3.4 HIRESSS input data

The HIRESSS model loads spatially distributed data arranged as input raster maps. Therefore, point data and parameters have to be adequately spatially distributed. In this application the spatial resolution was 10 m and 12 raster maps of static input parameters were prepared. These input raster were (Fig. 4): slope gradient; effective cohesion ( $c'$ ); root cohesion ( $c_r$ ); friction angle ( $\varphi'$ ); dry unit weight ( $\gamma_d$ ); soil thickness; hydraulic conductivity ( $k_s$ ); initial soil saturation ( $S$ ); pore size index ( $I$ ); bubbling pressure ( $h_s$ ); effective porosity ( $n$ ); and residual water content ( $q_r$ ).

The slope gradient (Fig. 5a) was calculated from the DEM (Digital Elevation Model). Effective cohesion, friction angle (Fig. 5b), hydraulic conductivity (Fig. 5c), effective porosity (Fig. 5f) and dry unit weight (Fig. 5g), were obtained, spatializing according to lithology, the soil punctual parameters derived from the in situ and laboratory geotechnical tests and analysis carried out as described in sect. 3.1. Soil thickness (Fig. 5e)

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was calculated by the GIST model (Catani et al., 2010; Del Soldato et al, 2016). Soil characteristic curves parameters (pore size index, bubbling pressure, and residual water content) were derived from literature values (Rawls et al.,

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1982) and they are constant in whole area. Root cohesion values (Fig. 5d), at the depth chosen for the physical modelling with HIRESSS, were obtained taking into account vegetational maps (Carta delle serie di vegetazione d'Italia, Italian Ministry of the Environment and Protection of Land and Sea) and values from literature of root cohesion (Bischetti, 2009; Burylo et al.,

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2010; Vergani et al., 2013) that were calculated considering the Fiber Bundle Model (Pollen et al., 2004).

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The initial soil saturation was empirical defined based on antecedent rainfall analysis. Moreover, considering the lithological and land use maps the exposure rock mask (Fig. 5h) was prepared, so that HIRESSS model avoided the simulation on steep rock slopes areas. The parameters are showed in Table 2 for all lithological classes.



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In the study area, the rainfall hourly data from 27 pluviometers were available, therefore it was necessary to spatially distribute them to generate 10x10 m cell size input raster to ensure the correct program operation. The rainfall data were elaborated applying the Thiessen's polygon methodology (Rhynsburger, 1973) modified to take into account the elevation. Thiessen's polygon methodology, in fact, allows us to divide a planar space in some regions, and to assign the regions to the nearest point feature. This approach defines an area around a point, where every location is nearer to this point than to all the others. Thiessen's polygon methodology do not consider the morphology of the area, so the alert Zone B was divided in three catchment areas and the polygons were calculated for each rain gauges considering the reference catchment basin (Fig.

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).

#### 4 Results

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<b>Pagina 7: [25] Eliminato</b>	<b>Teresa Salvatici</b>	<b>26/04/18 10:27:00</b>
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The HIRESSS model provide

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The HIRESSS model provide

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The HIRESSS model provide

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The HIRESSS model provide

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The HIRESSS model provide

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validation is necessary to have information on spatial and temporal location of landslides. In particular, the time of occurrence is very rarely known with hourly precision, and usually landslides are related to a rainstorm, without any more precise information on time of occurrence (Rossi et al., 2013). Concerning the spatial landslides locations, in many cases they are included in the database only as points without any information on the area involved. In our database, provided by the local authorities, landslides are points with information on the day of occurrence.

In general, for both events temporal validation shows that the daily highest probability of occurrence, computed by HIRESSS, correspond with the days with real landslide occurrence and with the most intense precipitation.

The results of the first

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event (24 - 31 May 2008) are shown in Fig. 7. The failure probability in the whole area is less than 25% for the first four days (from 24 to 27 May 2008) (Fig. 7a

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). The rainfall intensity increased since 27 May, reaching the highest value on 29 May, when the precipitation value was around 100 mm in the eastern sector of study area.

The HIRESSS model well simulate this passage: the 28 May and 29 May 2008 landslide occurrence probability maps show a considerable increase of the probability of failure with maximum values around 90% at the East of alert Zone B (Fig.

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b, c). In the following days rainfall intensity decreases, and also the probability slowly decreases, being anyway still high on 30 May 2008. Landslides reported in the database are dated 30 May and 31 May 2008 (Fig.

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7d).

Concerning the second event (25 - 28 April 2009) landslide occurrence probability is less than 25% for the first two days (25 and 26 April 2009) in the whole area (Fig.

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**Pagina 7: [33] Spostato a pagina 8 (spostamento n. 22)Teresa Salvatici 26/04/18 10:27:00**

a, b), because of the low rainfall intensity. From 27 April 2009 rainfalls become more intense, especially in the southeast sector of the region, where the cumulated rainfall average was about 151 mm. This event led to many landslides triggered during these days (as reported in the database). Also the probability maps show high values during these days (Fig.

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The temporal validation was also carried out considering daily cumulative rainfall compared to the landslide failure probability. In particular, a median of landslide occurrence probability was calculated for four pluviometric areas identified by Thiessen's polygons methodology, modified according to limits of river basins, both for the event of May 2008 and for the April 2009 event (Fig.

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a, b). As it could be expected, the results show that when the highest rainfall intensity is measured, the highest probability of occurrence is computed for the all areas and for both events.

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Spatial validation was performed following a pixel by pixel method: this method is the most complex since it consists in comparing the probability of instability of each pixel with the pixels involved in the actual event that occurred. This validation implies a great deal of uncertainty in the results since the reports of landslide events may have errors on the precise spatial location and on the size of the phenomenon. To overcome this problem and taking into account probable errors caused by the actual spatial location in the database, an area of 1 km<sup>2</sup> (called influence area) around the point of the landslide were considered in the validation analysis. Inside the influence area, pixels that have the 75% of probability of failure were considered instable.

shows an example of landslide event occurred in the Arnad municipality on 30 May 2008. The model computes a low failure probability on 24 May 2008 and an increase of probability on 30 May 2008. In Fig.

and b it is possible to note that inside the red circle the red and yellow area increase on 30 May with respect to 24 May. In this case, the model is able to identify correctly such movement. To better highlight this validation, Figure 10c shows the number of pixels above 75% of probability calculated by the model, within the circular area of about 1 km<sup>2</sup> around the all landslides occurred during the event of 2008. For some of the reported landslide events, the number of pixels above 75% increases on 30 May, 2008, only in case of the Champdepraz and Montjovet 2 events the probability does not increase. This may be caused by the low precision of location of the reported landslide, and maybe because some of the real landslides reported are other types of movements (rockfalls, rotational slides) that

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The final aim of the physically-based modelling for landslide prediction is to set-up an early warning system at regional scale based on the model output. The validation of the results performed in the previous section showed that the HIRESSS model performs good results with good prediction capacity both from a spatial and temporal point of view. In this work the HIRESSS model computes the daily probability of occurrence with a spatial resolution of 10 m. In order to become an active and proficient early warning system it is necessary to define a method for the interpretation of the probabilistic results (e.g., definition of probability values corresponding to alert thresholds). Furthermore, in order to have more usable results especially for public administration and civil protection authorities it is necessary to possibly aggregate the model outputs temporally and spatially.

In particular, we selected a spatial aggregation method at the municipality level. Three level of failure probabilities (low, medium and high) are defined based on the expert-judged analysis of the cumulated frequency of the municipality median values of failure probability in the most critical day of the event (e.g., highest rainfall and failure probability). This procedure was done for the two events described in Sect. 4, defining for each of them different failure probability thresholds.

Once defined the three classes of probability, each municipality was classified according to the median value of probability inside its perimeter for each day. The results for the two analysed events are shown in Fig. 11 and Fig. 12. It is worth to notice that for some municipalities with the increase of rainfall intensity there is an increase of failure probabilities values from low (green) to red (high) that can be further translated in alert levels. The validation reported in Table 3 show the number of landslides for each failure class (low, medium high). It is worth noticing that for both events the majority landslides are located in the municipalities with low and medium HIRESSS probability of occurrence.

Figure 11 and Figure 12 are examples of how the model results can be analysed but the validation results are not satisfactory. The results have to be refined and the approach should be tailored to end users needs and requirements, in particular, the following aspects should be taken into account:

spatial resolution: we have selected the municipality as spatial level of aggregation but also another types of spatial units (e.g., first or second order basins, Rossi et al., 2013) can be taken into account depending on the end-users needs and type of early warning system;

temporal resolution: in this work HIRESSS has computed daily failure probabilities. The model is coded anyway to compute FS with different temporal resolutions. In real time applications the model can produce results with different time steps (e.g., six or twelve hours);

definition of thresholds: the validation results show that the applied approach based on the analysis of cumulated median values of failure probabilities is not good enough to correctly forecast landslides. Different thresholds should be defined for each spatial unit of the early warning system based on a sound statistical analysis of HIRESSS results. To do a satisfactory analysis is necessary to have a good dataset of past triggered landslides.

<b>Pagina 10: [42] Eliminato</b>	<b>Teresa Salvatici</b>	<b>26/04/18 10:27:00</b>
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Gray, D. H., and Sotir, R. B.: Biotechnical and Soil Bioengineering Slope Stabilization, John Wiley & Sons Inc., 378, 1996.

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Intrieri, E., Gigli, G., Casagli, N., and Nadim, F.: Landslide early warning system: toolbox and general concepts. Nat. Hazard Earth Sys., 13, 85–90, 2013

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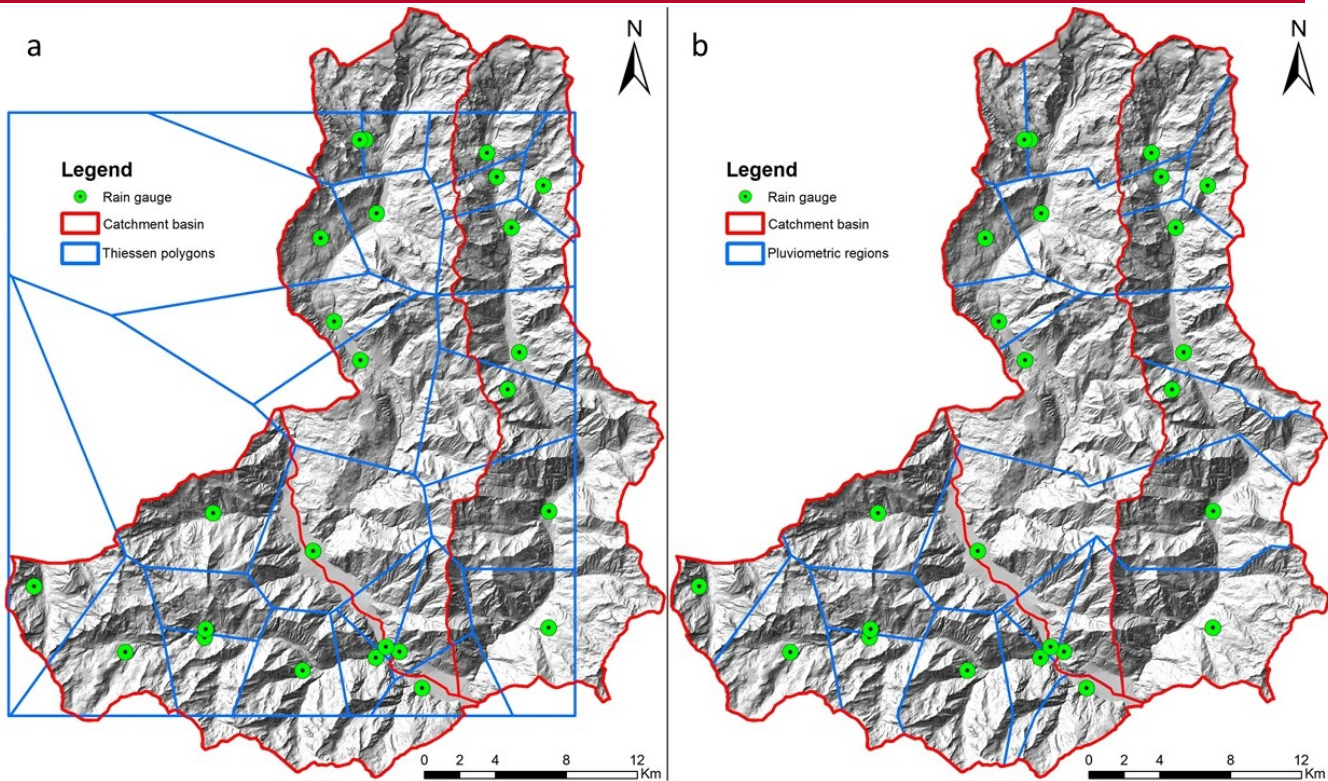


Figure 5

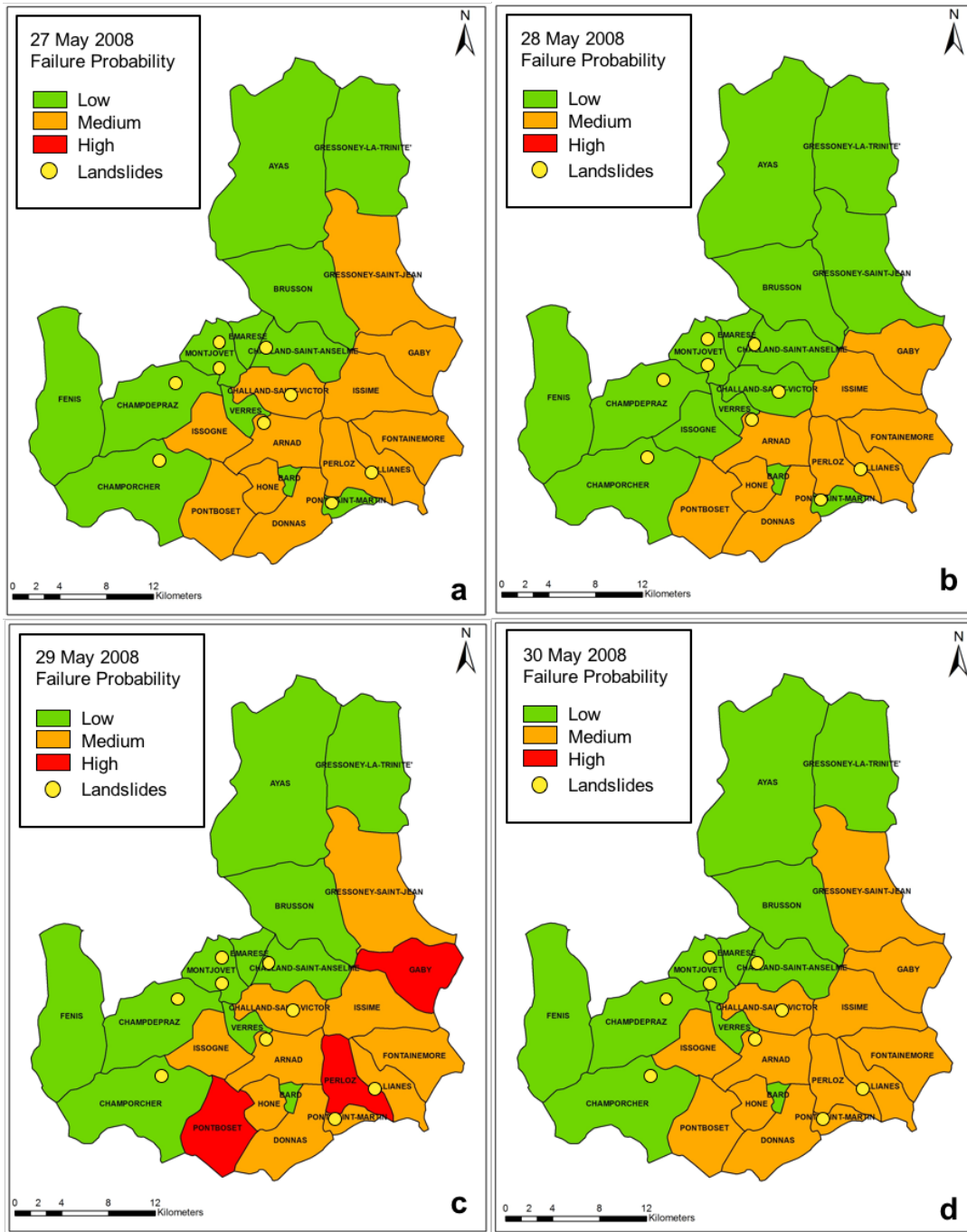
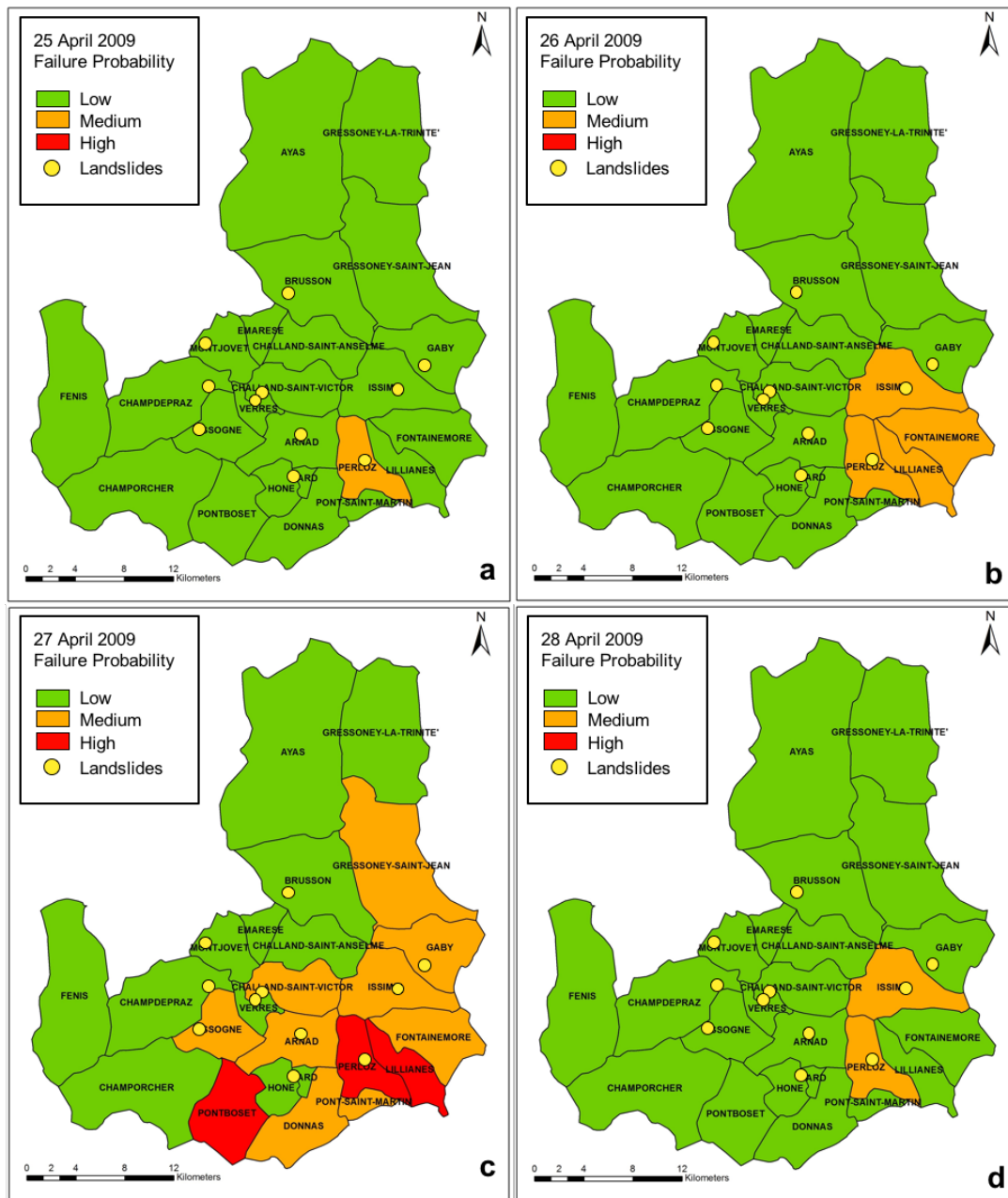


Figure 11. Spatial aggregation method at the municipality level for the events of May 2008 according to the value of failure probability.





**Figure 12.** Spatial aggregation method at the municipality level for the events of April 2009 according to the value of failure probability.