Dear Editor and Reviewers,

Thanks a lot for your kind comments on our paper. Now we have amended this manuscript according to the advice and used the track changes mode in MS to highlight modifications in this manuscript. Any change was also marked by yellow color.

We gave the detailed explanations about the revisions as follows:

## **Reviewer 1:**

## (1) Reviewer: L 88: Mohr-Coulomb is misspelled.

Authors: We have used "Mohr-Coulomb" to instead of "Mohr-Column" in our paper, location is L88.

## (2) Reviewer: L 92: c is a stress.

Authors: We modified this sentence as follows: "a stress and can be named of the cohesion force". Location is L92.

(3) Reviewer: L 92: "(which)" instead of "(Which)"

Authors: we use "which" to instead "Which". Location is L93.

## (4) Reviewer: L 133: "dependent on the variable r" instead of "dependent on the a variable r"

Authors: we deleted the word "a", and use the finial sentence: "dependent on the variable r". Location is L137.

# (5) Reviewer: L 145: add "to" before "identify"

Authors: We added this word before "identify". Location is 151.

## (6) Reviewer: L 193-194: I suggest to re-formulate this sentence.

Authors: The authors have rewritten these sentences in L199 and L200.

## (7) Reviewer: L 228: "takes" instead of "take"

Authors: The authors used "takes" to instead of "take". Location is L234.

- (8) Reviewer: L 305: the depth of the shear plane has a crucial influence on the FOS. It seems that the depth of the shear plane was assumed to equal the depth of the soil. Is this correct? Please comment (also in section 5) on the consequences of this assumption (sensitivity of model outcome) and on the accuracy of the spatial soil depth distribution assumed in this study.
  - Authors: if the depth of the shear plane was assumed to equal the depth of the soil, it is indeed not correct. In our model, we actually calculated FOS of each layer within each pixel. As described in L 307, each pixel was divided into 10 layers with the same soil depth. For example the depth of each layer was equal to 0.2m if the total soil depth was 2m. So if the FOS of  $i^{th}$  layer within a pixel was less than 1,

then our model will consider that the instable depth was i\*0.2m. The authors are so sorry for not clearly describing this issue and made the reviewer misunderstanding; we will add some detailed information to clear it, the location is at L246-L248.

## (9) Reviewer: L 307: please explain the discretization process in more detail

Authors: The authors gave detailed explanations about the discretization rule at Line 223-225 in Section 3.2.

## (10) Reviewer: Section 4: add information of the size of the investigation area could be given earlier.

Authors: we added the size of the investigating area at Line 258 in Section 4.1.

## (11) Reviewer: Figure 9: right: what does the star mean?

Authors: We have added the explanation about the red star at Line 364.

## **Reviewer 2**

- (1) Reviewer: the cohesion and friction angle always are not uniform distributed within two limits, the proposed method is not based physically as called by the authors.
  - Authors: In our paper, the reason that we proposed the physical model is based on the finite slope model. This model is suitable for mechanism analysis for shallow landslide and has been widely used and reported by many researchers, such as the articles written by Iverson et al (1997) and Apip et al (2010). Indeed, the cohesion force (c) and internal friction angle ( $\phi$ ) (the two important parameters in the infinite slope model) is difficult to determine and may not also be of uniform distribution, however, it should be noted that this distribution is only an assumption in our paper and only a method to solve the uncertainty of the two parameters. Additionally, even the two parameters are hardly to determine, they still cannot influence the physical meaning of infinite slope model, so the word of "physics" in the title of this paper is supposed to be retained.

# (2) Reviewer: As expressed in Eq. 1, the safety factor is influenced by phi\_b related to the matric suction. How this parameter is determined in the calculation?

Authors:  $\Phi_b$  is the parameter relating to the matrix suction, which is close to the internal friction angle  $\varphi$  in the condition of the low matrix suction. And low matrix suction means high soil water content; this situation is favorable to landslide. So in our paper,  $\Phi_b$  is set to be equal to the internal friction angle  $\varphi$ . This explanation is at Line 93.

## (3) Reviewer: How about the initial water content along the depth?

- Authors: Initial soil water content was based on the residual water content of each soil type. If the residual water content of a soil type is equal to w, then any pixel belonging to this soil type will be assigned this value. This means that the initial water content of each layer was assumed to be w, namely uniform distribution along the depth. This distribution has some drawbacks, for example, the deeper soil layer may have higher soil water content, but we cannot identify it at the regional scale and have to use the above easy method.
- (4) Reviewer: The distribution of cohesion and internal friction angle in Fig. 8 should be consistent with the soil type in Fig.7, but it is not so now.
  - Authors: We have explained why Fig.8 is different from Fig.7. Asked by editor, the authors added the detailed process to get these data according to the lithology map and handbook at Line 320-324.

**Editor:** 

(1) Editor: In addition to reviewer comments, please consider a further discussion in Section 2.3 of the choice of uniform distributions for the sampled parameters, and justification of the sampling scheme connecting sampled values of cohesion force and internal friction angle through the variable r\_i.

Authors: The authors added the detailed explanations about the two issues at Line 129-131, and line 139-140.

# A physics-based probabilistic forecasting model for rainfall-induced shal low landslides at regional scale

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9 Abstract: Conventional outputs of physics-based landslide forecasting models are presented as deterministic 10 warnings by calculating the safety factor (Fs) of potentially dangerous slopes. However, these models are highly dependent on variables such as cohesion force and internal friction angle which are affected by high degree of 11 12 uncertainty especially at a regional scale, which result in unacceptable uncertainties of Fs. Under such circum-13 stances, the outputs of physical models are more suitable if presented in the form of landslide probability values. 14 In order to develop such models, a method to link the uncertainty of soil parameter values with landslide probabil-15 ity is devised. This paper proposes the use of Monte Carlo method to quantitatively express uncertainty by assigning random values to physical variables inside a defined interval. The inequality Fs<1 is tested for each pixel 16 17 in *n* simulations which are integrated in a unique parameter. This parameter links the landslide probability to the 18 uncertainties of soil mechanical parameters and is used to create a physics-based probabilistic forecasting model 19 for rainfall-induced shallow landslides. The prediction ability of this model was tested in a case study, in which simulated forecasting of landslide disasters associated to heavy rainfalls on July 9 of 2013 in the Wenchuan 20 21 earthquake region of Sichuan province, China was performed. The proposed model successfully forecasted land-22 slides in 159 of the 176 disaster points registered by the geo-environmental monitoring station of Sichuan prov-23 ince. Such testing results indicate that the new model can be operated in a high efficient way and show more reli-24 able results attributing to its high prediction accuracy. Accordingly, the new model can be potentially packaged 25 into a forecasting system for shallow landslides providing technological support for the mitigation of these disas-26 ters at regional scale.

27 Keywords: Landslide, probabilistic forecasting, infinite slope model, hydrological process simulation

#### 28 1 Introduction

29 Rainfall-induced shallow landslides are common in many mountainous areas and are considered extremely 30 dangerous (Varnes, 1978). In despite the low volume of debris deposits involved in these processes (generally < 1,000 m<sup>3</sup>), rainfall-induced shallow landslides present high moving speeds (Cruden and Varnes, 1996), evolve 31 very rapidly, and can propagate even in presence of obstacles (Davide T. and Davide R., 2010). Current regional 32 33 landslide forecasting models mainly focuses on shallow landslides. They can be classified in three categories: 34 statistics-based methods (Caine, 1980; Crosta, 1998; Crosta and Frattini, 2001; Aleotti, 2004; Wei et al., 2004; 35 Wieczorek and Glade, 2005; Cardinali et al., 2006; Jacob et al., 2006), contributor-factor-based forecasting meth-36 ods (Dai and Lee 2003; Wei et al., 2007a; Chang et al. 2008) and physics-based forecasting methods (Montgom-37 ery and Dietrich, 1994; Wu and Sidle, 1995; Montgomery et al., 1998; Iverson, 2000; Wilkinson et al., 2002; 38 Crosta and Frattini, 2003; Salciarini et al., 2006). The physics-based forecasting models have overcome the draw-39 back of statistics-based models with respect to excessive dependence on rainfall data. Furthermore, by devising 40 mechanisms for coupling rainfall with soil surface mechanics using hydrological process simulation (Zhang et al., 41 2014a), the physically-based models represent an improvement over the independent treatment of these factors by 42 contributor-factor-based forecasting models e.g. (Wei et al., 2007a).

43 The physics-based forecasting model is able to describe the variation rule of hydrological parameters induced

44 by rainfall infiltration and further explain the failure mechanism of a slope due to the variation of hydrological 45 parameters. Those characteristics explain the interest of scholars to the physics-based forecasting model and its 46 implementation at regional scales (Schmidt et al., 2008; Montrasio et al., 2011; Raia et al., 2014). The most com-47 mon analysis unit used in physics-based forecasting models is the pixel, used for example in the well-known 48 TRIGRS model (Baum, et al., 2002, 2008). The safety factor of each pixel within a forecasting region,  $F_s$  ( $F_s$ =R/S: 49 where R is shear resistance and S is the driving force) is calculated considering rainfall infiltration, pixels are then 50 identified as unstable ( $F_s > 1$ ) or stable ( $F_s < 1$ ). From these results, landslide warnings are expressed determinis-51 tically by labeling each pixel of the forecasting area as either 'landslide occurrence' or 'nonoccurrence'.

However, it must be noted that the underlying physics-based forecasting model requires large number of surface data to be assigned to each pixel before safety factors can be calculated. The physics-based model is sensitive to the accuracy of such data, especially the soil mechanical parameters (cohesion force and internal friction angle) that can significantly influence the pixel stability. In general, and specially for large areas, seemingly deterministic soil mechanical parameters at pixel level used in physical models have different amounts of uncertainty (Schmidt et al., 2008; Rossi et al., 2013), which thus generate uncertain forecasting results. In this scenario, it is unwise to give deterministic forecasting results to the public while using the physical model in local forecasting service.

59 Providing probabilistic landslide forecasting results is the more direct solution to this issue. Currently, several 60 scholars advance in the development of physics-based probabilistic forecasting models (Schmidt et al., 2008; Raia 61 et al., 2014). However, the relationship between the landslide probability and the uncertainties in soil mechanical parameters is not addressed in their models. This effectively renders such probabilistic models actually still in 62 63 deterministic mode. For example, in Raia et al. (2014) a series of deterministic forecasting results are generated by the model during the simulation process from which an experienced forecaster with professional knowledge of 64 65 landslides is necessary for picking up the most probable one. Consequently, this approach requires a large number 66 of calculations, which is unsuitable for operational forecasting of shallow landslides.

This paper focuses on an effective method for linking landslide probability to the uncertain soil mechanical parameters. It uses Monte Carlo methods to propose a probabilistic forecasting model with a high calculating efficiency. The proposed model can directly generate probabilistic forecasting results instead of serial of deterministic results, and hence it will be more suitable to operational forecasting of shallow landslides, in special at the regional scale.

72 The next section introduces the physics-based probabilistic forecasting for shallow landslides model. Third 73 section addresses the general aspects of its application to a regional scale shallow landslide forecasting system. 74 Fourth section describes a case study in which the effectiveness of the proposed model is analyzed in a study case. 75 Sections five and six discuss the results and states the conclusions of this study respectively.

#### 76 2 Probabilistic forecasting for shallow landslides

## 2.1 The Infinite slope model for unsaturated soil slopes using safety factor $F_s$

There are two mechanisms that trigger failure in slopes subject to rainfall infiltration. They are loss of matrix suction and increasing of a positive pore water pressure (Li et al., 2013). In southwestern China, precipitation is rich in summer due to monsoon conditions from both Pacific and India Ocean (Wei et al., 2006). Before of the raining season slopes in this area are generally unsaturated during the relatively dry seasons. Almost all landslide disasters in southwestern China occur during the rainy season when the matrix suction of topsoil's suddenly decreases due to monsoon heavy rains. Consequently, this research focuses on the stability analysis of unsaturated soil mass.

During the evolution process from stability to failure driven by rainfall infiltration, the rapid loss of suction due to the increasing soil water content is the key triggering factor for shallow landslides. The safety factor  $F_s$  is used to evaluate the stability of slopes under the action of rainfall infiltration; in this scenario, the failure plane is 88 governed by the Mohr-Coulomb Mohr-Column failure criteria of unsaturated soil mass, and is assumed to be par-

allel to the slope surface (Fig.1). The expression of  $F_s$  based on the shear strength formula of the unsaturated soil

90 (Fredlund and Rahardjo, 1993) and the infinite slope model can be expressed as follows:

$$Fs = \frac{\tan\varphi}{\tan\beta} + \frac{c + \psi \tan(\varphi^{\flat})}{\gamma_t H_s \cos\beta \sin\beta}$$
(1)

92 Where c is <u>a stress and can be named of</u> the cohesion force,  $\varphi$  is the internal friction angle,  $\varphi^b$  is related to the

93 matrix suction (Which which is close to the internal friction angle  $\varphi$  in the condition of the low matrix suction),  $H_s$ 

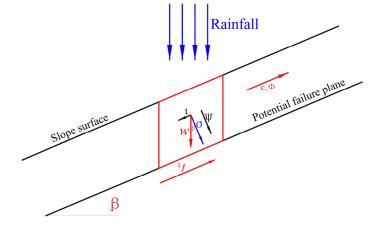
is the soil depth,  $\psi$  is the matrix suction of the soil, which is a function of the soil water content described as fol-

95 lows (Van Genuchten, 1980):

$$S_e = \frac{\theta - \theta_r}{\theta_r - \theta_r} = \left[\frac{1}{1 + (\alpha \times \psi)^n}\right]^m$$
(2)

97 where  $S_e$  is the saturation degree,  $\theta_s$  is the saturated water content,  $\theta_r$  is the residual water content,  $\theta$  is the soil

water content of the current hour,  $\alpha$ , *n* and *m* are the parameters of soil-water characteristic curve, and n=1-1/m.



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100 Fig.1 Infinite slope model for unsaturated soil in a slope

#### 101 2.2 Deterministic forecasting model using safety factor $F_s$

The infinite slope model aims to calculate the safety factor Fs to identify the stability of a slope. It has its basis in a theoretical hypothesis (Apip et al., 2010), which can describe the mechanical process of shallow landslides formation. This approach can give reliable results for each pixel as long as the soil mechanical parameters are accurate. From a deterministic point of view, this physical framework can be briefly drawn as follows: for each pixel in the forecast area, if  $Fs \le 1$  it's considered unstable, while pixels with Fs>1 are considered to be stable.

Acquiring the values for the soil mechanical parameters necessary for the infinite slope model require the use of field sampling or soil-texture based methods (Blondeau, 1973; Apip et al., 2010; Zhang et al., 2014a; Zhang et al., 2014b). However, the precision of these methods are relatively low (Schmidt et al., 2008), thus subject to high levels of uncertainty. Consequently, the seemingly deterministic infinite slope model based on soil mechanical parameters of each pixel is in fact uncertain (Schmidt et al., 2008; Rossi et al., 2013). This will be reflected in the safety factors *Fs* of each pixel, leading to a situation in which, despite the advantages of the physical-based landslide forecasting model, it may be misleading if used in a deterministic way for real world applications.

This is not an issue for other landslide forecasting models. For example, although the input variables of the contribution-factors-based forecasting model are also uncertain (Wei et al., 2007a) and thus it essentially belong to

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statistical models (Zhang et al., 2014a) it successfully account for the relationship between uncertainties of input
variables and results using fuzzy mathematics so that they are expressed as probabilistic forecasting for landslides.
The landslide probability is divided into five grades from 1<sup>st</sup> to 5<sup>th</sup> level, which represents a low, relative low,
medium, high and extremely high probability of occurrence of landslides, respectively. This forecasting result
conveys clearer landslide risk levels to the public (Wei et al., 2007b).

121 Due to the above reasons it is relevant to identify an effective relationship between the landslide probability 122 and uncertain input variables with uncertainty (cohesion force and internal friction angle) in a physics-based 123 probabilistic forecasting model.

#### 124 2.3 Probabilistic forecasting model for shallow landslides

In order to link landslide probability to uncertain variables, the nature of this uncertainty should be quantita tively expressed in mathematical language. Then, a physical parameter associated with both, input variables and
 landslide probability will be used to formalize the linkage.

128 The uncertainty of physical parameters can be described by a probability density function (Schmidt et al., 129 2008), because the uniform distribution suited in the investigation of large areas where information on the geo-hydrological properties is limited (Raia et al., 2014), which can easily allow authors to get random parameters 130 131 from its set approximate variation range instead of large amount of field and experimental works in large area. 132 Accordingly, the uncertainties of cohesion force and internal friction angle are described here as uniform probability distributions in the intervals of  $c=U(c_{min}, c_{max})$ , and  $\varphi=U(\varphi_{min}, \varphi_{max})$ , respectively. Then, Monte Carlo method 133 134 can be used to randomly extract cohesion force and internal friction angles from the two intervals n times in any 135 forecasting step. This random approach is used to account for the uncertain nature of soil mechanical parameters. 136 The detailed description of random extracting process is as follows: the extraction of the two parameters is de-<del>pendent on the adependent on the</del> variable  $r_i$  which is described as uniform probability distributions in the interval 137 of r = U(0,1), the random values of cohesion force  $c_i$  and internal friction angle  $\varphi_i$  can be identified via the equa-138 tions: Eq. 3 and Eq.4. In these equations,  $r_i$  can help to get a random number  $c_i$  with uniform distribution rule be-139 tween  $\mathcal{L}_{min}$  and  $\mathcal{L}_{max}$  because the variable  $r_{a}$  submits this distribution rule between 0 and 1.– 140

 $c_i = r_i (c_{max} - c_{min}) + c_{min}$  $\varphi_i = r_i (\varphi_{max} - \varphi_{min}) + \varphi_{min}$ 

143 There,  $c_{min}$  and  $\varphi_{min}$  are lower borders of intervals of the two mechanical parameters expected values;  $c_{max}$  and  $\varphi_{max}$ 144 are the upper borders. For any pixel in any forecasting step, a matrix  $M_i$  can be generated after the *n*-times random 145 extraction process:

 $M_{i} = [c_{i}, \varphi_{i}] = \begin{bmatrix} c_{1} & \varphi_{1} \\ c_{2} & \varphi_{2} \\ c_{3} & \varphi_{3} \\ \dots & \dots \\ c_{n} & \varphi_{n} \end{bmatrix}$ (5)

147 Any element contained in  $M_i$  has a specific physical meaning representing as a whole the physical phenome-148 non of uncertainty.

Provided other parameters identified in Eq. 1, each set of  $[c_i, \varphi_i]$  in  $M_i$  can generate a safety factor  $Fs_i = [Fs_1, Fs_2, Fs_3, \dots, Fs_n]$ . The array of safety factors  $Fs_i$  reflects *n* possible stable states for a pixel under these physical conditions. It's possible from there to identify a failure probability by the number of  $Fs_i \le 1$  (failure) in the *n* different states in the form of a ratio  $P(P \in [0,1])$  of  $Fs_i \le 1$  representing a tendency of a pixel to failure from stability.

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$$=\frac{Sum_{Fs<1}}{n}$$
(6)

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Larger *P* values in Eq. 6 indicates a forecasting result favorable to a high occurrence probability of failure under uncertain variables. This interpretation implies that a pixel will tend to one end failure when *P* exceeds 50% and its failure probability will only increase with larger values of *P*. Since *P* is derived from series of random (uncertain) variables  $[c_i, \varphi_i]$  via Eq.1 and Eq. 6, and is also directly associates with the landslide probability, the ratio ( $P \in [0,1]$ ) of  $Fs_i \le 1$  is a strong candidate for linking the landslide probability to the uncertain soil mechanical parameters.

For the purposes of practical implementation of this forecasting model, *P* is divided into a series of reference intervals in Table 1, the occurrence probability of shallow landslides increase from 1st interval to 5th interval of *P*. Five grades of landslide warnings are defined accordingly and color-coded Table 1.

164 Table 1 Reference intervals for shallow landslides forecasting based in probabilistic safety factor

Ratio intervals/%	P < 20	$20 \le P < 50$	$50 \le P < 60$	$60 \le P < 80$	$80 \leq P < 100$
Warning degree	1	2	3	4	5
Warning color	Colorless	Blue	Yellow	Orange	Red

#### 165 3 Probabilistic shallow landslides forecasting method at regional scale

## 166 3.1 Gathering basic data necessary for landslide forecasting

167 Topography is the main factor in shallow landslides. Nowadays, obtaining a DEM of precision adequate for 168 regional scale forecasting is straightforward. The DEM of the study zone is re-sampled into pixels with dimen-169 sions according to the extension of the area. The parameters required to calculate the ratio *P* for each pixel from 170 the array of safety factors  $Fs_i$  from a series of randomly extracted  $[c_i, \varphi_i]$  are identified in Eq.1. In this case matrix 171 suction, which is associated with the soil water content, should be identified by hydrological process simulation.

172 The key data necessary for the hydrological process simulation include the spatial distribution of precipitation, 173 land use, soil type and NDVI. Precipitation data with the same solution of the DEM can be obtained by 174 re-sampling rainfall prediction from Doppler radar supplied by meteorological bureaus. Land use, soil type and soil depth can be obtained from corresponding databases, all of which should be transformed into grid data with 175 176 the same solution of DEM. Other data necessary for stability calculations are slope angle for each pixel, parame-177 ters from soil-water characteristic curve ( $\alpha$ , m, n), and soil mechanical parameters. Slope angles can be derived from DEM using spatial analyst tools, parameters ( $\alpha$ , m, and n) of the soil-water characteristic curve are derived 178 from the different soil types within the pixel. 179

Regarding the identifications of soil mechanical parameters (cohesion force and internal friction angle), a relatively reliable way such as field sampling or soil-texture based methods should be used to assign an initial basic value to each pixel. Although these values include high uncertainty levels, they are used only as reference values while setting intervals of  $c=U(c_{min}, c_{max})$ , and  $\varphi=U(\varphi_{min}, \varphi_{max})$  (Raia et al., 2014). In this study, the lithology of the study zone is derived from a geological map, and the mechanical parameters (cohesion force and internal friction angle) of the corresponding lithology are identified using a rock mechanics handbook (Ye et al., 1991). Finally the data is assigned to each pixel using the grid cells of the DEM as reference.

From Eq.3 and Eq.4, it is necessary to identify the lower and upper border of intervals of the soil mechanical parameters. However, the exact values for lower ( $c_{min}$  and  $\varphi_{min}$ ) and upper ( $c_{max}$  and  $\varphi_{max}$ ) limits are very difficult to determine. From currently published papers, there is no known theoretical or experimental method to solve this issue. Raia et al. (2014) used variations of 1%, 10% and 100% around the values of cohesion force and internal friction angle (from field tests) to get several intervals, showing that the forecasting effectiveness is significantly improved by using a large variations. Consequently, this method applies a variation of 100% around the mean value of these parameters for each pixel to set the corresponding lower and upper borders as follows:

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$$c_{random} \in [0.5 \times c_{origin}, 2 \times c_{origin}]$$
195 $\varphi_{random} \in [0.5 \times \varphi_{origin}, 2 \times \varphi_{origin}]$ 

(7)

(8)

196 Where  $c_{\text{random}}$  and  $\varphi_{\text{random}}$  are the randomly extracted cohesion forces and internal friction angles,  $c_{\text{origin}}$  and  $\varphi_{\text{origin}}$ 197 are the mean value of each pixel (in this case from the rock mechanics handbook (Ye et al., 1991)).

#### 198 **3.2 Pixel level hydrological process simulation**

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The simulation of hydrological processes in the soil, which includes including rainfall interception, infiltration,
and evapotranspiration, is extremely complicate. However, in mountain conditions rainfall infiltration is the key
factor in the distribution of soil water content in soil-underlying surface which simplify the analysis. In southwestern region of China slopes are almost unsaturated before the rainy season due to characteristic distribution of
rainfall influenced by monsoon (Zhang et al., 2014b). The infiltration process in the vertical direction in unsaturated soil mass can be described by the 1D Richards's equation (1931):

$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial z} \left[ D(\theta) \frac{\partial \theta}{\partial z} \right] - \frac{\partial K(\theta)}{\partial \theta}$$
(9)

206 Where  $\theta$  is soil water content,  $D(\theta) = K(\theta)/(d\theta/d\psi)$  is the hydraulic diffusivity,  $\psi$  is the suction of unsaturated soil, *z* 207 represents the soil depth, which is positive along the soil depth and have the topsoil as the origin point,  $K(\theta)$  is the 208 hydraulic conductivity. The matrix suction is the dominant external force to drive the water movement in unsatu-209 rated soil mass, which can be calculated from Eq. 2.

Infiltration upper border: If the topsoil is unsaturated, it has a strong infiltration capacity (Lei et al., 1988).
Then, while the rainfall intensity is less than the infiltration capacity of the topsoil, all precipitation will infiltrate
into topsoil without any runoff. In this scenario, the infiltration border is governed by Eq. (10):

$$-D(\theta)\frac{\partial\theta}{\partial z} + K(\theta) = R(t), \quad t > 0, z = 0$$
(10)

214 Where R(t) is the rainfall intensity at time t. Here, the part of precipitation that exceeds the capacity of infiltration 215 of the topsoil will transform into runoff (no water storage above topsoil). In this case the topsoil of a pixel is con-216 sidered saturated. Thus, the Eq.10 that governs infiltration upper border is transformed into the equation of  $\theta = \theta_s$ 217 (Lei et al., 1988). There  $\theta_s$  is the saturated moisture corresponding to the soil type.

Infiltration bottom border: It has been experimentally demonstrated that the soil water content beyond a soil depth of 40 cm is barely influenced by rainfall infiltration (Cui et al., 2003). Consequently a region with a groundwater level near the surface of the soil has hydrological characteristics in which rainfall infiltration can hardly induce any groundwater level variation. In this case, it is reasonable to ignore the water exchange process between the lower boundary and groundwater (Zhang et al., 2015).

223 An implicit finite difference method is used for discretization of the 1D differential equation of water move-224 ment. The calculation time *t* is segmented into several intervals with the same time gap  $\Delta t$ , and the soil depth *L* of 225 each pixel is segmented into soil layers (each layer is named of *j* number) with the same depth  $\Delta z$ .

226 Identifying the initial soil water content is an important issue during the hydrological simulation process. However, this value cannot be directly determined at any given time for a large region due to complex rainfall 227 228 infiltration and evapotranspiration interactions. In the case of southwestern China, the winter is generally a rela-229 tively dry season thus; the soil water content value of the topsoil is very low, close to the residual water content of 230 the soil type (Zhang et al., 2014b). This situation is exploited setting the simulation time to start on January 1 of 231 the forecasting year (driest month in winter), which allows the use of the residual water content corresponding to 232 the soil type as and the initial value of the topsoil water content. Measured meteorological data from January 1 are 233 then feed to the simulation, which allows for a relatively accurate initial value of soil water content for the land-234 slide forecasting. Each simulation step take takes also into account the rainfall interception and evapotranspiration 235 processes by means of the algorithm of distributed hydrological model GBHM (Yang et al., 2002).

After the hydrological simulation process identify the initial soil water content of each pixel, the simulation focuses on the extraction of key hydrological parameters (soil water content and matrix suction) necessary for the 带格式的:突出显示

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stability calculation of each pixel using the expected rainfall from Doppler radar forecasting. During this last stage in the simulation in which landslide forecasting is performed, the evapotranspiration processes is not considered since this period is typically short, with rainfalls, negligible sunshine and lower temperatures.

#### 241 3.3 Probabilistic landslide forecasting at pixel level

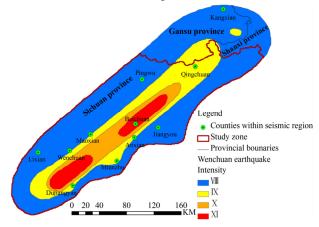
242 During the forecasting stage, the hydrological parameters (soil water content and matrix suction) of each pixel 243 in each forecasting step  $\Delta t$  are extracted via hydrological process simulation. Then the ratio P is computed for each pixel in several steps as follows: (1) The Monte Carlo method is used to extract the cohesion force and the 244 245 internal friction angle *n* times from the corresponding intervals ( $c=U(c_{min}, c_{max})$ , and  $\varphi=U(\varphi_{min}, \varphi_{max})$ ) of each pixel; (2) The safety factor  $F_s$  of <u>each divided layer within theone</u> pixel is calculated after each extraction, using the soil 246 247 mechanical parameters and the hydrological parameters only related to time as inputs of Eq.2, when the  $F_x$  of  $f_x^{d}$ layer is less than 1, then the calculation process within the pixel will stop; (3) Once the Monte Carlo process end, 248 249 the total times  $Sum_{Fs<1}$  of  $F_{s}<1$  is obtained, and the ratio P of  $F_{s}<1$  is calculated by Eq.6; (4) Finally the interval 250 of Table 1 where ratio P is located according to its value is assigned to the pixel as the early warning information 251 to be broadcasted.

After completing this process for all pixels within the forecasting region, the whole calculation at time *t* is finished, meanwhile a map of landslide warning degrees in the forecasting region will be generated at the end of each forecasting step. Such maps can then be used by the forecasting bureau of the region to issue landslide warnings to hazard mitigation units and the public.

## 256 4 Verification of the probabilistic landslide forecasting model

### 257 4.1 Study zone

The Wenchuan earthquake region with an area  $3.14 \times 10^4$  km<sup>2</sup> within Sichuan province, China is chosen as the 258 study zone in this study (Fig.2). In this region, at 14:28 PM (Beijing time) on May 12rd 2008, an Ms 8.0 earth-259 quake occurred. Massive potential unstable slopes were left after this earthquake, which are known to readily 260 261 evolve into shallow landslides by rainfall infiltration (Zhang et al., in Pres.). The close relationship between rain-262 fall and landslides in this region has been demonstrated by the short lag time of landslides and its strong correla-263 tion to rainfall time (Tang, 2010). The same study established that landslide events within the earthquake region are mainly in the form of shallow landslides (Tang, 2010). Tang (2010) also pointed out that shallow landslides 264 265 will be active within Wenchuan earthquake region at least for the next ten years. Such conditions make this region ideal for implementation of shallow landslides forecasting models. 266



7

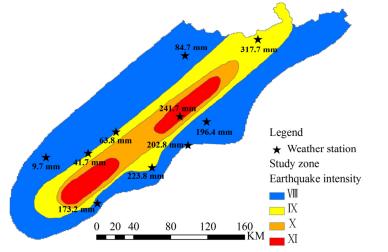
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268 Fig.2 Study zone and intensity distribution of Wenchuan earthquake

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#### 269 4.2 Rainfall process and related landslide events used for testing

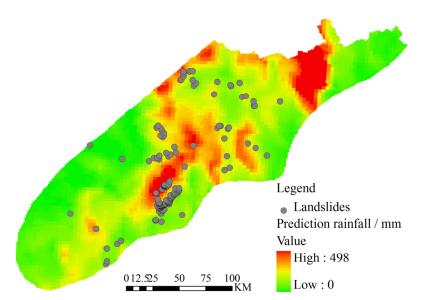
The chain of events in the Wenchuan earthquake area that ended in disastrous landslides in July 9<sup>th</sup> of 2013 was chosen to evaluate the proposed landslide probabilistic forecasting method. These events started with heavy rainstorms in the area during the days from July 1<sup>th</sup> to July 8<sup>th</sup> of 2013. As the rainfall measured by the weather stations within the area shows (Fig.3), the maximum accumulated precipitation during these days reached 317.7 mm, which become a key contributing factor for the landslide events of July 9<sup>th</sup> of 2013.



275

Fig.3 Total rainfall from 1<sup>st</sup> to 7<sup>th</sup> of July 2013

On July 9<sup>th</sup> of 2013, there was no evidence of decreasing rainfall intensity, on the contrary all evidence sug-277 gested heavier rainfalls. Records from the rainfall forecasted by Doppler radar provided by the weather bureau of 278 279 Sichuan province on that day, predicted a maximum 24-hour total precipitation within the earthquake region of up to 498 mm (Fig.4). Accordingly, the Weather Bureau of Sichuan province published red color warning signals 280 281 (which are the highest alert degree) for some locations within the study region. On that day, 176 landslide events were reported within the study region (Fig.4) leading to casualties and serious economic losses for local residents 282 283 (Zhang et al., 2014b). This typical landslide disaster triggered by intense rainfall is ideal to evaluate the main as-284 pects of the implementation of the proposed probabilistic landslide forecast model at regional scales.



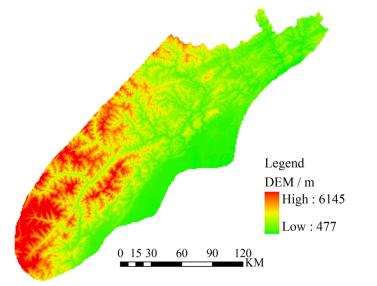
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286 Fig.4 Distribution of rainfall-induced landslides within Wenchuan earthquake region on July 9<sup>th</sup> of 2013

### 287 4.3 Gathering of basic data of study zone

The topography of the study region (Fig.5) was described by 125 m  $\times$  125 m DEM. This way, the study region was segmented into 6965505 pixels. A data matrix with 2576 rows and 2704 columns was created from the DEM and saved in text format. The basic data for hydrological process simulation and stability was resampled to

correspond to the same resolution of the DEM and saved as text matrices with the same dimensions.



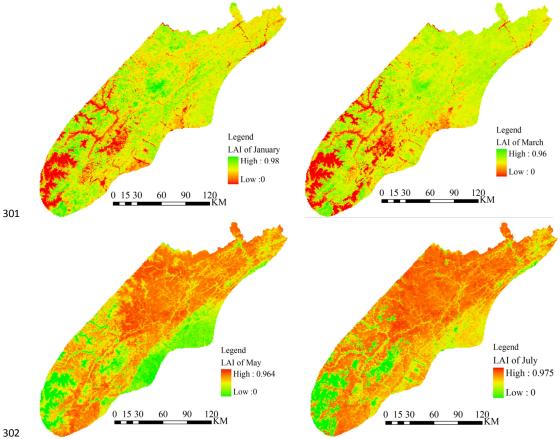
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293 Fig.5 DEM of Wenchuan earthquake area

## 294 4.3.1 Data for hydrological process simulation

295 The process of rainfall interception due to vegetation influence within the study region is taken into account

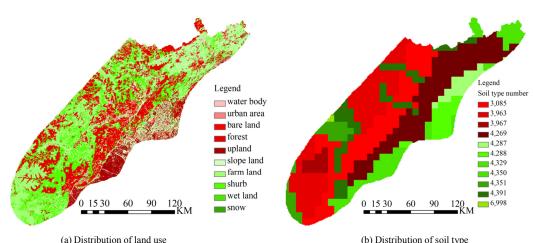
using NDVI values. Generally, the vegetation, and thus the values of NDVI vary with the variation of land uses and seasons. In this case, NDVI values from the same reason of the adjacent year are considered reasonably close, since the distribution of land uses within a region is relatively stable. The monthly NDVI distribution over the study region in the precedent year (2012) was used to adjust for canopy rainfall interception during the hydrological process simulation (Fig.6).



302 303

**03** Fig.6 Distributions of the LAI within the study zone

Other data required, such as land use (Figure 7 (a)), soil type (Figure 7 (b)), and the soil depth for Wenchuan
 earthquake region was obtained from the FAO database (http://www.fao.org/geonetwork/srv/en/main.home). The se data was processed using GIS functions so that they correspond to the pixels of the DEM.



307 308

309

(a) Distribution of land use

(b) Distribution of soil type

Fig. 7 Information of land uses and soil types within the study zone

310 The physical parameters of the soil required for the simulation of rainfall infiltration in the vertical direction 311 were determined by the land use and standard soil types within the study region. The soil thickness ranged from 1 to 4 m, soil depths of 1 m accounts for 44.1% of the study area, while deeper soils cover the remaining 55.9%. 312 313 Each pixel was divided into 10 layers (along the soil depth in the vertical direction) during the discretization pro-314 cess. There are 10 soil types in the area (shown in Fig. 7b). Their relevant physical properties are listed in Table 2.

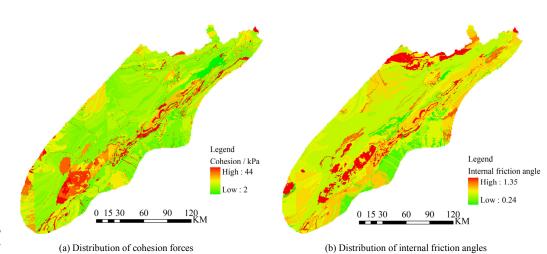
#### 315 Table2 Soil-water parameters for hydrological simulation

Soil trmo codo	Saturated moisture	Residual moisture	Parameters of curve		Saturated hydraulic
Soil type code	de Saturated moisture		Alpha	п	conductivity(mm/h)
3085	0.48278	0.07768	0.01896	1.40474	22.78608
3963	0.47303	0.07347	0.01796	1.42367	22.46508
3967	0.52726	0.08259	0.01867	1.41453	35.97075
4269	0.45649	0.06905	0.02306	1.55872	32.68625
4287	0.44596	0.07343	0.01971	1.47235	19.30871
4288	0.43797	0.07175	0.02064	1.53067	24.80996
4329	0.45049	0.07957	0.01604	1.44517	9.307170
4350	0.47990	0.07435	0.02156	1.42176	22.51646
4351	0.48278	0.07723	0.02040	1.41974	21.61279
4391	0.42784	0.06439	0.01623	1.63524	23.91267
6998	0.46154	0.06817	0.01770	1.46884	23.60925

#### 4.3.2 Data for calculation of slope stability 316

317 The Eq.1 indicates that matrix suction, cohesion force, and internal friction angle are the key mechanical pa-318 rameters influencing the slope stability. Simulation of the hydrological process is used to obtain the matrix suction 319 of soil mass as a function of the soil water content as shown in Eq. 2. Cohesion forces and internal friction angles for each pixel updated from the old database (Liu et al., 2016) are determined according to lithology map and the 320 321 rock mechanical handbook (Fig.8), the detailed process to obtain these data are as follows; each pixel will be firstly assigned the lithology attribution according to the lithology map, and then the rock mechanical handbook 322 323 which contains the mechanical parameters of all lithology will be used to find the corresponding parameters of 324 each pixel. These mechanical values are then used as a basic reference for constructing intervals of these parame-

325 ters ( $c=U(c_{min}, c_{max})$ ), and  $\varphi=U(\varphi_{min}, \varphi_{max})$ ) for each pixel.





328 Fig.8 Mechanical paramters of soil used for calculation of slope stability

(b) Distribution of internal friction angles

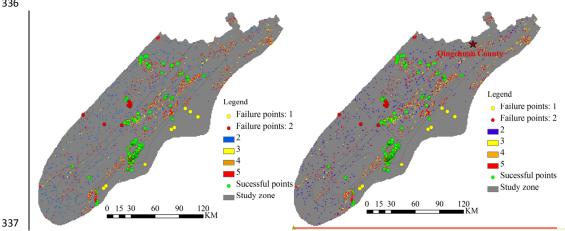
#### 329 4.4 Forecasting results

The landslide probability in Wenchuan earthquake region on July 9, 2013 was calculated, along with col-330 331 or-coded warnings for each pixel according to Table 1. This forecast covered 24 time nodes (hourly forecasts) covering the whole day. Two representative time nodes (at 6:00 AM and 15:00 PM) are chosen from the 24 h 332 forecasting results for further analysis (figure 9). The detailed forecasting results are listed in Table 3. These de-333 334 tails denote low variation in the forecast for these time nodes.

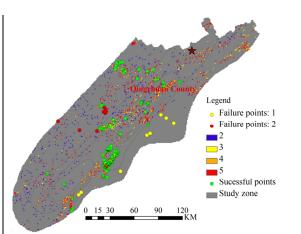
335 Table 3 Quantity of pixels with warning information

-	`		Blue	Yellow	Orange	Red
-	pixel	6:00 AM	534	150	332	699
	count	15:00 PM	527	158	321	704





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(a) Forecasting information at 6:00 (b) Forecasting information at 15:00 Fig.9 Landslide warning maps for Wenchuan earthquake region at two representative time nodes.

341 Colored points in fig. 9 represent landslide disasters occurred on July 9, 2013. Green points represent land-342 slides located in pixels forecasted with high degree of probability of landslides (orange-red), thus they are consid-343 ered successfully forecasted or true positives (159 events). The other 17 events represented by yellow and red points denote landslide events in low warning areas, which are considered as failed-forecasted landslides or false 344 345 negatives. These numbers indicate a missing-prediction rate of the new proposed forecasting model of about 346 9.7%.

347 Further analysis of these failures indicated that in some cases, the maximum slope angle of the corresponding pixel reported by the DEM is less than 4 degrees (yellow points). Furthermore, 4 of these pixels have slope angles 348 349 equal to 0 from the DEM. These small angles are for practical effect equal to flat terrain. In this scenario the probabilistic forecast model is unable to predict any unstable state, even during a more serious rainstorm. Howev-350 351 er, the real occurrence of landslide events at these locations indicates further analysis is necessary. In this case, the 352 most probable cause of this situation is the generalization process associated with the resolution of the DEM. It is 353 well known that increasing the size of the pixel tends to lower the estimated slope value, which in turn will raise 354 the failure prediction rate of models with high dependence on accurate slope values. A straightforward solution to 355 this problem is to further reduce the size of the pixel, which will in turn represent the real slope angle more accu-356 rately. This solution however will drastically increase the computing time. As reference, the current matrix dimensions of 2576×2704 (for 125 m pixel size) represent the limit for a regular workstation when the data is not parti-357 358 tioned.

359 There is still 8 prediction failures (marked by red dots) unexplained. These are considered to be related to oth-360 er aspects of the probabilistic forecasting model and unaccounted uncertainties. Detailed forecasting information about the landslide events in this study is listed in Table 4. 361

362 Table 4 Detailed forecasting analysis

	landslides	Successful predicted landslides	Failure to predict land-     Failure to predict land-       slides due to DEM     slides due to mode       imprecision     imprecision		Failure rate
	176	159	9	8	9.7%
363	The false predic	tion (false positives) rate	e for the probabilistic for	orecast model is high. Th	ne Fig. 9 shows high
261	warning degrees on	ncentrated around Guana	auon City and Oingah	uan County (marked by )	'rad star'' in Fig 0h)

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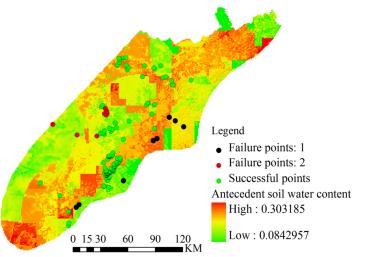
warning degrees concentrated around Guangyuan City and Qingchuan County (ma where landslide events did not occur. Looking at Fig. 3, the accumulative precipitation within Guangyuan City 365 366 during the days of July 1st and 7th are 317.7 mm according to the local weather station. This implies initial soil

367 water contents in the region close to saturation levels just before the forecasting time. Additionally, the cumulative 368 precipitation predicted from the Doppler radar reached more than 470 mm in Guanyuan City. Under the action of 369 such a combination of strong antecedent rainfall and forecasted rainfall, it is reasonable to expect high concentration of landslides (forecasted by the probabilistic model with different warning colors). Although the measured 370 rainfall data for July 9<sup>th</sup> was not available for this study, indirect information (absence of report of landslides and 371 other phenomena associated with heavy rainfall, even with notable initial soil water content levels) indicates the 372 real precipitation on July 9th was much smaller than forecasted from Doppler radar. Adding the known tendency of 373 Doppler radar forecasts to overestimate rainfall, it is reasonable to consider the precision of Doppler radar rainfall 374 375 as a key factor influencing the high false prediction rates of the proposed probabilistic forecasting model.

#### 376 5 Discussions

The general rule for the evolution of a slope from stability to failure is that the failure probability should increase as the rainfall process continues since increasing soil water content will decrease the suction matrix. This rule implies a forecasting result at 15:00 PM with more unstable pixels than the result at 6:00 AM. However, both of them are relatively close.

The distribution map of initial soil water content at 24:00 on July 8<sup>th</sup>, shown in Fig. 10, indicates significant effects of accumulated rainfall for landslide forecasting, the topsoil of some areas are even in saturated conditions (this means that only the topsoil was saturated rather than the whole soil layer). The total saturated pixels within study region are 532.



385

386 Fig.10 Intial conditions for landslide forecasting

Under these initial conditions, the mechanism of the runoff-infiltration process indicates that significant amount of precipitations will transform directly into runoff as the soil water content value of topsoil increases. In this case study, these high levels of initial soil water content attributed to strong antecedent rainfalls leads to lower variation rate of soil water content at pixel level. In this scenario, the variation of soil water content tends be gentle even during long and intensive rain, while excess water contribute mainly to the runoff process. This chain of events may explain the lack of clear evolution in the forecast in this particular study.

To further confirm this analysis, a new hydrological simulation was run in which the antecedent precipitation is ignored. The initial soil water content of each pixel for landslide forecasting was directly assigned with the residual soil water value according to the corresponding soil type (assuming a completely dry soil). All other parameters, including predicted rainfall from Doppler radar remained unchanged from the previous simulation. The forecast results at 6:00 AM and 15:00 PM under these new conditions are shown in Fig. 11 and Table 5. It is easy to observe differences between forecasting times, with quantity of unstable pixels at 15:00 PM larger than at 6:00 AM as expected. In this case, the low level of initial soil water content allows for strong infiltration process in the topsoil, which in turn leads to high variation rates for soil water content in each pixel, reflected in the differences of forecasting aligned with the expected evolution of the slope failure process.

Above analysis not only explain why there is not big difference between 6:00 AM and 15:00 PM forecasts during a high intensive rainstorm. It also to stress the relevance of the initial soil water content (or the effective antecedent rainfall) for any physically based landslide forecast model. A reliable method to calculate the initial soil water content can significantly influence the results of landslide forecasting models.

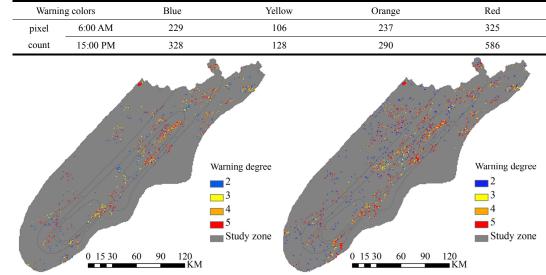


Table 3 Quantity of pixels with warning information, without considering the influence of antecedent soil water content

407 408

406

Fig.12 Forecasting results without considering the influence of the antecedent soil water content

Another issue is that most published physical models for landslide forecast such as the SLIP and TRIGRS 409 models (Montrasio et al., 2011; Tsai and Chiang, 2012) overestimated the probability of landslide occurrence at 410 regional scales. This proposed physics-based probabilistic forecasting model is also affected by this problem. 411 From the point of view of input parameters, three key factors can lead to this high false prediction rate. (1) The 412 413 soil mechanical parameters can only be obtained indirectly at regional scales, which greatly increase uncertainty. 414 Consequently, it is impossible to guarantee the correspondence of the fixed mechanical values at pixel level with 415 the actual values in nature, even using large intervals of soil mechanical parameters such as in this paper. Underestimating these values increase the probability to identify the corresponding pixel as unstable, which contribute 416 417 to high false prediction rates. (2) The nature of DEM models implies that a pixel identified as unstable by a pixel 418 based forecasting model may not really represent an unstable slope in nature. A slope may contain several pixels 419 of which only a few are unstable, or more likely at regional scales, a pixel may include several slopes. In this scenario isolated unstable pixels can contribute to high false prediction rates. (3) The precision of short term rainfall 420 421 forecasting is the last factor that can contribute to high false prediction rates. This is relevant in this study in which rainfall forecasts from Doppler radar overestimated the expected rainfall in some areas. 422

#### 423 6 Conclusions

The extreme complexity of the landslide formation process conditions that even physics-based forecasting models are unable to model the slope instability with 100% of confidence. However, the uncertainty of some input 426 variables (e.g., soil mechanical parameters) is responsible for a significant part of this situation. This research 427 adopted a probabilistic approach to express this uncertainty using Monte Carlo simulation. A single parameter (the 428 ratio P) was devised to couple the uncertain nature of input variables with shallow landslides forecasting. Fur-429 thermore, a regional physics-based probabilistic shallow landslide forecasting model was developed around this 430 parameter. The proposed model does not eliminate uncertainty; it manages it by explicitly introducing it into the 431 model expressing the forecast directly in probabilistic form. Our tests shown that this approach increases the 432 forecast precision (true positives) in real conditions, which is cardinal to protecting the public from catastrophic 433 consequences of shallow landslides and other associated disasters (such as debris flows).

It must be noted that the complexity of landslide forecasting is not limited to the uncertainty of physical soil properties, this research points to the initial soil water content as another key variable extremely difficult to identify accurately at regional scales. The model proposed in this paper implements a simulation of the hydrological processes occurring in the soil to estimate this value. Such simulation is time intensive, which is unfavorable for real world applications. Future research should focus in efficient methods for identification of soil water content at regional scales, which is a difficult but worthy challenge.

The goal of developing this physics-based probabilistic forecasting model is to serve for regional landslide disaster mitigation. Detailed resolution data, which in case of DEMs is readily available, are not always straightforward solutions for better forecasting results at this scale. In this case higher DEM resolution will improve the efficiency of the model failure prediction rates at individual pixel level due to better slope representation. However, it will also increase the time and resources required by the model to produce usable results. A balance point between pixel-level precision and operational efficiency is required for the proposed model in order to make it more suitable for regional operation.

Acknowledgement: This work was supported by Science and Technology Service Network Initiative (No:
KFJ-SW-STS-180), the Science and Technology Support Project of Sichuan Province (No. 2015SZ0214), the Risk
assessment on geohazards induced by extreme rainfall (CCSF201428), and hydrometeorological forecasting project from National Meteorological Center of China Meteorological Administration.

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