

BASIC FEATURES OF THE PREDICTIVE TOOLS OF EARLY WARNING SYSTEMS FOR WATER-RELATED NATURAL HAZARDS: EXAMPLES FOR SHALLOW LANDSLIDES

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2 **ABSTRACT** To manage natural risks, an increasing effort is being put in the development
3 of early warning systems (EWS), namely, approaches facing catastrophic phenomena
4 by timely forecasting and alarm spreading throughout exposed population. Research
5 efforts aimed at the development and implementation of effective EWS should
6 especially concern the definition and calibration of the interpretative model. This paper
7 analyses the main features characterizing predictive models working in early warning
8 systems, by discussing their aims and, consistently, their features in terms of model
9 accuracy, evolution stage of the phenomenon at which the prediction is carried out,
10 and model architecture. Original classification criteria based on these features are
11 developed throughout the paper and shown in their practical implementation through
12 examples referred to flow-like landslides and earth flows, both characterized by rapid
13 evolution and quite representative of many applications of EWS.

14

1. Introduction

Different natural hazards turning into catastrophes have occurred widespread in Italy in the recent past as well as in the last centuries. Seismic and volcanic phenomena have affected sporadically large areas, while rainfall-induced landslides, floods and snow avalanches have frequently hit sites spread all over the territory. Structural mitigation approaches are inapplicable throughout the entire territory at risk and might be planned only for areas relevant from a socio-economic point of view.

Hence, to manage natural risks, an increasing effort is being put in the development of non-structural approaches, based on timely forecasting the catastrophic phenomena from precursors or indicators, so to early spread the alarm throughout the exposed areas (early warning) and temporarily eliminate or, at least, reduce the exposure of people, preventing or limiting victims (Basher, 2006). The increasing importance of Early Warning Systems (EWS) is testified by the fact that they are among the priorities adopted by the United Nations, International Strategy for Disaster Reduction (ISDR) (UN-ISDR, 2005; 2006).

EWS indeed present undeniable advantages, among which are their fast, simple and low-cost implementation, and environmental friendliness. Focusing on water-related hazards, significant examples of operational EWS are currently found in the field of floods, landslides, snow avalanches, earth fill failures. A recent review of EWS operating in Europe for water-related hazards can be found in Alfieri et al. (2012).

As it will be described in detail hereinafter, the architecture of an EWS is strictly related to the time needed for the deployment of the mitigation measures, compared to the time of evolution of the hazardous event. In this respect, EWS for floods present quite different features if they are established along large or small rivers. In the first case, rainfall measurements or predictions are supplemented with river stage measurements in upstream sections (e.g., Rabuffetti and Barbero, 2005), and flood routing models can be run in cascade of hydrological models (e.g., Cranston and Tavendale, 2012). The lead time of prediction, which depends on the length of the river and on the extension of its catchment, can extend up to several days or weeks. In the case of small streams, the time lapse between rainfall and peak discharge may be so short that weather nowcasting is needed for the warning to be launched in due time (e.g., Alfieri and Thielen, 2015; de Saint-Aubin et al., 2016).

So far, most of the EWS dealing with rainfall-induced landslides are based on rainfall measurements, sometimes supported by weather forecasts (e.g., Keefer et al., 1987; Ponziani et al., 2012), rarely integrated with monitoring of some soil variables (e.g., Ortigao and Justi, 2004; Chleborad et al., 2008; Baum and Godt, 2010). Rainfall are

1 interpreted often merely statistically, with an empirical quantification of rainfall
2 thresholds for landslide initiation (e.g., Sirangelo and Versace, 1996; Sirangelo and
3 Braca, 2004; Guzzetti et al., 2007, 2008; Capparelli and Tiranti, 2010; Tiranti and
4 Rabuffetti, 2010; Martelloni et al., 2012; Segoni et al., 2014; Tiranti et al., 2014; Piciullo
5 et al., 2016). In rare cases, physically based approaches are adopted for the
6 interpretation of the effects of rainfall history. The few examples of inclusion of slope
7 infiltration and stability modelling in the assessment of the safety conditions are mostly
8 still at a prototypal stage (e.g., Schmidt et al., 2008; Capparelli and Versace, 2011;
9 Ponziani et al., 2012; Eichenberger et al., 2013; Pumo et al., 2016).

10 EWS operating for snow avalanches monitor snow accumulation and the melting
11 processes, with the former basing essentially on interpreting precipitation and air
12 temperature records, and the latter on air (or snow) temperature (e.g. Liu et al., 2009).

13 Even in the field of man-made systems, early warning is assuming a prominent role in
14 the assessment of the risk associated with failure. For instance, in the field of earth
15 dams, with regard to all possible collapse mechanisms, i.e. slope instability and internal
16 erosion phenomena, or even earthquake-induced effects, risk mitigation is de-facto
17 based on EWS (e.g., Pagano and Sica, 2013; Ma and Chi, 2016). The wide monitoring
18 system commonly installed to characterize time-by-time the behavior of these
19 structures, carried out essentially in terms of displacements, pore water pressure,
20 seepage flows, and accelerations, is pointed towards a continuous checking of dam
21 safety conditions, aimed at evacuating downstream settlements in case of predicted
22 collapse.

23 Literature indicates that common elements, which typically characterize an EWS (e.g.,
24 Intrieri et al., 2012; 2013; Calvello and Piciullo, 2016), are:

- 25 1. *a field monitoring system*, recording physical quantities related to the phenomenon
26 in hand, and transmitting them to a collection-elaboration center; measured
27 variables may conveniently be distinguished into two categories: *cause variables*,
28 leading to the initiation of the phenomenon; *effect variables* that, affected by the
29 formers, characterize the phenomenon itself during its evolution and at its
30 triggering, allowing also to recognize its intensity;
- 31 2. *a predictive model*, formalizing mathematically the relationships linking cause and
32 effect variables, allowing to catch the evolution stage of the phenomenon and
33 assess system safety conditions;
- 34 3. *thresholds* for the variables related to safety conditions of the system; these
35 thresholds correspond to different alert levels, with the highest one activating the
36 spread of the alarm message, aimed at eliminating people exposure;

1 4. different *actions* related to each alert level defined at 3.

2 Research efforts aimed at the development and implementation of effective EWS
3 should concern, above all, the definition, calibration and validation of the predictive
4 model (Michoud et al., 2013). It should be as accurate as possible and, at the same time,
5 capable of rapidly carrying out the turning of the monitored quantities into the
6 assessment of system safety conditions. In many applications, dealing with rapidly
7 evolving natural hazards, a real-time working system is in fact required, in order to
8 maximize the lead time available to reduce/eliminate people exposure to the hazard.

9 Aim of the paper is to address the main features of predictive models for water-related
10 natural hazards. The proposed frame is quite general and applicable to other types of
11 natural hazards, thus references will be briefly made throughout the paper also to
12 applications different from water-related hazards. In particular, based on the precise
13 definition of the aims of the EWS, this work addresses the importance of identifying the
14 evolution stage of the catastrophic event at which the prediction should be
15 implemented, so to maximize its effectiveness. For the first time the evolution stage at
16 which the predictive model is implemented is considered as one of its features, along
17 with the other traditional approach distinguishing between empirical or physically-
18 based models.

19 In principle, any predictive model might be referred to any spatial scale, which is thus
20 not considered as a valid classification element for EWS models. Rather, the
21 classification criteria proposed throughout the paper may be referred to all scales. The
22 choice to show specific examples all referred to rainfall-induced landslides at a slope
23 scale is not performed in the light to reduce generality to the proposed criteria but,
24 rather, in the attempt to select an application field which representativeness poses
25 challenges extendible to other natural phenomena.

26 27 **2. Prediction uncertainty and the minimization of the costs of missing and false** 28 **alarms of an EWS**

29 Whatever the predictive model adopted, it will never be capable of providing certainty
30 about the occurrence of a catastrophic event. A model yields variables systematically
31 affected by a given uncertainty degree due to the following possible causes:

- 32 - incompleteness of information about the physical system supposed to cause
33 catastrophes;
- 34 - various error types associated with the measurements provided by the monitoring
35 system;

- 1 - unavoidable simplifications of reality always introduced in building the predictive
- 2 model;
- 3 - randomness of some of the processes involved in the genesis of the catastrophic
- 4 event.

5 It is obvious that the uncertainties of the predicted variables related to the physical
6 system affect the assumption of different alert stages. With reference to the last stage,
7 it may occur that the EWS issues an alarm, but no dangerous phenomenon occurs (false
8 alarm) or, conversely, that a dangerous phenomenon takes place without any issued
9 alarm (missing alarm). Both false and missing alarms are costly to the community served
10 by the EWS. A lower uncertainty degree in the prediction is required to minimize their
11 number and, consequently, costs during the system operation. Efficiency of the EWS is
12 therefore considered with respect to its economic value for the community, rather than
13 merely to the provided safety performance. In this sense, alarm activation has to
14 account for the uncertainties associated with each alert threshold and its overcoming,
15 so to minimize false and missing alarms and related costs.

16 Decisional rules regarding actions associated with each alert threshold should be based
17 not only on the mere quantification of thresholds themselves, but also on criteria
18 defining the *sensitivity* of the EWS, intended as setting the activation of the system at
19 some probability of a given threshold to be exceeded.

20 The most suitable strategy to quantify such probability of threshold exceedance cannot
21 be generalized. It is in fact strongly affected by the following peculiarities characterizing
22 the EWS in hand:

- 23 - the uncertainty of the prediction, which may be reduced by increasing the initial
- 24 investment (by preliminary acquiring more information about physical system
- 25 features, implementing a more reliable monitoring system with higher spatial and
- 26 temporal resolution, elaborating a more sophisticated and accurate predictive
- 27 model);
- 28 - the costs suffered by the community in case of false alarms, in turn depending also
- 29 on the kind of actions planned in case of threshold exceedance;
- 30 - the costs resulting from a missing alarm, depending on both the event (type and
- 31 intensity) and resilience of the exposed goods (related to their nature as well as to
- 32 socio-economic aspects).

33 In setting up the EWS sensitivity, it should be taken into account that too many false
34 alarms would discredit the system, implying that, over time, the served community
35 would contribute less in carrying out all the required actions after alerts. In short, the
36 sensitivity has to be calibrated on the basis of a cost-benefit analysis, which can be

properly carried out only if the uncertainty of model predictions can be estimated after an adequate period of monitoring of the physical system.

3. Evolution stages of a natural hazard: when should the model do the prediction?

In order to generalize a typical architecture for the predictive model, it comes useful to account for a conventional sequence of stages describing the evolution of a natural phenomenon resulting into a catastrophe (Fig. 1):

- (a) the predisposing stage: the cause variables are subject to such changes to induce significant modifications of effect variables;
- (b) the triggering and propagation stage: the failure occurs locally (triggering time) and propagates from point to point throughout the physical system up to involve it entirely;
- (c) the paroxysmal stage: the physical system collapses and the kinematics of the system goes on, eventually hitting the exposed goods.

The duration of each stage may greatly vary, depending on both the kind of phenomenon and on the features of the physical system involved.

In an earthquake hitting structures located at a given site “S”, the *predisposing stage* (a) is determined by the occurrence of the seismic event at the epicenter and is indicated by the first arrival of the seismic waves at the seismometers nearest to the epicenter. The *triggering and propagation stage* (b) is determined by acceleration values exceeding the threshold for first local damages to structural elements and is monitored by seismic stations located at “S”; the *paroxysmal stage* (c) consists of the collapse of parts of the structures. For this specific example, the duration of stages (a) and (b) is few tens of seconds, while the duration of stage (c) depends on the system considered, spanning from seconds for systems like buildings, rock slopes, gas conduits etc., until hours or even days for natural earth slopes, dams, and, in general, systems which collapse is determined by a slow redistribution or propagation of earthquake-induced effects.

In a rainfall-induced landslide, the *predisposing stage* (a) is determined by the sequence of rainfall events and by the hydrological processes leading to increase of pore water pressure and worsening slope stability conditions (e.g., Bogaard and Greco, 2015). The *triggering and propagation stage* (b) spans from the first local slope failure until the formation of a slip surface. The *paroxysmal stage* (c) is the sliding of the mobilized soil mass downhill along the slip surface. In this second example, the duration of each stage

1 is strongly related to the geomorphology of the specific slope and to the type of
2 landslide (Varnes, 1978), and may vary from minutes (e.g., flow slides in slopes covered
3 with shallow coarse grained soils) to even years (e.g., some earth flows in slopes of fine
4 grained soils).

5 In a snow-avalanche, the *predisposing stage* (a) is determined by snow accumulation
6 and temperature increments; the *triggering and propagation stage* (b) starts when
7 local failures take place within the snow aggregate and ends with a slip surface
8 formation. The *paroxysmal stage* (c) starts when the mass slides downhill. In this
9 example, the duration of stage (a) may be of hours or days, depending on the evolution
10 of atmospheric variables, the duration of stage (b) results undetectable, and the
11 paroxysmal stage lasts only few seconds.

12 For the case of an overflow in a river, the *predisposing stage* (a) is a sequence of
13 precipitation events within the watershed, causing a progressive increase of the water
14 level along the river course; in this case, the *triggering and propagation stage* (b) and
15 the *paroxysmal stage* (c) are hardly distinguishable from each other. In fact, both stages
16 start when the first local overflow takes place, and both develop with the flood
17 propagating around the river. The stage duration depends on the extension and
18 geomorphology of the watershed. The entire phenomenon may last tens of minutes
19 (e.g., flash floods in small streams with relatively small catchment) to several days (e.g.,
20 large rivers with large watershed).

21 It is also important to highlight that for most phenomena the triggering event has to be
22 considered as random and, as such, time and location of its occurrence can be predicted
23 only with a probabilistic approach. On the other hand, the predisposing stage can be
24 usually described with physical laws, so that its spatial and temporal evolution can be
25 predicted deterministically by mathematical models.

26 For instance, the strategies followed for early warning with respect to snow avalanches
27 (e.g., Bakkeoi, 1987) neglect the detection of any possible triggering factor. These may
28 be internal to the physical system (related to some peculiar morphologies favoring the
29 susceptibility to local failures) or external (e.g., a skier path cutting transversally the
30 snow layer slope or a rock-mass falling onto the layer). The randomness of such kind of
31 triggering factors makes them undetectable and useless for early warning purposes.
32 However, it should be noted that these factors may become effective only if a
33 predisposing state takes place in terms of snow layer thickness and temperature. This
34 leads to define the different alert levels on the basis of these two variables, for which
35 experimental quantification is easy and reliable. Consequently, the warning does not

1 deal with exactly identifying when, where and what specific triggering factor might
2 generate an avalanche.

3 In general, early warning prediction can be carried out during any of the above-defined
4 evolution stages. The choice of the particular stage should obviously consider that
5 elapsed times needed to predict the event, spread the alarm and reduce people and
6 goods exposure must not exceed the time after which the destructive event occurs. On
7 the other side, the limited time available in-between prediction and event should
8 indicate which kind of actions could be reasonably carried out. So, only in some cases
9 it will be possible to consider the opportunity to evacuate all buildings of an entire
10 neighborhood or forbid all exposed streets to traffic and people access. In some cases,
11 the small available time only allows some short actions, such as the interruption of
12 dangerous supplied services (gas and electricity) or closure of important infrastructures
13 highly exposed, such as railways or highways.

14 The first step that has to be followed in the development of the predictive tool is hence
15 the detailed study of the mechanisms that control the evolution of the phenomenon in
16 hand, and identify which phenomenon stage is the most suitable for the assessment of
17 safety conditions. For some problems, the choice necessarily falls into a specific stage,
18 while for others the choice may be multiple. For instance, the slow kinematics of
19 landslides in fine grained soils allows to place the predictive tool in any of the above
20 defined three stages, while the rapid kinematics of rainfall-induced landslides in coarse
21 grained soils prevents considering the paroxysmal stage.

22

23 **4. The architecture of the predictive model**

24 The second step of the development of the predictive tool is choosing the predictive
25 model. Promptness and reliability are mandatory requirements of the prediction. The
26 promptness is usually obtained by introducing model simplifications, which should
27 however not imply excessive accuracy losses, because they would increase
28 uncertainties and, consequently, false and missing alarms. An increase of model
29 complexity usually corresponds to a reduction in the observational scale of the
30 phenomenon. Complex models can only be applied to slope scale problems, while,
31 increasing the observational scale from local to regional, progressive simplifications
32 have to be introduced in the model and, consistently, less ambitious goals have to be
33 set in terms of reliability.

34 The wide variety of applications for EWS makes it difficult to generalize criteria to guide
35 the choice of the predictive model. It is only possible to refer to some classification

criteria, aiming at clarifying the philosophy of the chosen approach, and what ingredients it requires for its best implementation.

A first classification criterion distinguish between empirical and physically-based models. Empirical models extract relationships among cause and effect variables from available monitoring data taken over a prolonged time interval. Once set up the empirical relationships, they typically do not take into any account the physics governing the phenomenon. Their reliability essentially depends on the amount, accuracy and representativeness of the available data-set.

On the other hand, physically-based models relate cause and effect variables through mathematical relationships derived straightforwardly from the physical principles governing the considered phenomenon. The mathematical description of the model typically involves the assumption of simplifications that could strongly affect the accuracy of the prediction.

These two categories may also be used contextually in setting up predictive tools consisting of physically-based as well as of empirical steps.

The second criterion of classification refers essentially to physically-based models, and is strictly related to the need for a rapid prediction. It distinguishes between on-line and out-of-line predictions. The formers consist in real-time solving of the model equations, updated continuously over time with changes in boundary conditions indicated by field monitoring. The latter, instead, define simple mathematical equations or abaci relating cause and effect variables, by solving the governing equations preliminarily for a number of possible scenarios in terms of initial and boundary conditions (e.g, Pagano and Sica, 2013). These simple mathematical equations or abaci represent the predictive tools adopted to rapidly interpret the data from field monitoring.

Strictly related with the selection of the model is, finally, the design of the monitoring system. It has to be consistent with all the choices made about the previously illustrated points. The considered specific stage of phenomenon evolution, as well as the choice of the predictive model, unequivocally identify the physical variables to be monitored, their location and, finally, the number of measurement points.

In the following sections, the different features above highlighted will guide along the illustration of some application cases developed in the field of rainfall-induced flow-like landslides.

5. Examples of set up and calibration of the predictive model for early warning

1 In Italy the destructive potential of rainfall-induced rapid flowslides and debris flows is
2 sadly known. The significance of the problem in terms of number of events and victims
3 becomes clear by merely referring to the disasters occurred over the last years in
4 Campania (Cascini and Ferlisi, 2003, Calcaterra et al., 2004; Pagano et al., 2010; Santo
5 et al., 2012), Piedmont (Villar Pellice, occurred in 2008), Liguria (Cinque Terre, occurred
6 in 2011) and Sicily (Maugeri et al., 2011). The rapid kinematics characterizing the post-
7 failure behavior of these phenomena implies that the setup of an EWS may not rely on
8 the analysis of the short-lasting paroxysmal stage (Fig. 2).

9 Exception is made for EWS implemented along some roads or railways, where the
10 probability that the sliding mass detaching from a slope directly impacts vehicles is
11 small, while the probability that vehicles crash against previously fallen mass
12 obstructing the road is much higher. In such cases, the alarm might be launched in case
13 of the feared road invaded by fallen masses. Hence, the alarm itself could be based on
14 promptly gathering the occurrence of slope instabilities by carrying out monitoring of
15 displacements, and inhibiting road access in case of recorded movements exceeding
16 some threshold (Mannara et al., 2009).

17 If the exposed goods are instead likely to be directly impacted by the sliding mass, the
18 triggering of the instability must be predicted in due advance. The time span required
19 to reduce exposure, typically some hours, implies that the prediction should be based
20 on monitoring and interpretation of triggering precursors, carried out already during
21 the predisposing stage.

22 The phenomena in hand typically involve the mobilization of shallow covers rarely
23 exceeding 2 m in thickness, induced by rainfall infiltration and related suction drop.
24 Further physical variables governing the phenomenon are effect variables describing
25 soil cover wetting (e.g., degree of saturation, water content, water storage).

26 The predictive model may be built on empirical bases whereas, for the reference
27 geographical context, historical rainfall related to their effects are available.
28 Alternatively, it is possible to adopt physically-based approaches through which
29 turning, at any time, rainfall into effect variables related to slope stability conditions.
30 Different levels of these effect variables (or, alternatively, of slope stability indices
31 derived from them), may be chosen as the alert thresholds of the EWS. If the
32 mathematical model of the slope has been properly simplified, it may be possible to
33 operate “on line” by performing model simulations in few minutes.

34 Recent advances in field monitoring of effect variables, in particular soil suction and/or
35 water content, nowadays offer an alternative approach to the interpretation of rainfall
36 effects. Sensors like tensiometers, heat dissipation probes and Time-Domain

1 Reflectometer (TDR) probes, in principle could directly deliver all the effect variables
2 needed for the assessment of slope stability conditions. However, the spatial variability
3 of soil properties likely makes an EWS relying only on field monitoring of effect variables
4 unreliable. Field data are in fact always affected by local issues, and so they are poorly
5 representative of the whole monitored area, unless an extremely rich network of
6 sensors is installed, which in most cases is unfeasible. Hence, field monitoring should
7 be deployed supplementing, rather than replacing, the estimation of effect variables by
8 means of a more or less simplified estimation of rainfall effects.

9 The following application examples refer to single slopes, with extension of few
10 hectares, located in the Lattari Mountains (Campania, southern Italy) and in the basin
11 of Stura di Lanzo (Piedmont, northern Italy).

12 As already pointed out in the Introduction section, the choice of presenting examples
13 all referred to slope scale does not imply that the proposed classifications and
14 procedures are limited to this case. The scale of the system does not intrinsically relate
15 to model features but, rather, to the spatial resolution of the available input data, which
16 affects the entire structure of the EWS. In the following examples, the choice of the
17 slope scale is indeed made to show how, when high resolution data are available, the
18 adopted models and procedures for their calibration could be different and, in
19 principle, applicable to any scale.

21 **5.1 Empirical approach based on rainfall records**

22 The example herein reported refers to the chain of Lattari Mountains and, in particular,
23 to an area spreading in-between the towns of Pagani and Nocera Inferiore (Campania,
24 southern Italy). An intensely fractured calcareous bedrock covered by silty volcanic soils
25 characterizes the geology of the site. Volcanic covers have formed due to pyroclastic
26 air-fall deposits generated by eruptions, mainly those of the volcanic complex of
27 Somma-Vesuvius, occurred over the last 40000 years. Several rainfall-induced flow-like
28 landslides have interested these covers over centuries. Numerous phenomena also
29 occurred in the recent past (Table 1), usually triggered along slopes with inclination
30 angle between 30° and 40°.

31 A pluviometer installed in 1950, around 3 km far from the downslope area, provides a
32 daily rainfall series spanning over 50 years (Pagano et al., 2010). During this period,
33 three significant flow-like landslides occurred in 1960, 1972 and 1997 (Table 1). Daily
34 rainfall heights triggering the three phenomena were 87, 77 and 110 mm, respectively.
35 Figure 3 shows all the observed daily rainfall heights larger than the minimum value

1 followed by a landslide ($h_{dL} = 77\text{mm}$; h_{dL} =minimum daily rainfall associated with a
2 landslide), plotted in ascending order. It may be noticed that the condition $h_{ds} > h_{dL}$
3 (h_{ds} =significant daily rainfall, with “significant” intended as exceeding h_{dL}) was met 39
4 times, but only twice a landslide was actually triggered. This low correspondence
5 between daily rainfall and landslides depends on the existence of additional influencing
6 factors, related to the conditions of the soil cover at the onset of triggering rainfall,
7 which are neglected if only daily rainfall height is considered. Antecedent precipitation,
8 in particular, is supposed to play a crucial role, as it determines the amount of water
9 stored in the cover and lowering soil suction significantly, before the crucial suction
10 drop induced by the triggering rainfall.

11 The effects of antecedent precipitations may be taken into account by assuming that,
12 besides the rainfall directly triggering the event (usually identified with rainfall fallen
13 during the last day), they also play an important role in establishing the predisposing
14 conditions for the triggering of a landslide. The duration “x” of the antecedent period
15 may be chosen as the one minimizing the number of events (h_{ds} , h_x) characterized by h_x
16 similar to the antecedent precipitation, h_{xL} , accumulated before the three observed
17 landslides. The minimization yielded $x=2$ months. This corresponds to h_{2mL} values for all
18 three landslides of about 500 mm. Over the reference period only 5 rainfall histories
19 (h_{ds} , h_{2m}) resulted similar to the three (h_{dL} , h_{2mL}) which were followed by a landslide. If
20 this double threshold criterion had been virtually implemented as early warning
21 criterion in the considered area, it would have produced 5 false alarms over 50 years.

22

23

24

25 **5.2 Stochastic approach**

26 Few examples of real-time predictions of the probability of triggering of rainfall-induced
27 landslides in a small area (i.e. a slope or a small catchment) can be found in the
28 literature (e.g., Sirangelo and Versace, 1996; Sirangelo and Braca, 2004; Schmidt et al.,
29 2008; Greco et al., 2013; Capparelli et al., 2013; Terranova et al., 2015; Manconi and
30 Giordan, 2016; Ozturk et al., 2016). This is due to the intrinsic difficulty of having
31 available historical data sets of rain storms and corresponding landslides occurred in a
32 small area, with enough data to allow reliable estimation of the probability of landslide
33 triggering during extreme (and thus rare) rainfall events. Usually, only few landslides
34 occur at a site during an observation period of typically some decades, so that
35 probabilistic landslide initiation thresholds are mostly defined at regional scale, so to

1 have a rich data set of observed landslides (e.g., Terlien, 1998; Guzzetti et al., 2007;
2 2008; Jakob et al., 2012; Ponziani et al., 2012; Segoni et al., 2015; Iadanza et al., 2016).
3 The use of physically based models of infiltration and slope stability can help in the
4 prediction of slope response under conditions different from those actually
5 encountered during the observation period, thus allowing the definition of site-specific
6 landslide initiation thresholds (e.g., Arnone et al., 2011; Ruiz-Villanueva et al., 2011;
7 Tarolli et al., 2011; Papa et al., 2013; Peres and Cancelliere, 2014; Posner and
8 Georgakakos, 2015; Greco and Bogaard, 2016), which can be useful for carrying out
9 stochastic predictions. However, the application of such physically based approaches in
10 operational EWS still suffers the involved computational burden, which makes difficult
11 carrying out in real time the calculations required for landslide probability assessment.
12 Consequently, empirical models of the relationship between rainfall and slope stability
13 are still preferred for early warning purposes (Sirangelo and Braca, 2004; Greco et al.,
14 2013; Manconi and Giordan, 2016; Ozturk et al., 2016).

15 An example of setting up an early warning predictive model taking into account the
16 uncertainty of the prediction has been developed by coupling a stochastic predictive
17 model of rainfalls (Giorgio and Greco, 2009) with the empirical model FLAIR (Sirangelo
18 and Versace, 1996), which yields predictions of the triggering time for rainfall-induced
19 landslides. The same coupling approach may be used with other recently proposed
20 empirical models, such as GA-SAKe (Terranova et al., 2015).

21 The FLAIR model associates landslide triggering conditions with values of a mobility
22 function $Y(t)$, obtained by a convolution integral of the rainfall history $R(t)$ with a
23 suitable transfer function $\psi(t)$, which allows to model a wide variety of
24 geomorphological contexts, taking into account predisposing conditions generated by
25 antecedent rainfall (Iiritano et al., 1998; Sirangelo et al., 2003).

26 The choice of the transfer function and calibration of its parameters are carried out
27 based on the historical rainfall data records in a way that the $Y(t)$ function may result
28 as a suitable proxy of slope stability conditions. In particular, parameters are calibrated
29 so that peaks of $Y(t)$ correspond to historical landslides, so to identify a threshold Y_{cr}
30 that, if exceeded, indicates landslide occurrence.

31 The FLAIR model is currently implemented as predictive model in EWS provided for
32 different thresholds of attention, alert and alarm, corresponding to a progressive
33 approach of $Y(t)$ to the Y_{cr} threshold. As an example, for the case of Sarno (pyroclastic
34 slopes in southern Italy) the three mentioned thresholds were suggested at values of
35 $0.4Y_{cr}$, $0.6Y_{cr}$ and $0.8Y_{cr}$, respectively (Sirangelo and Braca, 2004).

The coupling with a stochastic predictive model of rainfall allows adopting the FLaIR model as a predictor of the probability of occurrence of future landslides (Capparelli et al., 2013). In fact, the convolution integral may be separated into two parts, one deterministic, the other random. The first integral computes the convolution of the rainfall history $R_{obs}(t)$ until the time at which the prediction is carried out. The second integral computes the convolution of the rainfall history $R_{pre}(t)$ predicted for the future time interval t_{pre} , the upper bound of which represents the lead time of the prediction:

$$Y(t) = Y_{det} + Y_{pre} = \int_{-\infty}^{t-t_{pre}} \Psi(t - \tau) R_{obs}(\tau) d\tau + \int_{t-t_{pre}}^t \Psi(t - \tau) R_{pre}(\tau) d\tau \quad (1)$$

The prediction of Y_{pre} is carried out by evaluating the probability conditioned to the trend of the rainfall observed before prediction. To this aim, the model DRIP (Disaggregated Rectangular Intensity Pulse) is adopted (Heneker et al., 2001). It defines, through an alternating renewal process, the observed alternation of rainfall and dry periods. This process guarantees, in fact, the stochastic independence of a rainfall event from the duration of the immediately preceding dry period as well as from the duration and the total rainfall height of the previous rainstorm. This allows carrying out the conditioned prediction Y_{pre} by only taking into account the rainfall history observed during the current event, when the prediction is being carried out.

The prediction Y_{pre} is carried out by a non-parametric approach, by selecting within the historical data set only the N_i rainfall events meeting the following two conditions: their duration was equal or longer than the observed part of the current rainstorm; along a time interval as long as the lead time, t_{pre} , before the prediction, the mobility function increased in the same proportion as it occurred during the last observed t_{pre} interval of the current rainfall event.

The rainfall events selected by following this procedure allow computing the expected value of Y_{pre} and the probability that, at the end of the interval t_{pre} , the condition $Y > Y^*$ occurs, whatever Y^* . Hence, once alert and alarm thresholds of the mobility function are defined, the sensitivity of the EWS can be adjusted by setting up the probability of threshold exceedance at which the relevant messages are launched (activation probability), so to obtain the best trade-off between false and missing alarms (Greco et al., 2013). Low values of the activation probabilities result in high number of alerts and alarms, and may lead to wrong activations of the system (false alerts/alarms). Conversely, a less sensitive system unavoidably increases the number of erroneous non-activations of the system (missing alerts/alarms).

The choice of the more suitable values at which setting the activation probabilities represents an important and crucial feature in the setting of an effective EWS. As

1 already specified, the system sensitivity has to take into account all consequences
2 relating with false and missing alarms. For the alert level, it is usually better to set a
3 high sensitivity, since actions determined by alert activations usually do not imply high
4 costs, nor a significant involvement of the served community. The same, however,
5 cannot be stated for the alarm level, as the procedures resulting from alarm spreading
6 usually imply high costs and discomfort for the community. As an example, evacuation
7 of people involves stopping all activities and interruption of all infrastructures and
8 services of public utility.

9 The described approach has been applied to the slope of Pessinetto, 40km North-East
10 of Turin. The slope, oriented towards South-West, with inclination angle between 30°
11 and 35°, is part of the watershed of the river Stura di Lanzo. It is constituted by a
12 metamorphic bed-rock intensively fractured, covered by a clayey-silt. Six debris flows
13 of different magnitude occurred there, within an area of about 1 km², from November
14 1962 to October 2000. The thickness of mobilized soils ranged between 1.5 and 2.0 m,
15 with soil volumes between few hundreds to 10000 m³.

16 For the calibration of the stochastic model and of the alert system, the pluviometer
17 data recorded in Lanzo, located 6.5 km east of the slope, were available. In particular,
18 the calibration has been carried out by interpreting the hourly rainfall heights recorded
19 between 1 January 1956 and 10 September 1991, during which four of the six recorded
20 landslides occurred. The subsequent data, from 11 September 1991 to 15 June 2004,
21 have been adopted to validate the predictive model and the performance of an EWS
22 based on its predictions.

23 The critical value for the mobility function, estimated over the calibration period, was
24 $Y_{cr}=168.4$ mm.

25 The minimum duration of a dry period in-between two rainfall events has been set
26 equal to 10 hours. By assuming only rainfall events exceeding 5 mm to be significant for
27 early warning purposes, a series of 1102 rainfall events meeting the requirements in
28 terms of stochastic independency was selected within the calibration period. These
29 selected events were characterized by durations between 1 hour and 182 hours and
30 rainfall heights between 5 mm and 615 mm (Greco et al., 2013). The validation period
31 of the EWS included 456 rain events selected as for the calibration period.

32 The EWS has been implemented through the definition of two different operational
33 levels: an alert level and an alarm level. The alert triggers as soon as the mobility
34 function is predicted to approach the value of $Y_a=0.75Y_{cr}$ with a probability higher than
35 a predefined threshold P_1 . The alarm is issued when the probability that Y exceeds the
36 critical value Y_{cr} is higher than a second threshold P_2 . The two thresholds are two

examples of possible choices of warning thresholds. As it will be shown hereinafter, for a given choice of warning thresholds, the sensitivity of the EWS depends both on the chosen probability thresholds. Predictions are updated with a hourly frequency and refer to a lead time interval from 1 to 6 hour later than the prediction time.

Two examples of the potentiality of the predictions of the probability of exceeding the two defined thresholds are given for two rainfall events occurred during the validation period, both followed by landslides. In particular, the reported predictions were carried out with lead times of up to 5 hours.

The first event occurred between 22 and 25 September 1993, and Y_a and Y_{cr} were overtaken 54 and 58 hours after the beginning of the rain, respectively. A landslide was triggered after 60 hours. In the second example, a rainfall event occurred between the 12 and 15 October 2000, Y_a was passed 39 hours after the beginning of the rain storm, Y_{cr} after 45 hours, and the landslide occurred after 46 hours.

The effectiveness of the stochastic approach for early warning is shown in Fig. 4 and 5. The graphs give the probability of exceeding the alert and alarm thresholds in the following five hours, predicted in real time. During the two considered rainfall events, the system predicted high values of the probability of exceeding both thresholds several hours in advance. In particular, assuming the activation probabilities $P_1=P_2=0.3$, in both cases (25 September 1993, Fig. 4; 14 October 2000, Fig. 5) the alert would have been issued about 9 hours before the landslide, while the alarm would have been launched already 6 hours earlier than the triggering time.

Hence, for the chosen values of Y_a and Y_{cr} , by properly setting P_1 and P_2 , the EWS would have been capable to issue, in both cases, the alert and alarm messages several hours before the actual landslide triggering. Tables 2 and 3 show the influence of different choices for P_1 and P_2 on the performance of the EWS, evaluated in terms of total numbers of missing and false alerts and alarms during the entire validation period. It looks clear that, once the alert and alarm thresholds Y_a and Y_{cr} are defined, the sensitivity of the EWS depends on the chosen activation probability: higher probabilities correspond to larger numbers of missing alarms, and smaller numbers of false alarms.

The optimal choice of P_1 and P_2 should be identified by comparing the costs deriving from false and missing alerts and alarms, with the benefits of the true alarms. As already pointed out in the previous sections, such a cost-benefit analysis is of course peculiar of the particular considered case.

The capability of issuing the alert some hours earlier than the triggering time is a non-trivial feature of the system, when it is implemented to mitigate risks from phenomena

characterized by a very rapid evolution, such as debris flows and other types of fast landslides, as well as flash floods. In these cases, effective measures to prevent damages and victims may be successfully implemented only if the alarm is issued sufficiently earlier than the triggering time of the phenomenon.

5.3 Physically based approach

In the town of Nocera Inferiore a rain gauge, installed in 1997, recorded hourly rainfall 500 m far from the slope where on 4 March 2005 a large landslide was triggered (Fig. 6). The slope had an inclination angle of 40° and was covered with a 2 m thick layer of silty volcanic soils. Rainfall records are adopted in this example to validate a physically based approach (Pagano et al., 2010), suitable to take into account a number of known influencing factors (e.g., triggering event, antecedent precipitation, instantaneous rainfall intensity, evolution of potential infiltration) (Pagano et al., 2008; Rianna et al., 2014a).

In modelling the problem, only factors considered of minor importance were neglected, according to Pagano et al. (2010). In particular, a one-dimensional infiltration problem through an unsaturated rigid medium was set through Richards equations, solved by the finite element code SEEP/W (GEO-SLOPE 2004).

Hourly rainfall records were adopted to quantify boundary fluxes at the uppermost boundary, while at the lowermost boundary two different limit boundary conditions were assumed (Reder et al., 2017) to account for the possible effects exerted by the fractured bedrock on the silty volcanic cover: a seepage surface condition, which simulates the capillary barrier effect under the hypothesis that fractures are empty; a flux regulated by the unit gradient, which instead approaches the case of fractures filled with the same material as that constituting the cover. The hydraulic properties of the soil, i.e. water retention curve and hydraulic conductivity function, were obtained by means of laboratory tests (Nicotera and Papa, 2007), as well as by coupled measurements of soil matric suction (Jet-fill tensiometers) and volumetric water content (TDR) carried out in a lysimeter (Rianna et al., 2014b).

Results yielded by the analyses (Reder et al., 2017) in terms of suction evolution refer to the hydrological year 2004-2005 (Fig. 7), which includes the landslide event. They clearly show how the predictions indicate a singularity at the triggering time, consisting in a drop of suction throughout the cover below 3 kPa for both boundary condition-types assumed at the bottom. Analyses conducted for the whole historical series of recorded rainfall, covering a time interval of 10 years including the landslide (Pagano et al., 2010),

1 indicate that the same singularity is yielded by the prediction only once more. Hence, if
2 this singularity (suction below 3 kPa throughout the cover) had been adopted as an
3 alarm criterion, the number of false alarms would have resulted significantly low.
4 Furthermore, the short time required to update the prediction (few minutes) is
5 consistent with the requirement of promptness of an EWS and allows carrying out “in
6 line” predictions.

7

8 **6. CONCLUSIONS**

9 After preliminarily analyzing the reasons which may lead a community to adopt an EWS,
10 in place of structural approaches, to mitigate risks associated with natural hazards, the
11 paper identifies the key elements of an EWS, which make it effective in accomplishing
12 the task of continuously checking the safety of a system. In particular, the work
13 highlights the importance of the accuracy of the prediction of the future evolution of
14 the system, which is the feature allowing the minimization of false and missing alarms.
15 Then, the definition of three evolution stages of natural hazards is proposed, so to set
16 rational criteria to identify the time at which the prediction should be carried out within
17 an EWS. In fact, depending on the characteristics of the hazardous phenomenon and
18 on the time required for the prediction, the chosen stage should allow deploying in due
19 time the actions aiming at reducing people and goods exposure.

20 Two further classification criteria are also adopted throughout the paper: the well-
21 known distinction between empirical and physically-based models; and the distinction
22 between on-line and off-line predictions, never adopted in the field of water-related
23 natural hazards.

24 The practical application of the proposed evolution framing requires detailed physical
25 knowledge of how the phenomenon develops over time and of the variables which can
26 be used as a proxy of its evolution. This novel framework for EWS setting up attempts
27 to bring some order in their design procedures, and is introduced with reference to
28 various kinds of natural hazards, as in principle it is suitable of general application.
29 Nonetheless, the paper is mainly focused on water-related natural hazards, and
30 particularly to landslides, for which some application examples are given.

31 With reference to two different landslide phenomena, namely flow-like landslides and
32 debris flows, both characterized by rapid evolution, the paper describes examples of
33 application of the proposed framework. First, the considered natural hazards are
34 analyzed in terms of their possible evolution stages. Then, the most suitable stage for
35 implementing the prediction is identified, along with cause and effect variables suitable

to characterize its evolution and to assess system safety conditions. The presented examples show how either empirical or physically-based models may be adopted, and how prediction uncertainty can be considered in setting up the sensitivity of an EWS.

The proposed frame and examples of application show how, to design and set-up an effective EWS (i.e. choosing the predictive model, the prediction time, the alert and alarm thresholds and their sensitivity, the mitigation actions allowed by the obtained lead time of prediction), an in-depth analysis of the physical characteristics of the hazardous phenomenon is mandatory.

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Location	Town	Date (yyyy.mm.dd)	H (m)	L (m)	V (m ³)
S. Pantaleone	Pagani	1960.12.08			
Scrajo	Vico Equense	1966. 11.23	220	300	10,000
Monte Pendolo	Gragnano	1971.01.02	205	375	7,500
S. Pantaleone	Pagani	1972.03.06	90	180	5,000
S. Pantaleone	Pagani	1997.01.10	135	240	4,500
Pozzano	Castellammare di Stabia	1997.01.10	440	750	40,000
Monte Pendolo	Gragnano	1997.01.10	125	210	4,500
Monte Pendolo	Pimonte	1997.01.10	125	135	4,300
Corsara	Corbara	1997.01.10	160	135	750
Ospedaletto	Ospedaletto	1997.01.10	250	450	10,000
S. Egidio M. Albino	S. Egidio	1997.01.10	215	500	10,000
Molina di Vietri	Vietri sul Mare	1998.05.05	570	1700	9,000
S.Egidio M. Albino	Nocera Inferiore	2005.03.04	295	530	33,000

1 Table 1

2

d _{pre} (h)	P ₁ =0.2			P ₁ =0.25			P ₁ =0.3		
	N _{1L}	N _{1F}	N _{1M}	N _{1L}	N _{1F}	N _{1M}	N _{1L}	N _{1F}	N _{1M}
2	23	7	2	19	3	2	18	2	2
4	27	11	3	22	7	4	21	6	4
6	31	12	3	25	7	4	22	5	5

Table 2

d _{pre} (h)	P ₂ =0.2			P ₂ =0.25			P ₂ =0.3		
	N _{2L}	N _{2F}	N _{2M}	N _{2L}	N _{2F}	N _{2M}	N _{2L}	N _{2F}	N _{2M}
2	16	4	0	14	2	0	11	1	2
4	22	10	0	17	5	0	15	4	1
6	29	16	1	20	7	1	13	4	5

Table 3

CAPTIONS

Figure 1. Evolution stages of a collapse mechanism

Figure 2. Evolution stages of collapse mechanism in rainfall-induced landslides featured by rapid kinematic

Figure 3 - Daily and antecedent-bi-monthly rainfall recorded at the Nocera Inferiore site and corresponding to significant events (red circles are associated with landslide triggering, green circle with rainfall histories similar to those resulting in landslides)

Figure 4. Stochastic approach to early warning: probability of exceeding alert and alarm thresholds of the mobility function at the slope of Pessinetto, predicted in real time (the upper panel reports the observed hyetograph) during the storm of 22.09.1993, when an earth flow occurred 60 hours after the beginning of the rain.

Figure 5. Stochastic approach to early warning: probability of exceeding alert and alarm thresholds of the mobility function at the slope of Pessinetto, predicted in real time (the upper panel reports the observed hyetograph) during the storm of 12.10.2000, when an earth flow occurred 46 hours after the beginning of the rain.

Figure 6. The Nocera Inferiore 2005 landslide area (Pagano et al., 2010, modified)

Figure 7. Prediction of suction evolution over the hydrological year of the Nocera Inferiore 2005 landslide at four different depths and for two different hydraulic conditions at the lowermost boundary (Reder et al., 2017, modified)

Table 1. Major flow-like landslides triggered since 1950 in the Mts. Lattari (H = difference in elevation between the main crown and the tip of the accumulation zone; L = projection on the horizontal plane of the distance between the crown and the tip; V = volume of the landslide body) (modified from de Riso et al., 2007)

Table 2. Stochastic approach to early warning: numbers of launched (N_{1L}), false (N_{1F}) and missing (N_{1M}) alerts at the slope of Pessinetto for three different lead times t_{pre} and

1 three different choices of the probability of alert activation P_1 . For each lead time, the
2 system carried out 964 predictions between 11 September 1991 and 15 June 2004
3 (validation period).

4

5 Table 3. Stochastic approach to early warning: numbers of launched (N_{2L}), false (N_{2F})
6 and missing (N_{2M}) alarms at the slope of Pessinetto for three different lead times t_{pre}
7 and three different choices of the probability of alarm activation P_2 . For each lead time,
8 the system carried out 964 predictions between 11 September 1991 and 15 June 2004
9 (validation period).

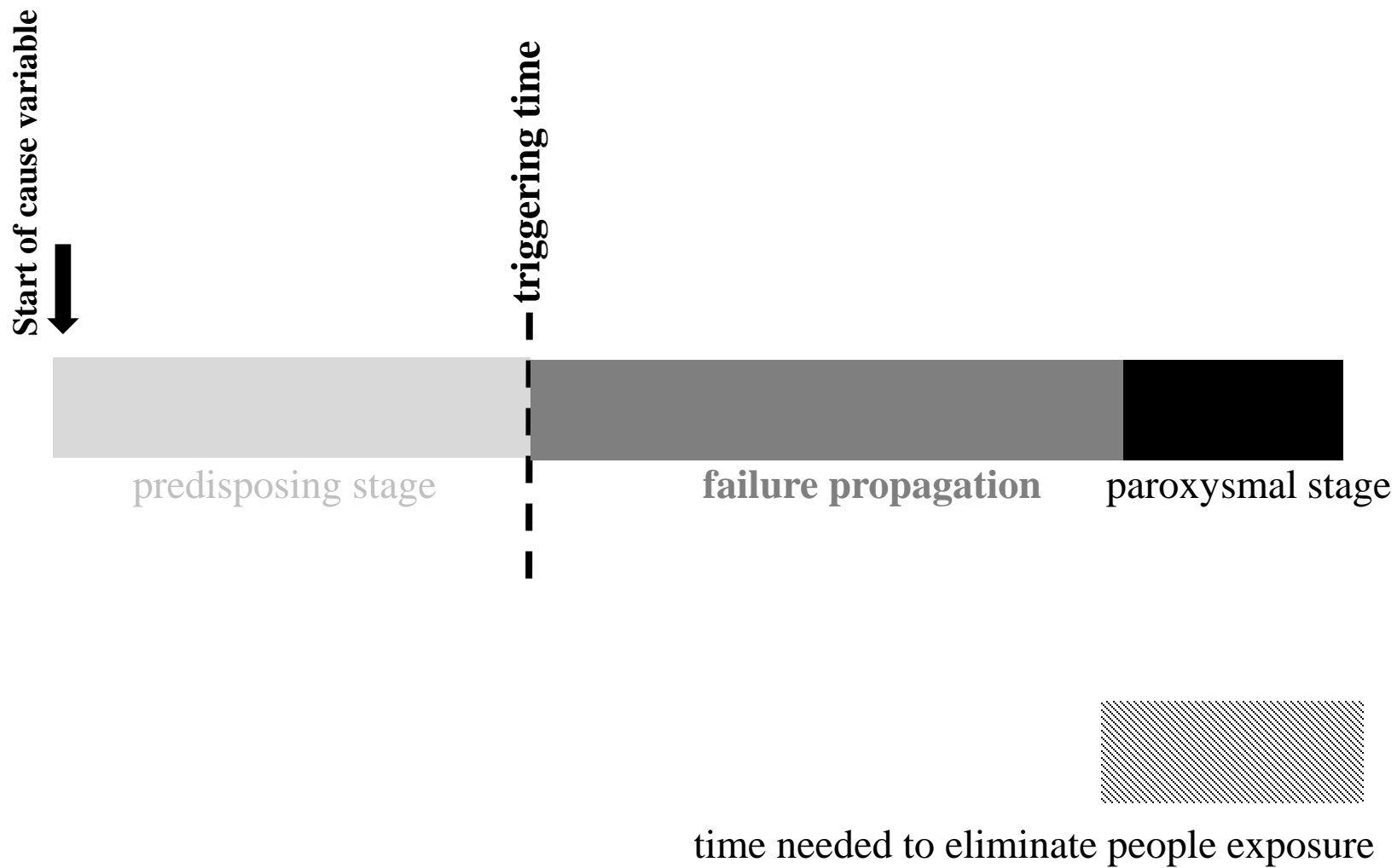


Figure 1

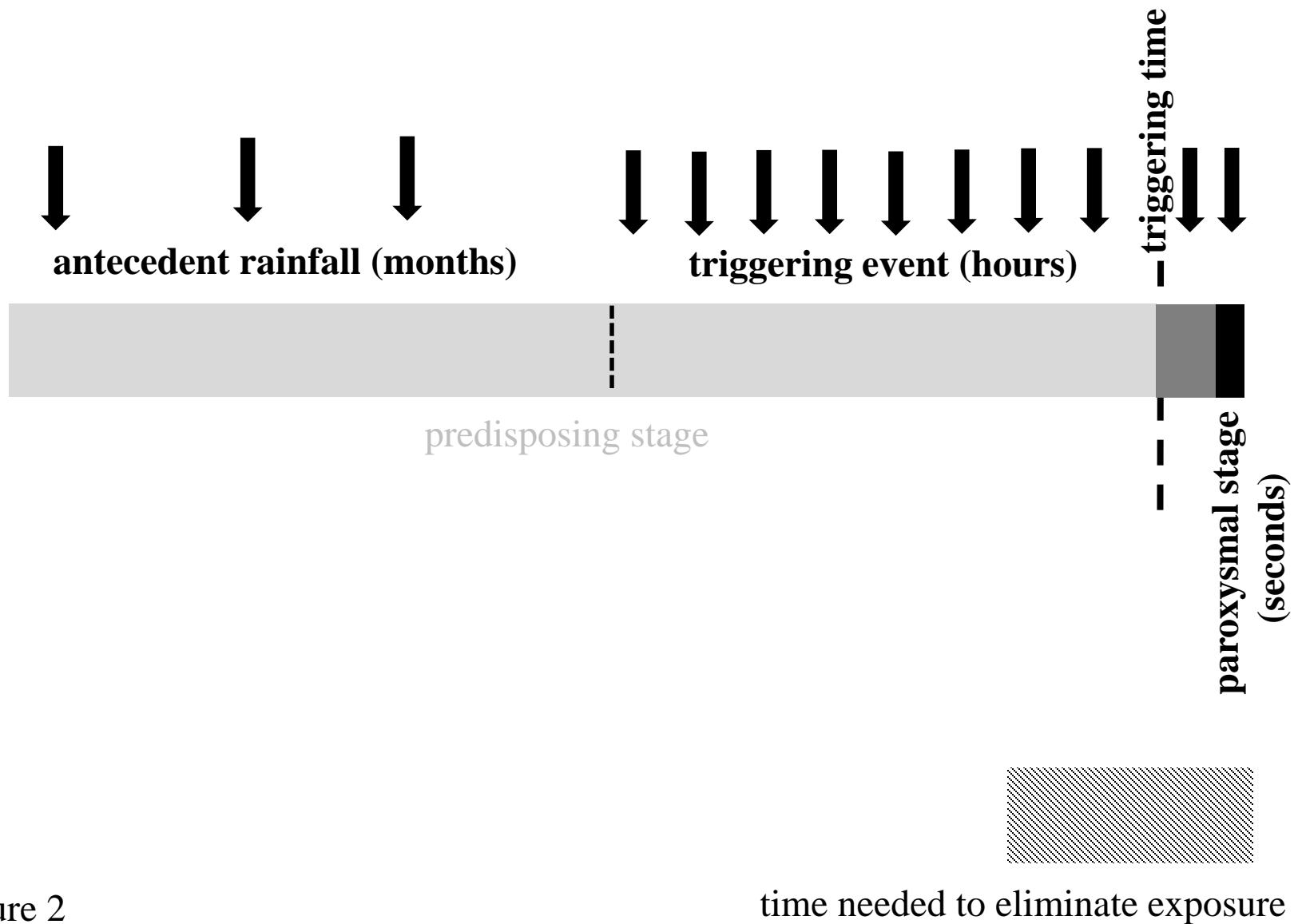


Figure 2

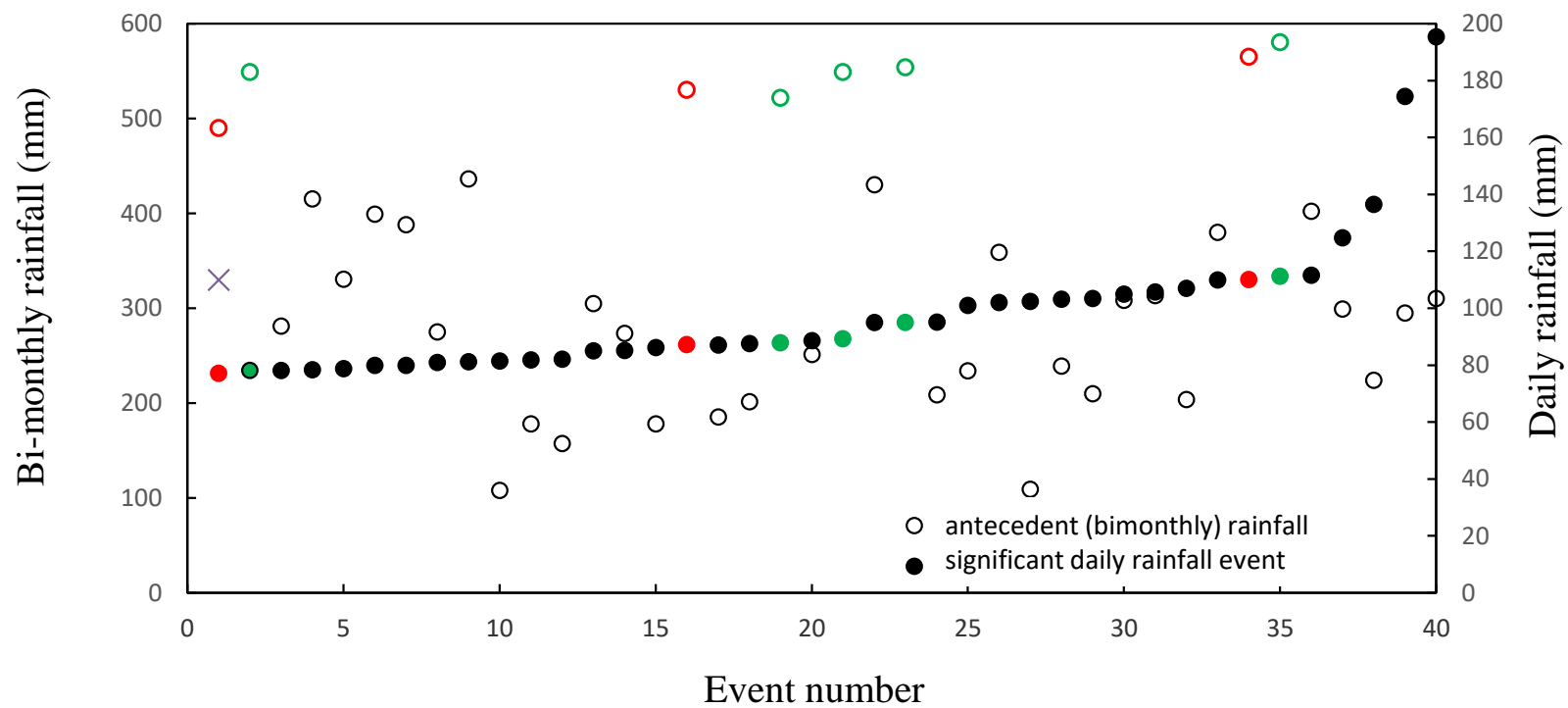


Figure 3

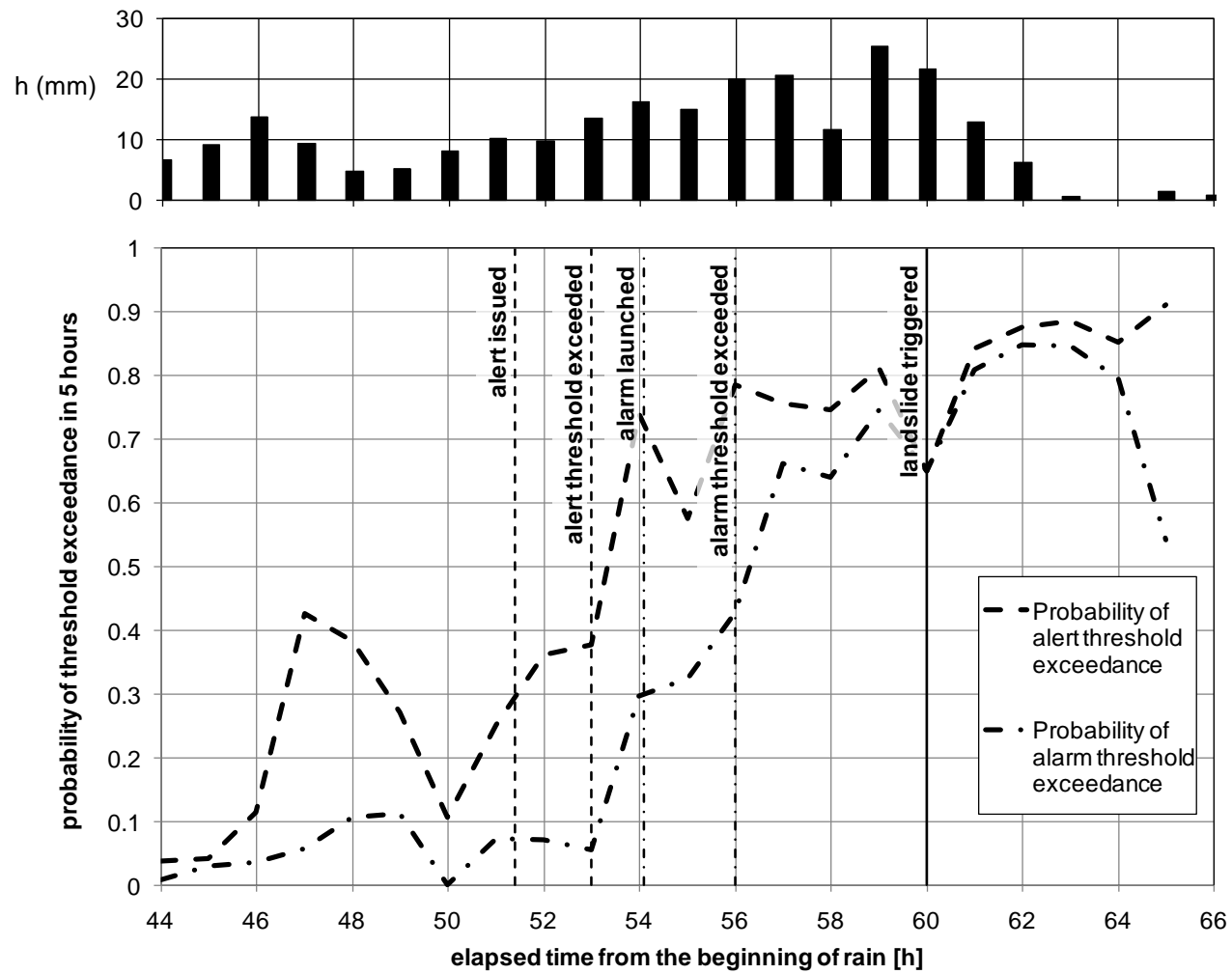


Figure 4

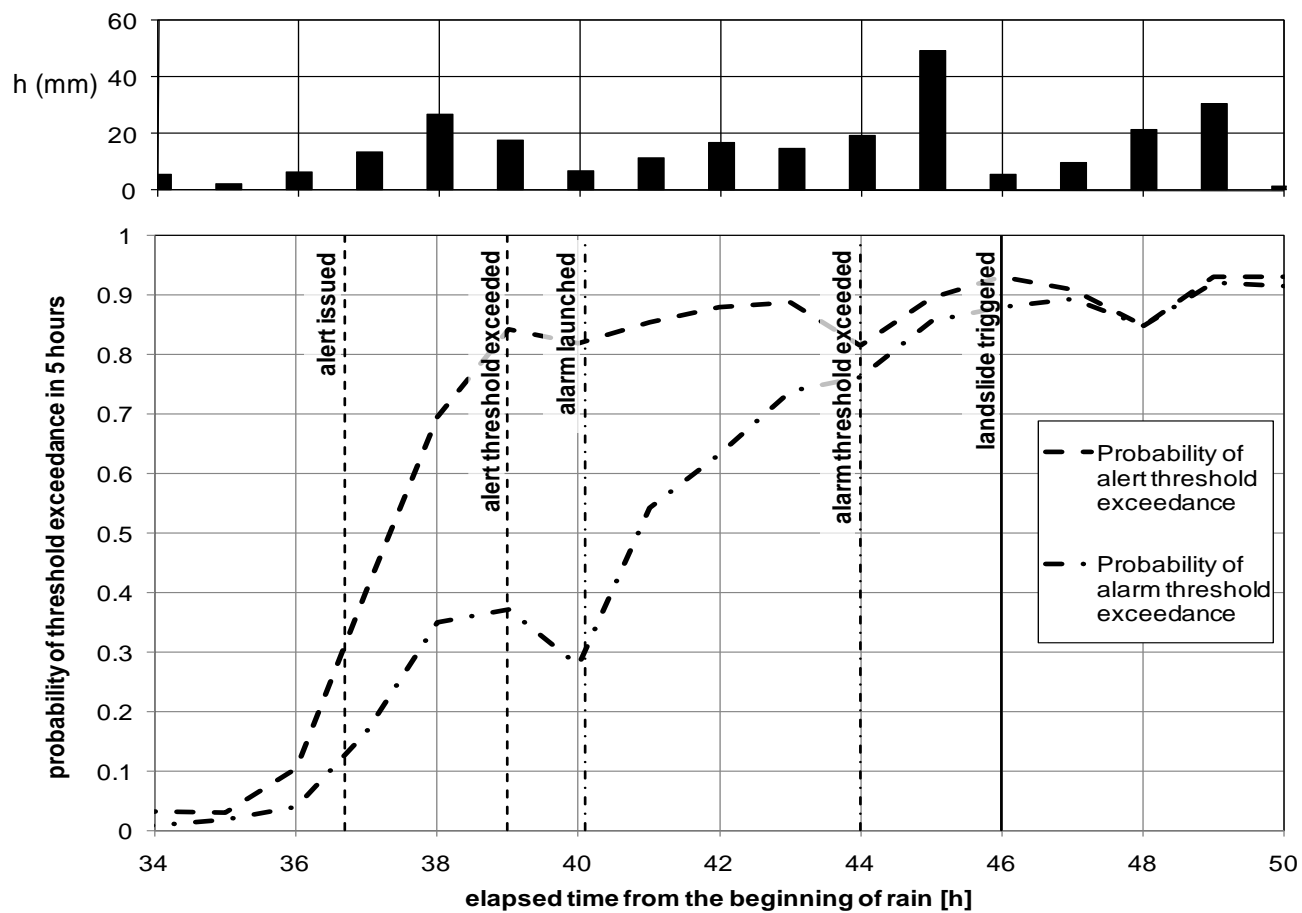


Figure 5



Figure 6

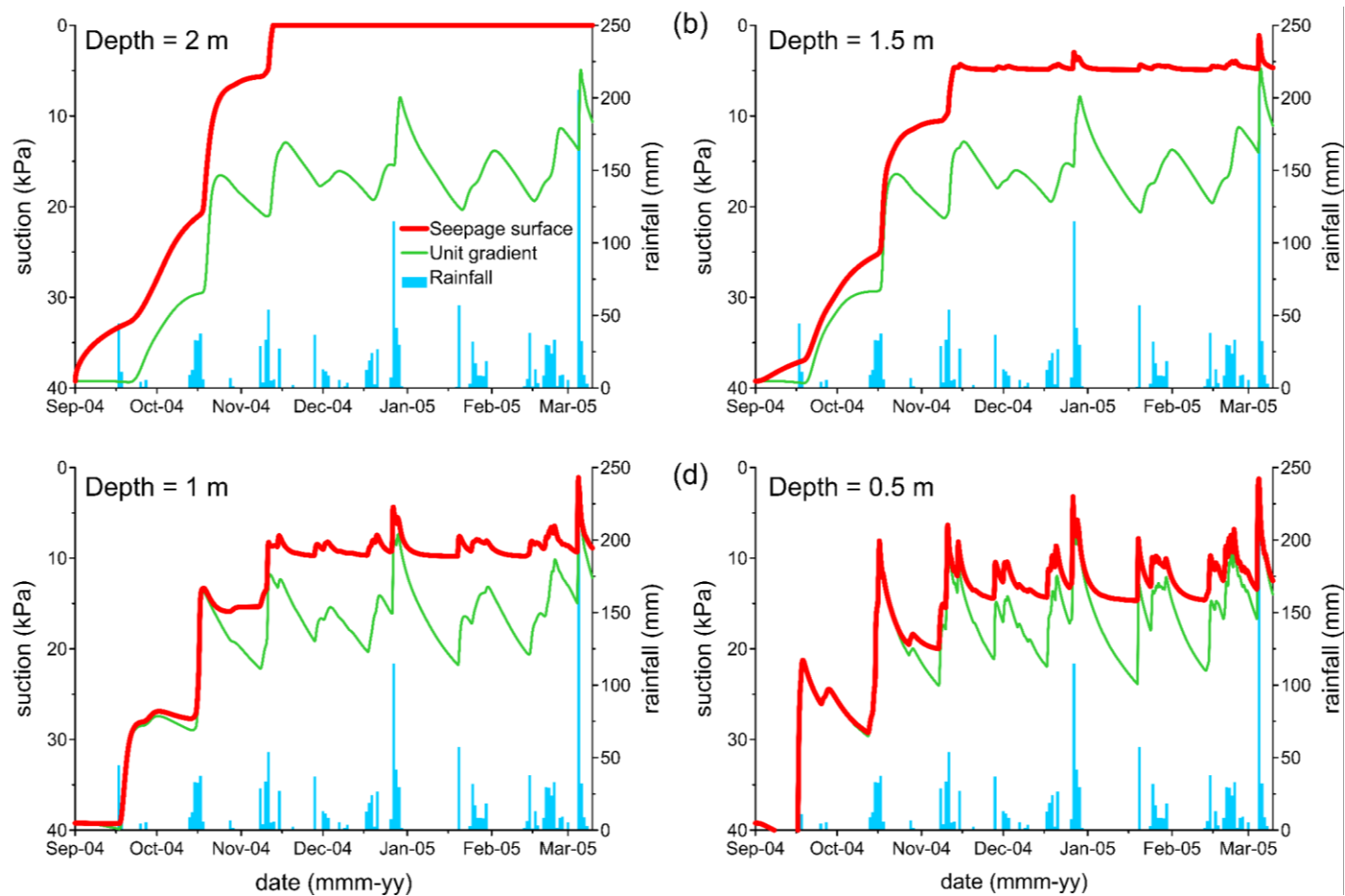


Figure 7