



1 Stability assessment of roadbed affected by ground 2 subsidence adjacent to urban railways

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4 **Ki-Young Eum¹, Young-Kon Park², Sang-Soo Jeon³**

5 ¹Advanced infrastructure research team, Korea Railroad Research Institute, Chuldobakmulgwan-Ro 176,
6 Uiwang City, Gyeonggi-Do 16105, South Korea

7 ²Smart station research team, Korea Railroad Research Institute, Chuldobakmulgwan-Ro 176, Uiwang
8 City, Gyeonggi-Do 16105, South Korea

9 ³Department of Civil and Urban Engineering, Inje University, Inje-Ro 197, Gimhae City,
10 Kyungsangnam-Do 50834, South Korea

11 *Correspondence to:* Sang-Soo Jeon (ssj@inje.ac.kr)

12
13 **Abstract.** In recent years, leakages in aged pipelines for water and sewage in urban areas have frequently induced
14 ground loss resulting in cavities. One third of the pipelines buried in Seoul city in South Korea are more than fifty
15 years old. Train loadings and change in groundwater levels in the undiscerned development of urban areas induce
16 roadbed settlements. Train derailment may occur as the roadbed exceeds the allowable settlements associated with
17 location and size of the cavity adjacent to the roadbed. In this study, FLAC3D, which is a three-dimensional finite-
18 difference numerical modeling software, is used to do stability and risk level assessment for the roadbed in
19 adjacent to urban railways with respect to various groundwater levels and the geometric characteristics of cavities.
20 Numerical results show that the roadbed settlements in simulated ground conditions in South Korea, that satisfy the
21 allowable values for a cavity of diameter of 10 m exists adjacent to the roadbed. The distance between the center of
22 the roadbed and the center of the cavity should be greater than 25 m and the groundwater level should be greater
23 than 22 m below the ground surface.

24 25 **1 Introduction**

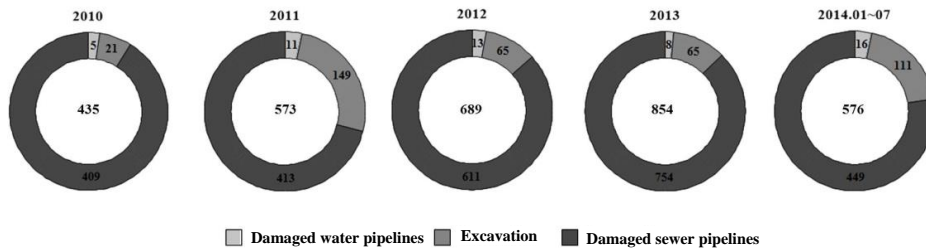
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27 Urban railways in South Korea have been initiated from the Seoul subway 1st line in 1974 and have been operating
28 in Seoul city and several metropolitan cities. The number of passengers using urban railway are being increased
29 and it has played a significantly important role in public transportation for urban development. Urban railway is
30 defined as transportation facility and method for smooth transportation in the city and includes light rail transit and
31 subway as indicated in the law of urban railway (Ministry of land, 2017). As roadbed settlements exceed the
32 allowable limits, it may result in track irregularity and derailments of trains causing heavy loss of life. Therefore,
33 methods to secure the stability of roadbeds have been examined.

34 Research on stability assessment and reinforcement of railway roadbeds has been actively carried out, but the
35 effect of the cavity adjacent to urban railways on roadbed behavior has rarely studied. In recent years, the number
36 of accidents induced by cavities larger than 2-m in diameter has increased especially in highly populated cities in
37 South Korea. Therefore, the residents in these cities were terrified of cavities after the accidents (Shin and Roh,
38 2006). Especially, ground subsidence near subways due to self-weight and/or surcharge loading was around 60%
39 (Lee and Kang, 2014). Changes in groundwater levels may cause increased occurrences of ground subsidence
40 because the lowering of groundwater levels lead to ground settlement (Lee et al., 2015). Groundwater level
41 influences both ground settlement and stability of underground structures. Deep excavation of the ground adjacent
42 to urban railways has a significance influence on the allowable tensile strength of underground structures (Lee at
43 al., 2017). If large underground cavities are located at nearby roadbeds, there is a high potential of ground
44 subsidence.

45 80% of the ground subsidence occurred from 2010 until the beginning of 2014 in Seoul City (Fig. 1) was
46 induced by aged pipelines for water and sewage (Oh et al., 2015). Since 48% and 30% of sewage pipelines in Seoul
47 city were constructed more than thirty and fifty years ago, respectively. Aged pipelines for water and sewage
48 pipelines cause numerous cavities in the near future (The Segye times, 2016).



49 As a cavity exists at the center of the railway track in the box structures of urban railways, its influence on box
 50 structures and roadbed settlements has been examined to observe the effects of cavities adjacent to the roadbeds of
 51 urban railways (Lee et al., 2015). A method to establish a data-base was proposed to prevent and manage the
 52 disasters (Choi et al., 2007).
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Figure 1. Frequency and cause of ground subsidence in urban areas

57 As a cavity exists adjacent to the roadbed, in this study, a three-dimensional numerical analysis is carried out to
 58 assess both roadbed stability and risk level with respect to the distance between the center of the roadbed and the
 59 center of the cavity, diameter of the cavity, and groundwater levels.
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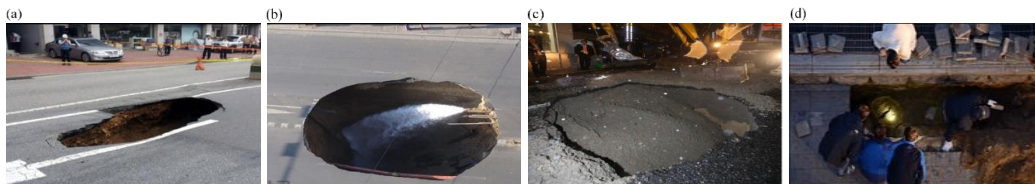
61 2 Case studies of ground subsidence

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65 2.1 Ground subsidence in South Korea

66 The ground subsidence (Fig. 2) occurred at nearby urban railways most recently (Kyunghang times, 2016). The
 67 ground subsidence (Fig. 2a) occurred with a cavity of depth 5 m, width 8 m, and length 80 m near the Seokchon
 68 subway station in Seoul City. The accident was induced by the inappropriate deep excavation near the subway.
 69 The ground subsidence (Fig. 2b) was caused by the leakage of a water pipeline with a large-scale cavity of depth 21 m,
 70 width 11 m, and length 12 m near Bakchon subway station in Incheon City (Newshankuk, 2016). The ground
 71 subsidence (Fig. 2c) occurred near Samseongjungang subway station. Six cavities were found almost simultaneously
 72 in Seoul City (Kyunghang times, 2016). A small-scale cavity of depth 2.2 m (Fig. 2d) occurred near Janghanpyeong
 73 subway station in Seoul City, but the cause of this accident has not been clarified. The accident was assumed to be
 74 caused by inappropriate recovery construction near subway extension. Ground subsidence, as described above,
 75 occurred at nearby urban railways.

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76 **Figure 2.** Ground subsidence nearby subway of urban railway in South Korea. Seokchon subway station (a). Bakchon subway
 77 station (b). Samseongjungang subway station (c). Janghanpyeong subway station (d).

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81 2.2 Representative ground subsidence in the US, Canada, Japan, and China

82 Ground subsidence (Fig. 3) has occurred at nearby urban railways most recently [10]. Ground subsidence with a
 83 cavity having depth 3.6 m (Fig. 3a) occurred as the replacement work of a sewage pipeline was carried out at Texas
 84 in the US (Wikitree, 2016). Ground subsidence with a cavity having a width of 15 m (Fig. 3b) occurred as tunnel
 excavation work for subway extension was carried out at Fukuoka in Japan (Chosun Ilbo, 2016). Ground



85 subsidence with a cavity of width 25 m (Fig. 3c) occurred as a 50-m tunnel excavation near the light rail transit was
 86 carried out at Ottawa in Canada (Yonhap news, 2016). Ground subsidence with a cavity of depth 10 m (Fig. 3d)
 87 occurred as subway construction was carried out near Guangzhou in China (Sisa china, 2016). Ground subsidence
 88 with a large-scale cavity in urban areas is highly correlated with the undiscerned development of urban areas, abuse
 89 of groundwater and inappropriate underground construction.
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91 **Figure 3.** Ground subsidence nearby subway of urban railway in the US, Canada, Japan, and China. Texas in the US (a).
 92 Fukuoka in Japan (b). Ottawa in Canada (c). Guangzhou in China (d).

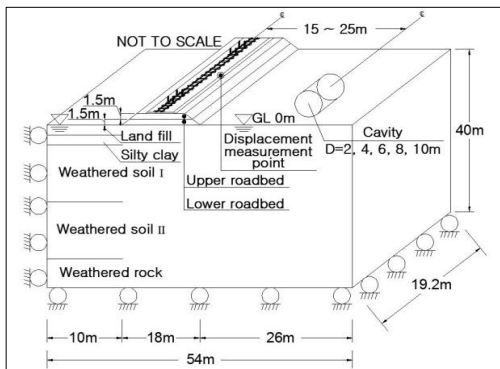
93 **3 Numerical analysis**

94 **3.1 Conditions for numerical analysis**

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 98 In this study, FLAC3D, which is a three-dimensional finite-difference numerical code, is used to conduct stability
 99 and risk level assessment for roadbeds associated with various groundwater levels and the geometric characteristics
 100 of cavities adjacent to urban railways. The Mohr-Coulomb failure model has been used for the analysis (Itasca
 101 Consulting Group Inc., 2015). Since there are various causes and sizes of the cavity of ground subsidence occurring
 102 near urban railway, it is very difficult to simulate the process of cavity generation. A circular cavity below the
 103 ground surface has been modelled with respect to diameters (D) of 4-10 m at distances of 15-25 m from the cavity
 104 to the center of the roadbed associated with various groundwater levels. The analysis is performed based on the
 105 configuration and numerical model of the analysis, respectively (Figs. 4 and 5).
 106

107 **3.1.1 Ground conditions**

108 An embankment consists of the lower roadbed, upper roadbed, and gravel ballast. The roadbed width at the
 109 bottom of the ballast is 8m. The widths of its bottom and top are 5.1 m and 3.3 m, respectively, and its slope is
 110 1:1.8. In-situ soil consists of reclaimed soil, silty clay, weathered soil, and weathered rock. Its physical properties
 111 listed in Table 1 are obtained from lab experiments of soil sampled at a construction site.
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 114 **Figure 4.** Configuration of the railway roadbed and cavity
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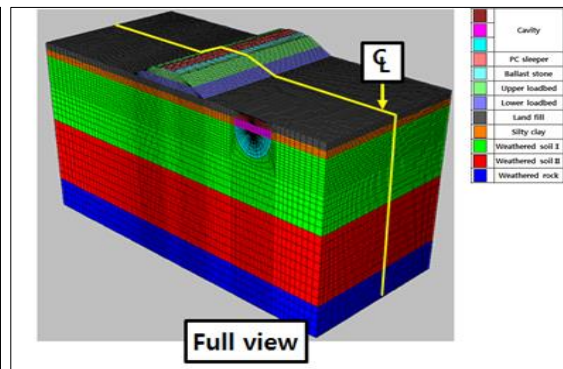


Figure 5. Three-dimensional view of the numerical model



117 **3.1.2 Physical properties of rail, rail pad, and prestressed concrete (PC) sleeper**

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KS60 rail and prestressed concrete (PC) sleeper commonly used in gravel ballast have been used for the numerical analysis. A rail pad, which is widely used to minimize vibration and impact loading during train operation is made of ethylene vinyl acetate (EVA). However, in this study, a thermoplastic polyurethane (TPU) rail pad, which is more economical and has higher tensile strength has been used for the numerical analysis. Its properties are listed in Tables 2, 3, and 4. The beam element is used for the rail and rail pad.

125 **Table 1.** Physical properties of soil
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	Height (m)	Unit Weight (kN/m ³)	Elastic modulus (kPa)	Poisson's ratio (ν)	Cohesion (kPa)	Friction angle (°)	Coefficient of permeability (cm/s)	K _o
Ballast stone	0.3	19.0	133,900	0.30	-	35	-	0.43
Upper roadbed	1.5	18.0	81,600	0.20	3.0	32	-	0.47
Lower roadbed	1.5	18.0	51,000	0.30	10.0	30	-	0.50
Land fill	1.5	17.0	30,000	0.35	5.0	24	1.0×10 ⁻³	0.59
Silty clay	1.5	17.0	20,000	0.35	5.0	25	5.0×10 ⁻⁴	0.58
Weathered soil I	15.0	19.0	75,000	0.33	10.0	30	1.0×10 ⁻⁴	0.50
Weathered soil II	15.0	19.0	70,000	0.33	10.0	33	1.0×10 ⁻⁴	0.46
Weathered rock	7.0	20.0	110,000	0.31	60.0	42	1.0×10 ⁻⁵	0.33

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Table 2. Physical properties of the rail

	Area (mm ²)	Unit weight (kN/m ³)	Elastic modulus (kPa)	Moment of inertia(m ⁴)	
				I _{xx}	I _{yy}
KS60 rail	7,741	77.5	21,000×10 ⁴	30,820×10 ⁻⁹	5,120×10 ⁻⁹

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Table 3. Physical properties of the PC sleeper

	Length (m)	Width (m)	Height (m)	Interval between sleepers (m)
PC sleeper	2.45	0.28	0.20	0.58

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Table 4. Physical properties of the rail pad

	Thickness (mm)	Unit weight (kN/m ³)	Vertical spring coefficient of rail pad (kPa)
Rail pad	5	11.5	15.3×10 ⁷

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141 **3.1.3 Applied train loading**

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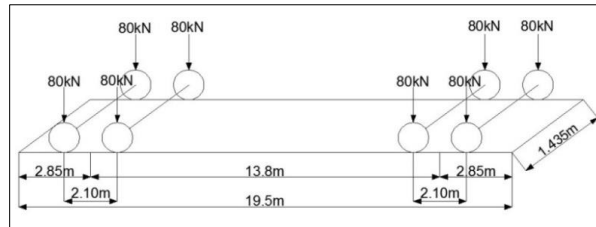
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An axial load of the urban railway train (16 tons) is applied for the numerical analysis. The effective loading is estimated by multiplying 1.2 with half of the axial load considering a wheel loading increment of 20% and a marginal safety of deficiency of the cant. Dynamic loading to reflect dynamic impact ratio (Fig. 6) was estimated by multiplying 1.2 with the effective loading (Ministry of land, 2013).



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3.2 Allowable settlement of the roadbed

In general, an allowable settlement of 10 mm has been recommended in South Korea. The vibratory loading induces the gravel to be in a loose state, and frequent repairs of ballasts are required. Therefore, an allowable settlement of 2.5 mm is used to attain additional marginal safety considering the compressive displacement of both the rail pad and ballasts, settlement of rail, ride quality, and both water inflow and cracks in the pavement surface of roadbeds (Jeon, 2014).

4 Roadbed Settlement and Stability

4.1 Roadbed settlement

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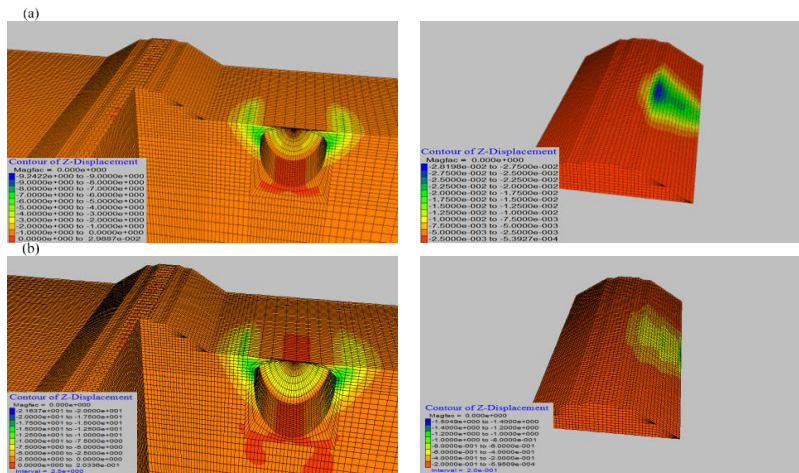
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The contours of ground settlement are presented for how the roadbed (Fig. 7) is influenced by a cavity adjacent to the urban railways. The contours of ground settlement are presented for cavities with diameters of 8 m and 10 m, respectively, at a distance of 20 m between the center of the roadbed and the center of the cavity. As shown in the figures, ground settlement increases as the diameter of the cavity increases. As a cavity is generated on the right side of the roadbed, the right end of the roadbed is significantly settled down.



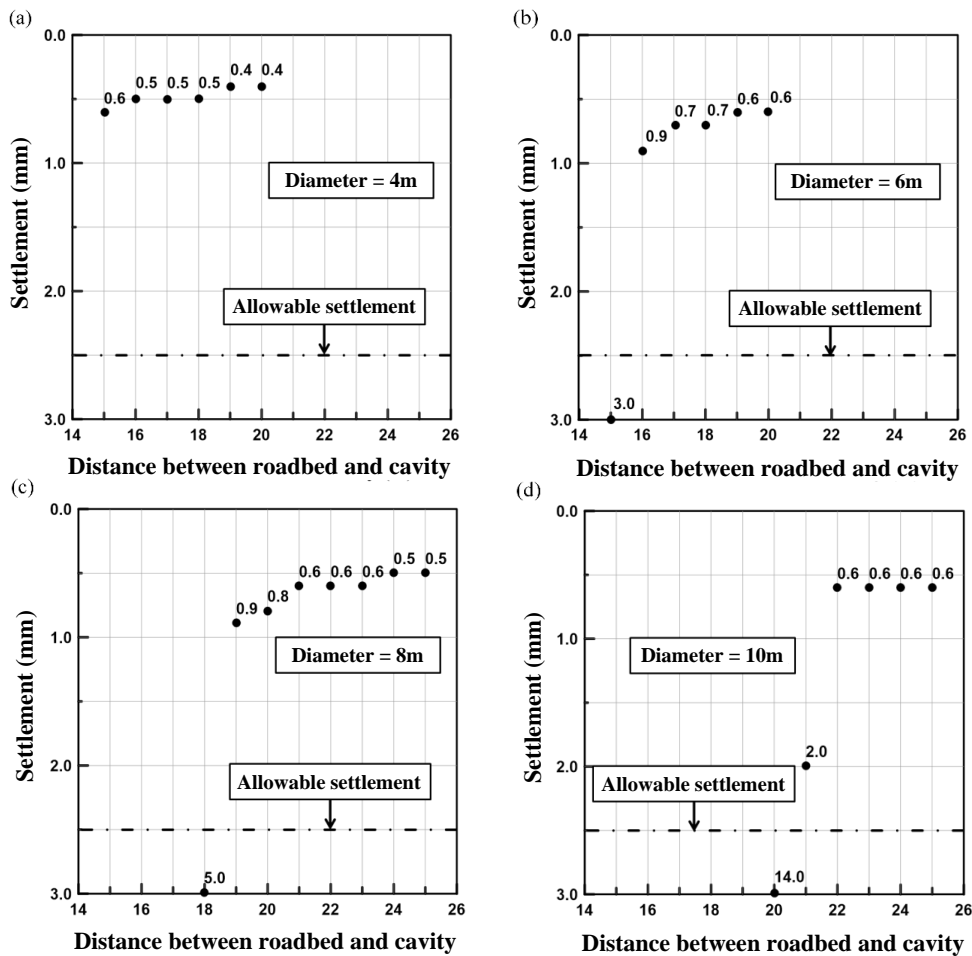
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Figure 7. Vertical displacement contour of the roadbed at a distance of 20 m between the center of roadbed to the center of the cavity=20 m with respect to diameter of the cavity. Diameter = 8 m (a) and Diameter = 10 m (b)



169 The analysis results (Fig. 8) are presented for cavities with diameters of 4-10 m. As the variation from 15 to 20
 170 m in the distances between the center of the roadbed and the center of the cavity is applied to the 10-m cavity,
 171 roadbed settlements are calculated with respect to various diameters of the cavity. The cavity with a diameter of 10
 172 m at a distance of 20 m has little influenced on the roadbed. However, as the diameter of the cavity at the same
 173 distance exceeds 10m, the roadbed settlement exceeds the allowable value. As cavities with diameters of 8 m and 6
 174 m are generated, at distances less than 18 m and 15 m, it exceeds the allowable settlement and may result in an
 175 accident
 176



177 **Figure 8.** Roadbed settlement with respect to distance between roadbed and cavity. Diameter = 4 m (a). Diameter = 6 m (b).
 178 Diameter = 8 m (c). Diameter = 10 m (d).
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180 **4.1.1 Regression analysis of roadbed settlement**

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 182 The regression analysis results (Fig. 9) are presented for the roadbed settlement with respect to the distance (d)
 183 between the roadbed and the cavity and diameter (D) of the cavity shows high correlations of $r^2=0.72$ and
 184 substantial increment as D/d exceeds 0.35.



185 **4.2 Effects of groundwater level**

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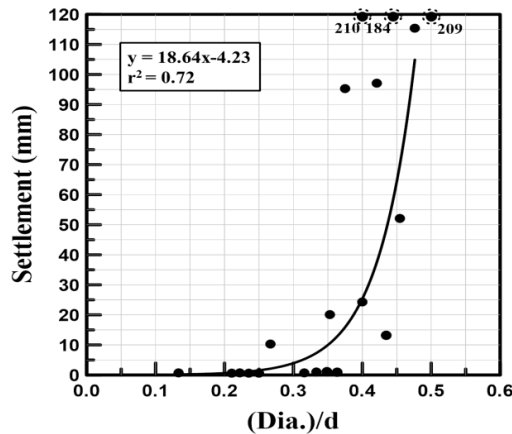
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In this study, the effects of groundwater level on the roadbed settlement are examined and it is lowered until the allowable settlement value of the roadbed is satisfied. The maximum distance between the roadbed and the cavity for the analysis is determined as the maximum value for the satisfied allowable settlement with no groundwater condition. A stability assessment of the roadbed has been carried out at the distance of 20 m for both 4-m and 6-m diameter cavities and at 25 m for both the 8-m and 10-m diameter cavities.

The contours of ground settlement (Fig. 10) are presented to examine the groundwater level (GWL) effects in the case of the 8-m diameter cavity located at a distance of 25 m from the roadbed to cavity. The contours of ground settlement are presented with GWL on the ground surface and 20 m below it, respectively (Figs. 10a and 10b). The settlement of the roadbed is highly subject to groundwater levels.



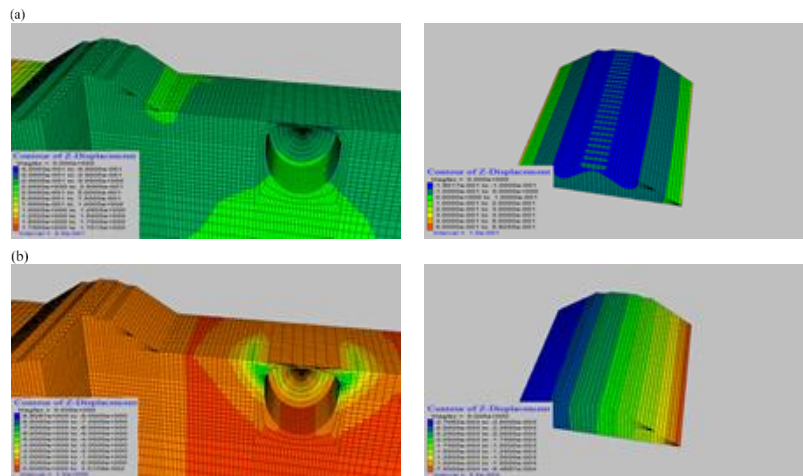
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Figure 9. Regression analysis of roadbed settlements with respect to the diameter of the cavity and distance between roadbed and the cavity



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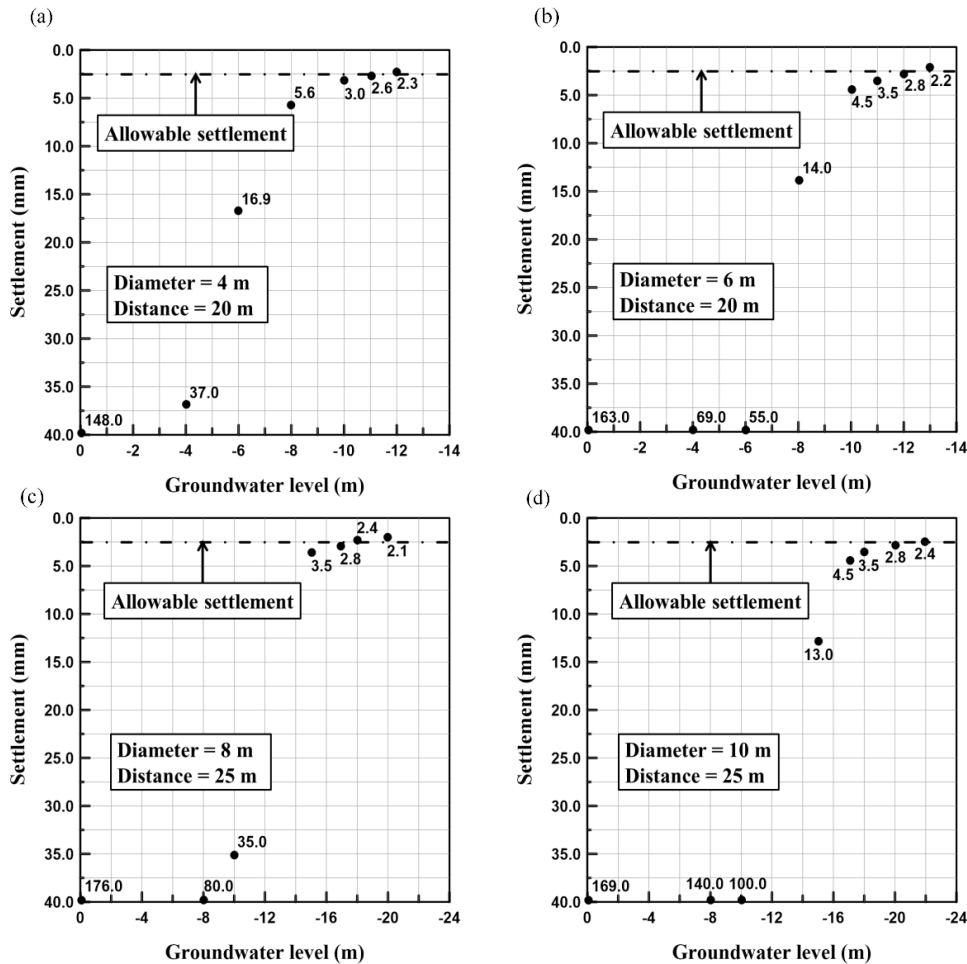
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Figure 10. Vertical displacement contours of the roadbed for a cavity diameter 8 m, at the roadbed-to-cavity distance of 25 m with respect to GWL = ground surface (a). GWL = (-)20 m (b).



205 The roadbed settlement (Fig. 11) is highly influenced by groundwater. Ground settlement for 4-m and 6-m
 206 diameter cavities located at a distance of 20 m from the roadbed (Figs. 11a and 11b) satisfies the allowable value
 207 for $GWL=(-) 4m$ and $(-) 12m$, respectively. The ground settlement for 8-m and 10-m diameter cavities located at a
 208 distance of 25 m from the center of the roadbed (Figs. 11c and 11d) has substantially decreased as groundwater
 209 level is 8 m and 15 m below the ground surface, respectively, and satisfies the allowable value as its level is 18m
 210 and 22m below the ground surface, respectively. Therefore, a roadbed settlement is highly influenced by
 211 groundwater levels to an extent greater than even the influence of the size of the cavity.
 212



213 **Figure 11.** Roadbed settlement with respect to the groundwater level. Diameter of the cavity = 4 m and distance of roadbed from
 214 the center of the cavity = 20 m (a). Diameter of the cavity = 6 m and distance of the roadbed from the center of cavity =20 m (b).
 215 Diameter of the cavity = 8 m and distance of the roadbed from the center of cavity = 25 m (c). Diameter of the cavity = 10 m and
 216 the distance of the roadbed from the center of the cavity = 25 m (d).
 217

218 4.3 Risk level assessment of roadbed

219
 220 Roadbed settlements induced by the cavity near urban railways have been estimated with respect to the
 221 groundwater level, distance between the roadbed and cavity, and size of the cavity. The risk level has been
 222 estimated by the occurrence of roadbed settlements. Its risk level has been defined by the value of the roadbed



223 settlements relative to the allowable settlement. The risk level is defined as safe (not problematic for both ride
 224 quality and track repair), caution (not problematic for track repair), warning (between caution and danger), and
 225 danger (highly probable traffic accident) as a settlement is equal to or less than 2.5 mm, greater than 2.5 mm and
 226 equal to or less than 4 mm, greater than 4 mm and equal to or less than 9mm, and greater than 9 mm, respectively
 227 As listed in Table 5, the roadbed settlement increases as the size of the cavity increases and the cavity is located
 228 close to the roadbed. As listed in Tables 6 and 7, the roadbed settlement for the groundwater condition is less than
 229 the allowable value, whereas it is in extreme danger when groundwater is present. When it is in the status of danger,
 230 train operation should be stopped and the roadbed should be reinforced or repaired. A database of measurement
 231 sensors for urban railways should be established for real-time monitoring of the roadbed, structures and
 232 groundwater for disaster prevention.
 233

234 **Table 5.** Risk level of the roadbed with respect to the diameter of the cavity and the distance between the roadbed to the cavity
 235 for the groundwater condition
 236

		Distance (m)										
		25	24	23	22	21	20	19	18	17	16	15
Diameter (m)	10	Case B										
	8									Danger		
	6			Safety								
	4							Case A				

※ Safety(Settlement≤2.5mm), Danger(9.0mm<Settlement)

237
 238 **Table 6.** Risk level of the roadbed with respect to the diameter of the cavity and groundwater levels at a distance of
 239 20-m between the roadbed and cavity for case A
 240

		Groundwater Level (m)							Ground surface
		-13	-12	-11	-10	-8	-6	-4	
Diameter of cavity (m)	4	Safety		Caution		Warning		Danger	
	6								

※ Safety(Settlement≤2.5mm), Caution(2.5mm<Settlement≤4.0mm), Warning(4.0mm<Settlement≤9.0mm), Danger(9.0mm<Settlement)

241
 242 **Table 7.** Risk level of the roadbed with respect to the groundwater levels at a distance of 25 m between the roadbed and
 243 cavity for case B
 244

		Groundwater Level (m)							Ground surface
		-22	-20	-18	-17	-15	-10	-8	
Diameter of cavity (m)	8	Safety						Danger	
	10		Caution		Warning				

※ Safety(Settlement≤2.5mm), Caution(2.5mm<Settlement≤4.0mm), Warning(4.0mm<Settlement≤9.0mm), Danger(9.0mm<Settlement)

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246 5 Conclusions

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The number of occurrences of ground subsidence induced by a leakage of aged pipelines for water and sewage in urban areas resulting in various sizes of cavity near the urban railway in Seoul City has been found to increase and it may cause the roadbed settlement to exceed the allowable value. A large-scale cavity is rarely found, but if it is close to the roadbed, the roadbed is highly influenced by the cavity and may cause train derailment.

In this study, a three-dimensional numerical analysis is carried out to estimate roadbed stability and its risk level associated with various groundwater levels, sizes of cavities, in simulated ground conditions and tracks in Daejeon urban railways in South Korea.

Future work should focus on coupled hydro-mechanical analysis for groundwater flow and a real-time monitoring system of measurement sensors installed on the railway ground and structures with groundwater to prevent disasters in advance.

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