

1 Debris flow modeling at Meretschibach and Bondasca catchment, 2 Switzerland: sensitivity testing of field data-based entrainment model

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12 **Abstract**

13 Debris flow volumes can increase due to the incorporation of sediment into the flow as a
14 consequence of channel-bed erosion along the flow path. This study describes a sensitivity analysis
15 of the recently-introduced RAMMS debris flow entrainment model which is intended to help solve
16 problems related to predicting the runout of debris flows. The entrainment algorithm predicts the
17 depth and rate of erosion as a function of basal shear stress based on an analysis of erosion
18 measurements at the Illgraben catchment, Switzerland (Frank et al., 2015). Starting with a
19 landslide-type initiation in the RAMMS model, the volume of entrained sediment was calculated
20 for recent well-documented debris-flow events at the Bondasca and the Meretschibach catchments,
21 Switzerland. The sensitivity to the initial landslide volume was investigated by systematically
22 varying the initial landslide volume and comparing the resulting debris-flow volume with estimates
23 from the field sites. In both cases, the friction coefficients in the RAMMS runout model were
24 calibrated using the model where the entrainment module was (1) inactivated to find plausible
25 values for general flow properties by adjusting both coefficients (ξ and μ) and then (2) activated to
26 further refine coefficient μ which controls erosion (patterns). The results indicate that the model
27 predicts plausible erosion volumes in comparison with field data. By including bulking due to
28 entrainment in runout models, more realistic runout patterns are predicted in comparison to starting
29 the model with the entire debris-flow volume (initial landslide plus entrained sediment). In
30 particular, lateral bank overflow – not observed during these events – is prevented when using the
31 sediment entrainment model, even in very steep (≈ 60 – 65 %) and narrow (4–6 m) torrent channels.
32 Predicted sediment entrainment volumes are sensitive to the initial landslide volume, suggesting
33 that the model may be useful for both reconstruction of historical events as well as the modeling of
34 scenarios as part of a hazard analysis.

35 1. Introduction

36 Sediment erosion caused by debris flows causes flow bulking (in our case an increase in flow mass,
37 e.g. Iverson 1997) which strongly influences the runout behavior of debris flows. The term *erosion*
38 can be defined as the process of removing sediment from the channel bed while sediment
39 *entrainment* describes the procedure of incorporating the eroded sediment into the debris flow. The
40 entrainment of eroded sediment along the channel has been observed to considerably increase the
41 volume of debris flows (i.e. *bulking* process) at many different locations (e.g. Hungr et al., 2005;
42 Scheuner et al., 2009; Iverson et al., 2010; Berger et al., 2010a; Berger et al., 2011; Schürch et al.,
43 2011; Iverson et al., 2011, McCoy et al., 2012; Tobler et al., 2014; Frank et al., 2015). Two recent
44 extreme examples from the central Swiss Alps in the last decade showed significant bulking along
45 the flow path. In the Spreitgraben catchment (2009-2011), the overall multi-surge event volumes
46 increased to about 90'000 to 130'000 m³ – mainly due to entrainment along the active channel on
47 the fan (Tobler et al., 2014; Frank et al., 2015). At the Rotlauigraben catchment (2005), about 2/3
48 of the total volume of 550'000 m³ was eroded from the debris-flow fan during a multiple-surge
49 debris-flow event initiated by the failure of a glacier moraine during an intense rainfall event
50 (Scheuner et al., 2009). Therefore, the debris-flow entrainment and bulking process should be
51 included in debris-flow runout models to increase the accuracy of runout predictions including the
52 overall runout distance, location and amplitude of lateral bank overflow but also – importantly for
53 hazard assessment – the flow and depositional pattern on the fan (Gamma, 2000; Scheuner et al.,
54 2009; Hussin et al., 2012; Han et al., 2015; Frank et al., 2015).

55 However, models which include bulking by debris flows are relatively new and their performance
56 for practical applications has not yet been systematically investigated. Most entrainment modeling
57 studies focused on the field site where the erosion data for the underlying entrainment modeling
58 concept was collected and/or exclusively dealt with a single model application field site to test their
59 concept for entrainment modeling (e.g. Han et al., 2015; Frank et al., 2015). Herein we describe the
60 systematic application of the new RAMMS entrainment/bulking model (Frank et al., 2015) for
61 several recent events in the Swiss Alps.

62 Computational debris-flow runout models, which usually neglect entrainment, are often used to
63 assess runout distance and pattern (Crosta et al., 2003; D'Ambrosio et al., 2003; Medina et al.,
64 2008; Hungr and McDougall, 2009; Christen et al., 2012) and are therefore useful for hazard
65 analysis where predictions of flow intensity (e.g. the spatial distribution of flow depth and velocity)
66 are required (e.g. Scheuner et al., 2011). Because the debris flow process often was observed to
67 cause significant entrainment of sediment which can strongly influence the flow (e.g. Dietrich and
68 Dunne, 1978; Suwa and Okuda, 1980; Gallino and Pierson, 1984; Hungr et al., 1984; Benda, 1990;
69 Pierson et al., 1990; Meyer and Wells, 1997; Vallance and Scott, 1997; Berti et al., 1999; Cannon
70 and Reneau, 2000; Fannin and Wise, 2001; May, 2002; Wang et al., 2003; Revellino et al., 2004;
71 Scott et al., 2005; Godt and Coe, 2007; Breien et al., 2008; Gartner et al., 2008; Pastor et al., 2009;

72 Guthrie et al., 2010; Procter et al., 2010; Berger et al., 2010; Berger et al., 2011; Schürch et al.,
73 2011; Iverson et al., 2011; McCoy et al., 2012; Cascini et al., 2014; Tobler et al., 2014; Frank et al.,
74 2015 including entrainment and bulking debris flow runout modeling would be appropriate.
75 Processed-based entrainment rates using algorithms which consider the material properties of the
76 debris flow bulk (Crosta et al., 2003; D'Ambrosio et al., 2003; Medina et al., 2008; Deubelbeiss
77 and McArdell, 2012) as well as pre-specified entrainment rates which pre-define the absolute
78 volume of eroded material (Beguería et al., 2009; Hungr and McDougall, 2009; Hussin et al., 2012)
79 have been introduced in numerical runout models.

80 Recently, we introduced an entrainment algorithm in the RAMMS debris flow model for the
81 assessment of debris flow entrainment and bulking (Frank et al., 2015). The entrainment algorithm
82 uses a relation between basal shear stress and erosion depth based on an analysis of data from the
83 Illgraben catchment, Switzerland (Frank et al., 2015; Berger et al., 2011; Schürch et al., 2011). The
84 entrainment model was used to predict the overall erosion pattern and erosion volume at the first
85 site where it was tested, the Spreitgraben, Switzerland. However, secondary erosion processes such
86 as bank collapse and small torrential flood events between the debris-flow events increased the
87 uncertainty in the evaluation of the model. As a consequence, additional sensitivity tests were not
88 made. In this study we therefore focus on testing the sensitivity of the RAMMS debris flow and
89 entrainment model by assessing the sensitivity of total event volume (initial landslide volume plus
90 volume of eroded sediment) to initial flow volume. This is especially important in hazard analysis
91 where landslide scenarios are considered to trigger debris flows. For this sensitivity analysis, we
92 evaluated two Alpine catchments with diverse topography and recent well-documented debris
93 flows with volumes up to a few 10,000 m³: the Bondasca catchment in Southeastern Switzerland
94 and the Meretschibach catchment in Southern Switzerland.

95 **2. Entrainment modeling study sites and available data**

96 **2.1. Meretschibach catchment, Switzerland**

97 The Meretschibach catchment is located in Southern Switzerland, adjacent to and east of the
98 Illgraben catchment (Figure 1). The catchment area is about 9.2 km² and ranges from the summit of
99 the Bella Tola mountain (3,025 m a.s.l.) to the confluence with a drainage channel (619 m a.s.l.)
100 following into the Rhone River. Debris flows in the Meretschibach currently originate mainly in
101 the Bochtür subcatchment (1.42 km² area) which is covered mostly by steep debris slopes with
102 hillslope angles on the talus deposits of up to 60%. Patches of forest are present below the treeline
103 (2,200 m a.s.l.) and at the margins of the catchment, and largely contiguous forest is found along
104 both sides of the channel below an elevation of 1,600 m. The Bochtür subcatchment is underlain by
105 Triassic sericitized quartzite and white quartzites of the Bruneggjoch formation (Gabus et al. 2008).

106 The surface has several terrace-like structures have been mapped as sacking-type features (Gabus
107 et al., 2008) and are likely sources of landslides and rockfall.

108 Sediment deposits are abundant on the steep slopes of the catchment, originating from a variety of
109 mass wasting processes. Field observations of rockfall, the presence of damaged trees, and
110 unpublished records in the community forestry archives records indicate that rockfall is a dominant
111 process for generating sediment. Observations in the source area also indicate that dry ravel of
112 gravel and sand is also common in the summer months when the hillslopes are relatively dry.
113 According to the event inventory debris flows occur mainly between April and October (Szymczak
114 et al. 2010). Small debris flows start and deposit in the upper catchment, often depositing at an area
115 of lower slope located an elevation of approximately 2,000 m a.s.l. Convective storms or long
116 duration rainfall events have been observed to mobilize these sediment deposits and initiate debris
117 flows.

118 Georadar profiles on the west side of the unforested part of the Bochtür subcatchment as well a
119 airborne georadar measurements indicate that the sediment deposits are up to 5 m thick (Lucas et
120 al., 2017), although independent observations of the spatial distribution of sediment thickness are
121 not available. However extrapolation of that value to other parts of the catchment must be made
122 with caution because the profiles were made on a talus deposit, which may be interpreted as a
123 depositional area on the hillslope, that exhibits little geomorphic evidence of debris-flow activity.

124 In the years 2013 and 2014 several instruments and devices were installed in the catchment. In
125 October 2013, a meteorological station was installed above the initiation zone to measure
126 precipitation, temperature and snow height. Inexpensive wildlife-observation cameras recorded
127 images every 15 minutes during daylight were positioned along the most active western channel to
128 document the changes along the active channel. A debris flow monitoring station was installed on
129 23 July 2014 (Oggier et al. 2015a). It consisted of three geophones and a radar to measure the flow
130 stage. The radar is triggered by the geophones or the meteorological station and provides detailed
131 recordings of the debris flow hydrograph at a resolution of 1 Hz.

132 During summer 2014, three debris flows occurred. Because the monitoring station was installed
133 after the first event (20 July 2014), no hydrograph data are available for this event. Precipitation
134 and hydrograph data for the debris-flow events on 28 and 29 July 2014 indicate that the debris-flow
135 event on 28 July was triggered due to convective storms with large rainfall intensity (up to 3.3 mm
136 / 10 min) while the event 29 July 2014 initiated after a few hours of steady rainfall with moderate
137 intensity (up to 1.5 mm / 10 min). The pictures from camera 4 (see Fig. 1 for the location) clearly
138 showed that the initiation of the event on July 28 took place between 19:45 and 20:15 (UTC +2),
139 corresponding with the hydrograph measured at the observation station.

140 To obtain additional information about the initial volume and the spatial distribution of erosion, the
141 height models from 15 July and 28 October were compared. The digital elevation model of 17 July
142 was the result of a photogrammetry flight by swisstopo. The second digital elevation model (28

143 October) – which is a surface model (including vegetation) – was taken with a drone (Oggier et al.
144 2015b). The results indicate that the volume of the events eroded at the open debris slopes of
145 Bochtür was between 800 and 1,200 m³. Due to additional erosion downslope of the Bochtür
146 subcatchment, the total volume of the debris-flow events was between 8,000 and 10,000 m³.

147 **2.2. Bondasca catchment, Switzerland**

148 The Bondasca catchment in south-eastern Switzerland is a tributary to the Bergell valley (Figure 2).
149 The catchment area covers about 20.9 km². The geology is dominated by the Tertiary intrusion of
150 the Bergell granite. Originating from within the North wall of Pizzo Cengalo, a rock avalanche on
151 27 December 2011 deposited about 1.5 10⁶ m³ of sediments in the upper catchment with a runout
152 of up to two kilometers from the rock wall. The deposits are up to 17 m thick and cover an area of
153 about 0.760 km² while the hydrological sub-catchment is about 1.18 km² defined by the point
154 where the channel leaves the rock avalanche deposits at the lower end of the deposit.

155 The sudden sediment input from the rock avalanche was followed by several debris flows in the
156 summer of 2012 (5 and 14 July, 25 August, 24 September) whereof the two events in July
157 evacuated about 90'000 m³ of sediments from the rock avalanche deposit. The debris flows
158 originated mainly just below a planar rock face. Some of the debris flow surges are thought to have
159 been triggered due to water accumulation at the toe of the wall causing firehose-type debris flow
160 initiation (Figure 3B and 5B) e.g. as described by Godt and Coe (2007). The slope of the channel
161 on the rock avalanche deposit varies between approx. 32° (≈ 71 %) below the flat-shaped rock face
162 and regularly decreases to 15° (≈ 33 %) at the lower end of the rock avalanche deposit.

163 **3. Debris-flow entrainment modeling**

164 The goal of this study is to evaluate the entrainment algorithm implemented in the RAMMS debris
165 flow model (version 1.6.25) which has been previously described by Frank et al. (2015). In
166 particular, the sensitivity of the predicted erosion to the input parameters will be investigated, and
167 the data sets described above provide a new basis for evaluating the model. The previous study
168 (Frank et al., 2015) focused on demonstrating that more realistic runout results can be achieved
169 when including sediment entrainment and bulking into the runout model. However that study also
170 left many unanswered questions regarding the sensitivity of the model to input parameters,
171 especially the initial landslide volume, which was not possible to assess in the previous study.
172 Herein we focus on describing the sensitivity of the model to the initial landslide volume, using the
173 two well-documented events described previously.

174 Although the RAMMS debris model and the entrainment algorithm have been published elsewhere,
175 they will be briefly . The underlying numerical formulas of shallow water equation and the

176 Voellmy friction approach used in the RAMMS debris flow model are presented in detail in
 177 Christen et al. (2010); the entrainment model is described in Frank et al. (2015).

178 **3.1. Computational debris-flow model RAMMS**

179 The RAMMS debris-flow model is based on 2D depth-averaged shallow water equations for
 180 granular flows in three dimensions given by the coordinates of the topographic surface of the
 181 digital elevation model in a Cartesian coordinate system (x, y, z) and at time (t) (Bartelt et al.,
 182 1999; Christen et al., 2010). The mass balance equation incorporates the field variables flow height
 183 $H(x, y, t)$ and flow velocity $U(x, y, t)$ and is given by

$$184 \quad \dot{Q}(x, y, t) = \partial_t H + \partial_x(HU_x) + \partial_y(HU_y). \quad (1)$$

185 where $\dot{Q}(x, y, t)$ describes the mass production source term and U_x and U_y represent the depth-
 186 averaged velocities in horizontal directions x and y (Christen et al., 2010). The depth-averaged
 187 momentum balance equations account for the conservation of momentum in two directions x and y :

$$188 \quad S_{g_x} - S_{f_x} = \partial_t(HU_x) + \partial_x\left(c_x HU_x^2 + g_z k_{a/p} \frac{H^2}{2}\right) + \partial_y(HU_x U_y), \quad (2)$$

$$189 \quad S_{g_y} - S_{f_y} = \partial_t(HU_y) + \partial_x(HU_x U_y) + \partial_y\left(c_y HU_y^2 + g_z k_{a/p} \frac{H^2}{2}\right). \quad (3)$$

190 where the earth pressure coefficient $k_{a/p}$ is normally set to 1 when running the standard Voellmy-
 191 Salm friction approach, c_x and c_y represent topographical coefficients determined from the digital
 192 elevation model, S_g is the effective gravitational acceleration, and S_f the frictional deceleration in
 193 directions x and y (Christen et al., 2010). The frictional deceleration S_f of the flow is determined
 194 using the Voellmy friction relation (Salm et al., 1990, and Salm, 1993) and specifies the dry-
 195 Coulomb term (friction coefficient μ) scaling with the normal stress and the *viscous* or *turbulent*
 196 friction (coefficient ξ) depending on the flow velocity U (Christen et al., 2010; Christen et al., 2012;
 197 Bartelt et al., 2013):

$$198 \quad S_f = \mu \cdot \rho \cdot H g \cos(\phi) + \frac{\rho g U^2}{\xi} \quad (4)$$

199 where ρ is the mass density, g is the gravitational acceleration, ϕ is the slope angle, and $H g \cos(\phi)$
 200 is the normal stress on the overflowed surface. The tangent of the effective internal friction angle of
 201 the flow material can be defined for the resistance of the solid phase (the term containing μ) which
 202 extensively controls deceleration behavior of a slower moving flow. The resistance of the *viscous*
 203 or *turbulent* fluid phase (the term including ξ) prevails for a quicker moving flow (Bartelt et al.,
 204 2013).

205 **3.2. Debris-flow entrainment model**

206 The entrainment model was constructed using field data from the Illgraben catchment in
 207 Switzerland (Frank et al., 2015). The entrainment model describes the maximum erosion depth as a
 208 function of channel-bed shear stress and the vertical erosion rate of channel-bed sediment erosion.
 209 In detail, the model is based on the analysis of differential elevation models from pre- and post-
 210 event DTMs by Schürch et al. (2011b). This provides the depth of net erosion in a cell as a function
 211 of the local shear stress acting on the channel bed at the base of the flow. Similarly, the rate of
 212 erosion is constrained to be at the rate reported by Berger et al., 2011, using *in situ* erosion sensors,
 213 also at the Illgraben channel. In the analysis of Schürch et al (2011b), flow heights were determined
 214 using values interpolated between lateral levees after each event and the shear stress τ is
 215 approximated using the depth-slope product:

216
$$\tau = \rho ghS \tag{5}$$

217 where ρ is the bulk mass density of the flow, h is flow height, and S is the channel slope. An
 218 approximation of the typical potential erosion depth at the Illgraben follows the 50% percentile line
 219 fit to the distribution of elevation change for four debris-flow events (Fig. 3a in Schürch et al.,
 220 2011b). The entrainment algorithm implemented in the RAMMS debris flow model is defined by
 221 the maximum potential erosion depth e_m and a specific erosion rate $\frac{dz}{dt}$ (Frank et al., 2015). The
 222 relationship between the shear stress estimated (based on flow heights observed in the field) and
 223 the measured erosion depth (Schürch et al., 2011b) is described as a linear function of shear stress
 224 using a proportionality factor $\frac{dz}{d\tau}$ (Eq. 6). The maximum potential erosion depth e_m (for each grid
 225 cell) is calculated using a critical shear stress τ_c ($= 1$ kPa) and the proportionality factor $\frac{dz}{d\tau}$ ($= 0.1$ m
 226 kPa^{-1}) as a function of basal shear stress τ (Frank et al., 2015):

227
$$e_m = \begin{cases} 0 & \text{for } \tau < \tau_c \\ \frac{dz}{d\tau}(\tau - \tau_c) & \text{for } \tau \geq \tau_c \end{cases} \tag{6}$$

228 The average rate of erosion recorded at the erosion sensor site during the Illgraben debris-flow
 229 event of 1 July 2008 (Berger et al., 2011) is used to define a specific erosion rate $\frac{dz}{dt}$.

230
$$\frac{dz}{dt} = -0.025 \text{ for } e_t \leq e_m \tag{7}$$

231 When the critical shear stress τ_c is exceeded, sediment can be entrained from the channel.
 232 Entrainment stops when the actual erosion depth e_t reaches the maximum potential erosion depth
 233 e_m (Eq. 6). Normally, the specific erosion rate is implemented using the default value $\frac{dz}{dt} =$
 234 -0.025 ms^{-1} (Eq. 7) as presented in Frank et al. (2015). However, the model also allows to

235 account for larger or smaller entrainment scenarios by either doubling the rate or cutting it in half.
236 In this study, we will use these variable erosion rates for testing the sensitivity of the model.

237 **3.3. Entrainment model setup**

238 **3.3.1. Topographic resolution**

239 This study focuses on the evaluation of the sensitivity of the predicted (modeled) channel-bed
240 erosion in relation to the initial volume (e.g. initial landslide size) and the comparison of the model
241 results and the erosion pattern observed in the field. The ability to reproduce the observed erosion
242 patterns highly depends on a realistic representation of the channel morphology where the channel
243 is clearly visible in the DTM (Deubelbeiss et al., 2010 and 2011; Scheuner et al., 2011; Hohermuth
244 and Graf, 2014) and the channel dimensions (e.g. cross-sectional area) in the DTM have to be
245 similar to what is observed in the field (e.g. Frank et al., 2015). In this study, the initial topographic
246 data available for the Meretschibach catchment (described above) are on a square grid of 0.5 m for
247 a channel with a width of 2 to 4 m. At the Bondasca catchment data are available on a 2 m square
248 grid for channel varying in width from about 5 to 20 m. Although a channel width to DTM grid
249 spacing ratio of more than 5 to 10 would probably produce more accurate results, such data are
250 generally unavailable and the increase in the time for a simulation would be impractical.

251 **3.3.2. Entrainment model starting condition: block release and input hydrograph**

252 The type of initial release mechanism, block release (e.g. landslide) or input hydrograph, can be
253 determined based on field observations, potential model constraints and previous modeling
254 experience using the RAMMS debris flow model (Bartelt et al., 2013). Recent debris flow
255 modeling studies (Deubelbeiss et al., 2010; Deubelbeiss et al., 2011; Han et al., 2015) summarized
256 that debris flows in steep channels are mostly triggered by the sudden destabilization of material
257 originating from lateral bank collapses or dam-type deposits located within the channel itself. Han
258 et al. (2015) concluded that a hypothetical scenario such as the breaking of a dam – which they
259 used to start their entrainment model simulations – provides a stable and consistent release method.
260 Deubelbeiss et al. (2010 and 2011), for a case study in the Swiss Alps, suggested that the block
261 release method is most appropriate method for small to moderate initial volumes ranging from 1 m^3
262 up to 100 m^3 using the RAMMS debris flow model. The alternative release method using a
263 discharge hydrograph seems to be more suitable for larger initial volumes (Deubelbeiss et al., 2010
264 and 2011) ($> 100 \text{ m}^3$) which might be plausible for the larger channel of the Bondasca catchment.
265 The main problem with the block release is that the initial flow depth, width, or length of the initial
266 landslide can be unrealistically large in comparison to field observations. Users have to resort using
267 unrealistically large initial landslide volumes because most models do not allow for entrainment
268 along the channel path. The total debris flow volume, typically measured in the deposition zone, is

269 often used as the initial landslide volume, thereby implicitly ignoring the possibility that channel-
270 bed erosion and flow bulking occur (Frank et al., 2015). The input hydrograph starting condition in
271 RAMMS was intended to help circumvent this problem by allowing users to specify an influx of
272 debris as a function of time at a point lower in the watershed (e.g. just above the fan apex).
273 The block release volume is calculated by defining a specific block release height (with a precision
274 of 1 cm in this study) based on a pre-defined release area. The model assumes an instantaneous
275 failure of the landslide. The initial landslide surface elevation is then set to the initial elevation of
276 the land surface using an automatic procedure in RAMMS (the *subtract release from DTM* option
277 in RAMMS introduced in version 1.6.45). The main advantage of this procedure is that it prevents
278 unrealistic lateral spreading of the initial landslide mass in comparison with a landslide “block”
279 situated on top of the land surface.

280 3.3.3. Specified erosion rates

281 As a basis for comparison of the sensitivity of the entrainment algorithm, we hold constant the
282 default entrainment model coefficients (critical shear stress τ_c , potential erosion depth as a function
283 of basal shear stress $\frac{dz}{dt}$, erosion rate $\frac{dz}{dt}$) described above. In the previous study (Frank et al., 2015)
284 we demonstrated that an erosion rate of $\frac{dz}{dt} = -0.025 \text{ ms}^{-1}$ based on field data from the Illgraben
285 catchment, Switzerland (Berger et al., 2011) produces plausible results for the much steeper
286 Spreitgraben catchment. The catchments described in this paper are different in size and slope, so
287 one might expect some variation in erosion rate. However, the entrainment algorithm in RAMMS
288 allows for erosion rates up to $\frac{dz}{dt} = -0.05 \text{ ms}^{-1}$, with an option to include a shape file describing
289 where erosion may occur e.g. to account for engineering structures such as check dams or sills, or
290 natural features such as bedrock, where significant erosion is not expected during one debris-flow
291 event. For comparison we also used a rate of $\frac{dz}{dt} = -0.0125 \text{ ms}^{-1}$ based on a lower rate from
292 Berger et al. (2011).

293 4. Erosion and entrainment: observations and modeling results

294 4.1. Erosion patterns and entrainment model calibration

295 The observed erosion patterns are the basis for calibrating the RAMMS model coefficients, in
296 particular the friction coefficients ξ and μ are systematically adjusted in successive model runs,
297 until a satisfactory model result is achieved. The erosion pattern is derived by assessing the
298 difference between the digital elevation models. In both study areas, a measured erosion pattern
299 caused by one single debris-flow event is not available. We therefore focus on the spatial

300 distribution of erosion and deposition, instead of attempting to exactly predict the spatial change
301 due to the debris flow process.

302 In the Meretschibach, the change in the DTM includes the erosion due to three debris-flow events
303 which appear to have originated on an open-slope talus deposit (Figure 3A). The location of the
304 release area at the Meretschibach corresponds to the upper most visible erosion scar visible in the
305 DTM analysis and as described above includes the erosion due to three debris-flow events between
306 July 17 to October 28, 2014 (Fig. 3A). Therefore, the release area was placed within the channel,
307 where up to 2.5 meters of erosion was observed (upper end of the blue polygon at about 1750 m
308 a.s.l. in Fig. 3A.). The location is just below a bedrock step intersecting the main channel at about
309 1800 m a.s.l. Further monitoring at the upper Bochtür subcatchment using interval cameras and
310 conducting field observations on the site itself confirmed that at least some of the debris flows most
311 likely initiated at this location.

312 We calibrated the RAMMS model using an initial block release volume of 10 m^3 which
313 corresponds to the channel depth of 1-2 m and a width of 2-4 m at this location. To keep the initial
314 volume within the channel and prevent unrealistic lateral outflow, the method of subtracting the
315 initial landslide block from the elevation model was applied. Within the middle and lower channel
316 sections (Fig. 3A, blue polygon), the observed runout and relative erosion patterns can be best
317 reproduced using Voellmy friction parameters $\xi = 200 \text{ ms}^{-2}$ and $\mu = 0.6$ (Fig. 3B2). The parameter ξ
318 was determined by varying it within the range proposed by the developers of the RAMMS model (ξ
319 = 100, 200, 400) and inspecting the results (Bartelt et al., 2013). The modeled velocities of $6\text{-}9 \text{ ms}^{-1}$
320 using $\xi = 200$ are plausible, although independent field data are not available for comparison. The
321 parameter combination $\xi = 200 \text{ ms}^{-2}$ and $\mu = 0.7$ results in overbank flow along both sides of the
322 middle channel, which was not observed in the field (Fig. 3C2). There were neither deposits or
323 levees accumulated outside of the channel along this entire channel reach (Fig. 3A, blue polygon).
324 In contrast, the erosion pattern using $\xi = 200 \text{ ms}^{-2}$ and $\mu = 0.5$ resulted in an even distribution of
325 erosion along the entire channel length, which is inconsistent with the field results which showed
326 locations of deeper erosion depths (Fig. 3A). Within the normal range of the ξ parameter (Bartelt et
327 al., 2013) the differences in flow and erosion patterns were small in comparison to those resulting
328 from variations in μ , and are therefore not described herein. Hence, the further model runs were
329 conducted using the best-fit parameters $\xi = 200 \text{ ms}^{-2}$ and $\mu = 0.6$ in the sensitivity analyses
330 described in subsequent sections.

331 In the Bondasca catchment, the differential elevation model includes both the rock avalanche
332 deposit (27 December 2011) and the erosion due to one debris-flow event (5 July 5, 2012) (Fig. 5).
333 The upper end of channel erosion is located just below a planar outcrop of bedrock (Fig. 4B)
334 corresponding to the likely location debris flow initiation zone (Fig. 5C). The surface runoff
335 channels along the west side of the wall and runoff across the wall surface (Fig. 4B) converge on
336 the sediments at the bottom of the rock wall (see pictures from 2014 in Fig. 5). This scenario

337 suggests a firehose-type debris-flow initiation (e.g. Godt and Coe, 2007). Hence, this location was
338 used as the initiation area for the runout modeling.

339 The observed erosion along the main debris flow channel (Fig. 5C) – resulting from the two debris-
340 flow events in July 2012 – was used to calibrate the RAMMS model within the upper two thirds of
341 the study reach (Figure 4B, brown polygon). The best fit was found with the parameter
342 combination $\xi = 400 \text{ ms}^{-2}$ and $\mu = 0.3$. However, the observed elevation change also includes
343 secondary processes such as lateral bank collapse and the deposits of debris-flow snouts and levees
344 within the channel. Channel sections where the events eroded into the deposits can also be
345 identified by the stratigraphy in the field.

346 **4.2. Entrainment modeling and runout patterns**

347 The runout of a (landslide-type) block release of 10 m^3 , neglecting erosion (Fig. 6A) results in
348 maximum flow heights smaller than 0.5 m and the flow stops in the channel upstream of the
349 deposition zone. By contrast, including debris-flow erosion (Fig. 6B) leads to a more realistic flow
350 pattern consisting of flow within the channel reaching the deposition zone without any lateral
351 outflow. For comparison, if the total event volume ($\approx 1,555 \text{ m}^3$) is released as a landslide and the
352 debris-flow is not allowed to erode the channel (Fig. 6C), the runout shows overbank flow along
353 the upper channel reaches below the initiation area. The last scenario illustrates again the problems
354 associated with starting a runout model with the entire event volume assigned to the initial volume.
355 These results illustrate the ability of the runout model to better predict the erosion pattern and
356 runout if the channel-bed erosion and bulking process is included in the model.

357 **4.3. Erosion model sensitivity testing**

358 The results show that the total volume of eroded sediment, at both field sites, depends strongly on
359 the initial landslide volume. At both the Meretschibach and the Bondasca catchments, there is a
360 strong increase in the amount of sediment entrained and consequent increase in debris-flow volume
361 (Fig. 7) for relatively small increases of the initial landslide volume. At the Meretschibach
362 catchment, the entrainment model – using the default maximum erosion rate $\frac{dz}{dt} = -0.025 \text{ ms}^{-1}$ –
363 shows the highest sensitivity to the total erosion volume between 2 and 3 m^3 of initial block release
364 (e.g. initial landslide volume). Above 4-5 m^3 of initial block volume the increase of the total
365 erosion volume within the erosion domain remains approximately constant. The cause for the rapid
366 increase is related to the critical shear stress in the entrainment algorithm. Small initial landslides
367 do not generate enough shear stress to initiate erosion, whereas larger landslides can cause erosion
368 over the entire computational domain.

369 If we double the erosion rate to $\frac{dz}{dt} = -0.05 \text{ ms}^{-1}$ based on field estimates reported by Frank et al.,
370 2015 for the Spreitgraben catchment, a similar pattern is observed in the relationship between total

371 erosion volume as a function of initial release volume. However the erosion volumes are 3 to 5
372 times larger than the ones resulting from the default erosion rate at the same initial release volume.
373 In contrast, implementing only half the default maximum erosion rate ($\frac{dz}{dt} = -0.0125 \text{ ms}^{-1}$) for
374 low entrainment scenarios decreases the sensitivity to initial volume in an analogous manner.
375 Similar trends in total erosion volume as a function of initial block release (landslide) volume are
376 observed at the Bondasca catchment. However, the model only starts to predict significant erosion
377 volumes for block releases exceeding 20 m^3 , and the progressive increase in total erosion volume
378 as a function of initial block release volume is somewhat less steep. For the default erosion rate
379 $\frac{dz}{dt} = -0.025 \text{ ms}^{-1}$ (Frank et al., 2015), total erosion volumes increase most strongly between
380 initial volumes of 20 to 100 m^3 . The topography at the Bondasca catchment is somewhat less steep
381 and more variable, which may help explain these differences. Doubling the default erosion rate at
382 the Bondasca catchment results in the onset of erosion for initial volumes between 20 and 30 m^3 .
383 When reducing the default erosion rate to half of the default value, the erosion model depicts a
384 somewhat less sensitive reaction of the entrainment model than using the default rate.
385 Further assessment of the relation of the total erosion volumes depending on the initial volumes can
386 be made by calculating a growth rate (Hungr et al., 2005). We call it volume growth (VG) because
387 we address an overall ratio for a specific channel section instead of a classic “yield rate” per
388 running meter (Hungr et al., 2005):

$$389 \quad VG = V_{final}/V_{ini} = (V_{ini} + V_{ero})/V_{ini} \quad (8)$$

390 The volume growth (VG) is the ratio between the final debris flow volume V_{final} (consisting of the
391 initial volume V_{ini} and the erosion volume V_{ero}) to the initial volume V_{ini} . We analysed the
392 development of the volume growth (VG) to assess the sensitivity to various model parameters such
393 as critical shear stress τ_c (Fig. 8) as well as erosion rate $\frac{dz}{dt}$ and initial volumes V_{ini} (Fig. 9).

394 5. Discussion

395 The total erosion volumes observed in the sensitivity tests (Fig. 7) indicate a strong sensitivity to
396 block release volume (initial landslide volume) over a relatively narrow range of block release
397 volumes. This result is based on the assumption that the entire landslide fails instantaneously and
398 not progressively as a sequence of smaller landslides over a longer period of time. Information on
399 the style of initial landslide failure are not available for either field site, therefore we focus the
400 discussion on other factors related to the runout modeling. One striking difference between the two
401 field sites is that the size of the block release necessary to cause significant erosion is an order of
402 magnitude larger at the Bondasca site. The channel cross-sectional area where the flow travels and
403 therefore where the entrainment model is active is different at the two field sites. The

404 Meretschibach is substantially steeper (50 to 65% vs. 15 to 35%). This results in larger shear
405 stresses at the Meretschibach for the same initial landslide thickness, because the shear-stress varies
406 as the product of initial release thickness, flow density and channel slope. Other factors such as
407 differences in channel-bed roughness may also be important, however the Voellmy friction relation
408 within RAMMS does not explicitly consider channel-bed roughness.

409 In the RAMMS debris flow model, the development of the flow properties is controlled by the
410 Voellmy friction parameters ξ and μ (described in section 3.1) where ξ is the dominant control over
411 the flow velocities when the flow is moving rapidly and μ controls the runout distance (Bartelt et
412 al., 2013). The ξ parameter was found in this study to have a relatively small influence over the
413 flow behavior in comparison with the Coulomb friction term μ . However, a calibration of the
414 parameter ξ using an approximate discharge (block release volume or hydrograph implementation)
415 and as observed at a particular channel section can help determine the most plausible ξ value
416 within the ranges proposed by the developers of the RAMMS model ($\xi = 100, 200, 400$) (Bartelt et
417 al., 2013). The RAMMS manual (Bartelt et al., 2013) suggests using the tangent of the fan slope as
418 first estimate to determine μ . As described in the calibration procedure (section 4.1), this
419 corresponds to relative erosion patterns determined by differential DTM analysis. Hence, we
420 conclude that the tangent of the channel slope can be used as a first approach to define parameter μ
421 also for the entrainment model when applying to channel sections which exhibit a roughly constant
422 channel slope. This was also found to be useful by Frank et al. (2015) in the first application of the
423 model.

424 For some field studies, applying this two-stage calibration method (inactive vs. active entrainment
425 model) will benefit model users who previously conducted RAMMS runout modeling studies
426 without entrainment. They can enhance their existing calibration procedure of parameter ξ and μ
427 by mainly refining on parameter μ to reflect a documented, relative erosion pattern when activating
428 the entrainment model. In that sense, this method might be primarily limited by the potential lack
429 of field data (flow heights, discharge, erosion patterns) which were available in this study.
430 However, more case studies are needed before we are able to draw any general conclusions
431 regarding potential benefits and limits of this enhanced methodology for the RAMMS entrainment
432 model application.

433 In general, morphological effects influence the erosional behavior of the field data based
434 entrainment model. The Bondasca channel is more variable in width and planform direction
435 compared to the comparably uniform and straight Meretschibach channel. This difference will
436 cause larger spatial variability in shear stress at Bondasca channel and therefore the channel will
437 have a more variable onset of debris flow erosion along the length of the channel. In the Bondasca
438 catchment, the channel where erosion takes place is significantly wider (4-10 m) than in the
439 Meretschibach (1-3 m). On the one hand, the flow can laterally spread more often in Bondasca than
440 in the Meretschibach, thereby locally reducing flow height, shear stresses and maximum potential

441 erosion depth. On the other hand, once the critical shear stress is exceeded, the potential erosion
442 depth tends to increase more rapidly in a narrow channel such as in the Meretschibach channel.

443 Other studies have addressed the spatio-temporal variation of bed entrainment interplaying with
444 debris flow rheology (Cuomo et al., 2014; Cuomo et al., 2016). In RAMMS, we do not adjust the
445 Voellmy friction coefficients as a function of flow properties because data to support the
446 implementation of bed entrainment-flow properties interplay is not available for the catchments
447 addressed herein (Meretschibach and Bondasca).

448 Another difference between the Meretschibach and the Bondasca channels is that the Bondasca
449 channel bed has a rougher surface with more scours holes, and larger blocks within the channel
450 which are similar in size to the nominal width of the channel. The model does not consider local
451 variations in erodibility due to the presence of large blocks, so local scour patterns in the field
452 around the large blocks are not present in the model results. Prancevic and Lamb (2015a) suggested
453 that in rough mountain channels the large particles can be interlocked and hence more stable. In
454 contrast, local concentration of the flow between such large blocks may cause locally very large
455 shear stresses and corresponding large erosion rates. However, we do not have enough information
456 on the mobility of the large blocks, so this question cannot be addressed in more detail herein.

457 The current version of the RAMMS debris flow model with entrainment (version 1.6.45) does not
458 adjust the elevation of the bed when erosion occurs. The erosion can be subtracted from the initial
459 DTM as a post-processing step within the user interface, e.g. for modeling subsequent surges. This
460 issue was discussed at length by Frank et al (2015), and it can potentially complicate the
461 interpretation of erosion patterns resulting from multiple debris flows. Insufficient field data are
462 available to help constrain the events described herein.

463 For the sensitivity assessment of volume growth (VG) to the critical shear stress τ_c , we selected the
464 Meretschibach catchment because it has a simple single-channel morphology and therefore serves
465 as a clear case for illustration (Fig. 8). Because the erosive channel reach addressed in our study
466 shows steep slopes reaching 50 to 65 %, the resulting shear stresses are very high – even for very
467 low flow heights and small initial volumes (1-10 m³). This leads to a high model susceptibility to
468 erosion (volumes) and volume growth when $\tau_c = 0$ kPa which results in scenarios of a few cubic
469 meters of initial volumes eroding some 1,000 to more than 10,000 cubic meters (Fig. 8). However
470 the initial landslides observed at the Meretschibach were larger in volume, suggesting that a critical
471 shear stress is appropriate. Small debris flows do not necessarily erode the channel bed, which has
472 been observed in the field e.g. at the Illgraben (Schürch et al., 2011; Berger et al., 2010). The
473 presence of a critical shear stress in steep channels is also supported by investigations of
474 entrainment in torrential sediment transport (Lamb et al., 2008), although we are not aware of any
475 systematic investigations of the critical shear stress for entrainment by landslides or debris flows.

476 The results show that when exceeding $\tau_c = 0.5$ kPa, the volume growth remains steady within a
477 value range of 20 to 60 for middle to larger initial volumes (≥ 10 -50 m³). Smaller initial volumes (\leq

478 5-10 m³) show much more variation, i.e. are more sensitive to the critical shear stress. We conclude
479 that a value of $\tau_c = 1$ kPa produces plausible results and we use that value for the other sensitivity
480 tests in this study. However it may be possible to constrain this value at other field sites if small
481 non-erosive debris flows can be identified and used to better constrain τ_c . The critical shear stress
482 of $\tau_c = 1$ kPa used herein will be applied for further sensitivity analysis.

483 The sensitivity to initial landslide volume is apparent at the Meretschibach. Using the default
484 erosion rate and an initial volume of 3 m³, a volume growth of ≈ 200 is reached. A maximum of
485 $VG = 300$ is observed for an initial release volume of 4 m³. It then drops to a $VG \approx 30$ for an initial
486 volume of 100 m³. The model simulations using the doubled default erosion rate show a volume
487 growth peak $VG_p \approx 1,800$ for an initial release volume of 2 m³; half the default erosion rate shifts
488 this peak to 50 m³ for the initial volume but the corresponding volume growth peak drops
489 significantly down to $VG_p \approx 14$.

490 The behavior of the volume growth for the default erosion rate at the Bondasca catchment is
491 relatively smooth when compared to that at the Meretschibach. But comparing erosion patterns as
492 modeled using 10 vs. 20 m³ as the initial volume in the Bondasca case e.g., we observed that the
493 model run using 20 m³ is large enough that part of the flow enters a secondary channel. The
494 volume of the flow, then divided among two channels, causes a reduction in flow depth and a
495 consequent decrease in shear stress, resulting in smaller erosion depths and therefore smaller
496 erosion volumes – leading to lower volume growth approaching a value of 1 ($VG \approx 1.2$) for $V_{ini} =$
497 20 m³ compared to $VG \approx 3$ for $V_{ini} = 10$ m³. When using an initial volume of 10 m³ then the flow
498 remains entirely in the main channel. This may provide an explanation for the dip in the Bondasca
499 volume growth between $V_{ini} = 10$ m³ and $V_{ini} = 20$ m³.

500 A volume growth peak can be identified between 200 and 500 m³ but the value is lower in
501 comparison (≈ 10 -12.5) for the default erosion rate. The doubled rate leads to a volume growth
502 peak VG_p of ≈ 700 at a release volume of 30 m³. That is large compared to examples in the
503 literature (VG from 10 to 50 reported by Berti et al., 1999 and Vandine and Bovis, 2002).

504 Nevertheless, a several hundred fold increase of the debris flow volume due to bulking is plausible
505 for extreme entrainment cases. Larger erosion rates might be expected for pyroclastic deposits (not
506 present in the catchments described herein) or due to the presence of very recent rock avalanche
507 deposits which may contain firn-ice-debris mixtures (e.g. Spreitgraben, Tobler et al., 2014; Frank et
508 al., 2015). Such highly erosive events represent an inherent feedback in the entrainment process
509 whereby a rapid (e.g. double) erosion rate results in a more rapid increase in flow depth leading to
510 larger shear stresses and then to even larger potential erosion depths. This can potentially explain
511 the very rapid growth of debris flows, which is has been observed in some natural field cases (e.g.
512 Spreitgraben, Tobler et al., 2014; Frank et al., 2015) and also in laboratory experiments involving
513 realistic debris-flow sediments (e.g. video documentation of experiments at the USGS Debris-flow
514 flume 1992-2006, Logan & Iverson, 2007).

515 In addition, large erodibilities may be expected at the Bondasca catchment because the rock
516 avalanche event occurred during winter and may have contained significant amount of snow.
517 However, due to the very long (≈ 4 km) and flat ($\approx 15\%$) channel section in the middle segment of
518 the Bondasca catchment, the estimated deposition volumes ($\approx 40,000$ m³) above the inlet of the
519 Bondasca river in the central valley are highly influenced by further erosional and depositional
520 processes along the channel.

521 **6. Conclusion**

522 Debris-flow runout predictions can be improved when considering the increase in flow volume
523 along the flow path. Using a recently-introduced empirical entrainment algorithm within the
524 RAMMS 2D runout model (Frank et al., 2015) we illustrate that runout patterns at the
525 Meretschibach and Bondasca catchments, in Switzerland, can be accurately modeled. When
526 calibrated with field data, the model produces more realistic runout patterns compared to
527 simulations which do not consider entrainment and bulking. In particular, we could show that even
528 in very steep (≈ 60 – 65%) and narrow (4–6 m) torrent channels, lateral overflow – not observed in
529 the field case – is prevented when applying the entrainment model. However the model results can
530 be quite sensitive to the volume of the initial block release in the model which corresponds to the
531 initial landslide volume. The predicted erosion volumes are sensitive to the initial debris flow
532 volume, with volume growth values approaching 2000 predicted by the model, depending on the
533 scenario considered. However, the results are also sensitive to slope angle and channel
534 morphology. The two field sites differ substantially: the Meretschibach catchment is very steep
535 with a straight and narrow channel, whereas the Bondasca channel is less steep and
536 morphologically more complex, yet the calibration procedure is the same as for the standard
537 RAMMS model which does not include the entrainment process. The overall method presented
538 herein is useful for case studies where sufficient data are available to constrain the model results.
539 However, more case studies have to be conducted to develop a more comprehensive
540 recommendation for modeling the runout of erosive debris flows in natural terrain.

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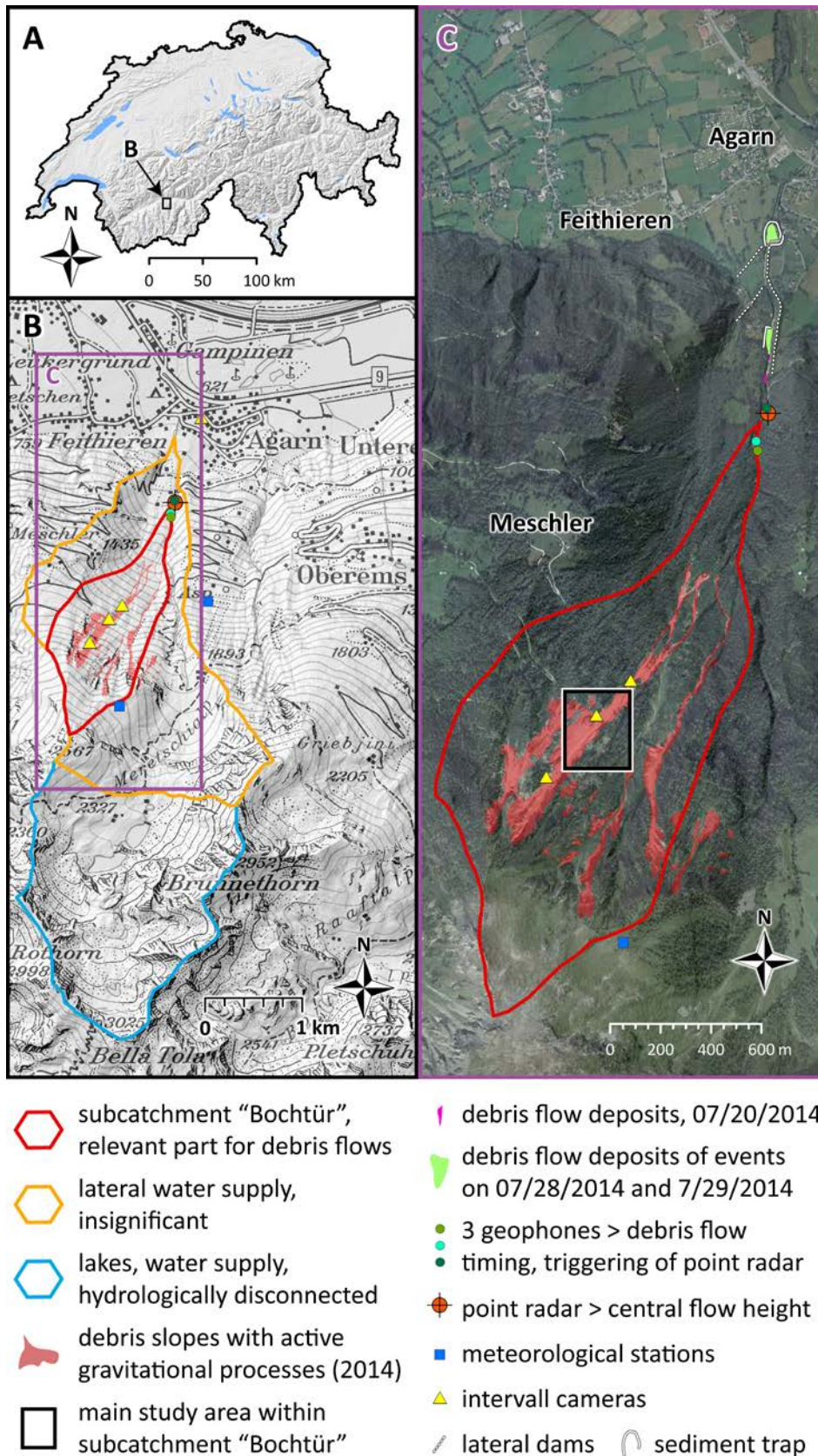
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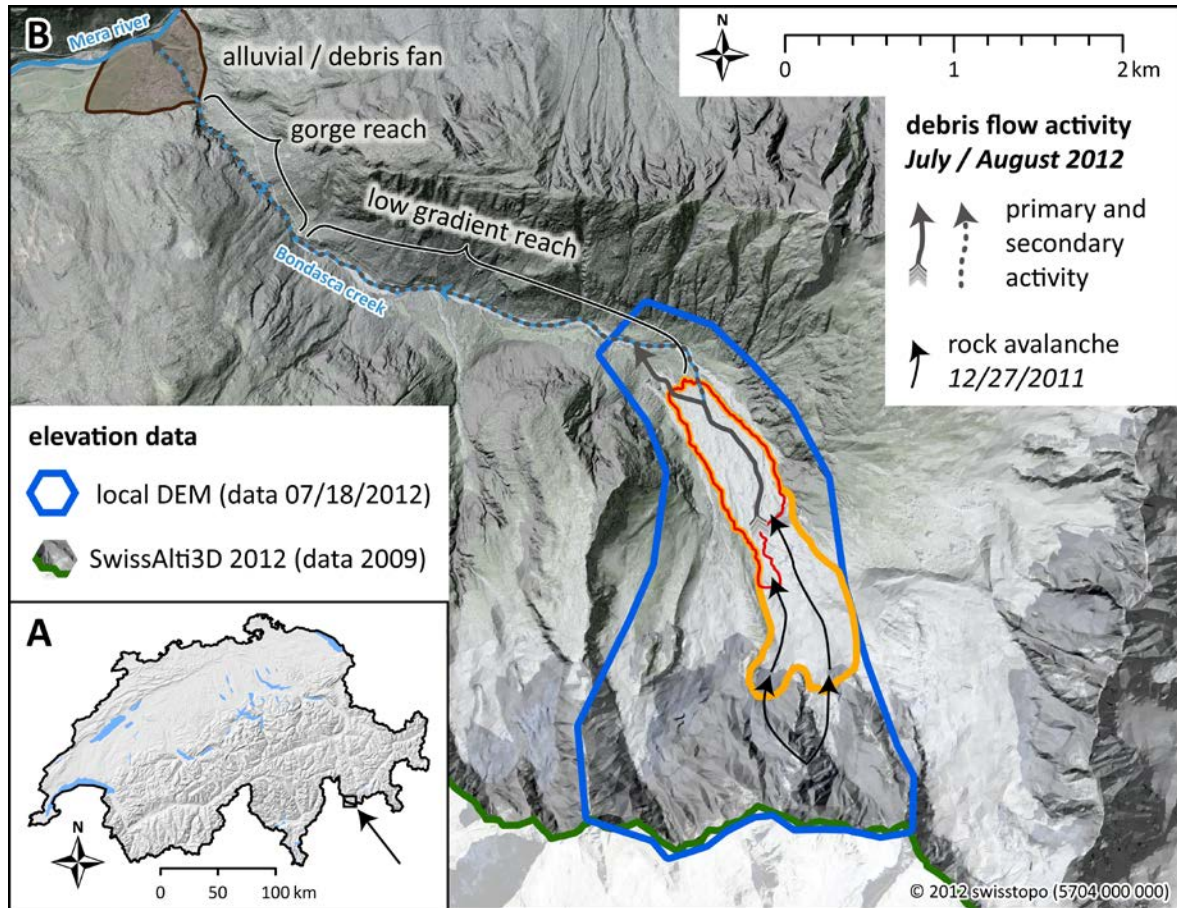
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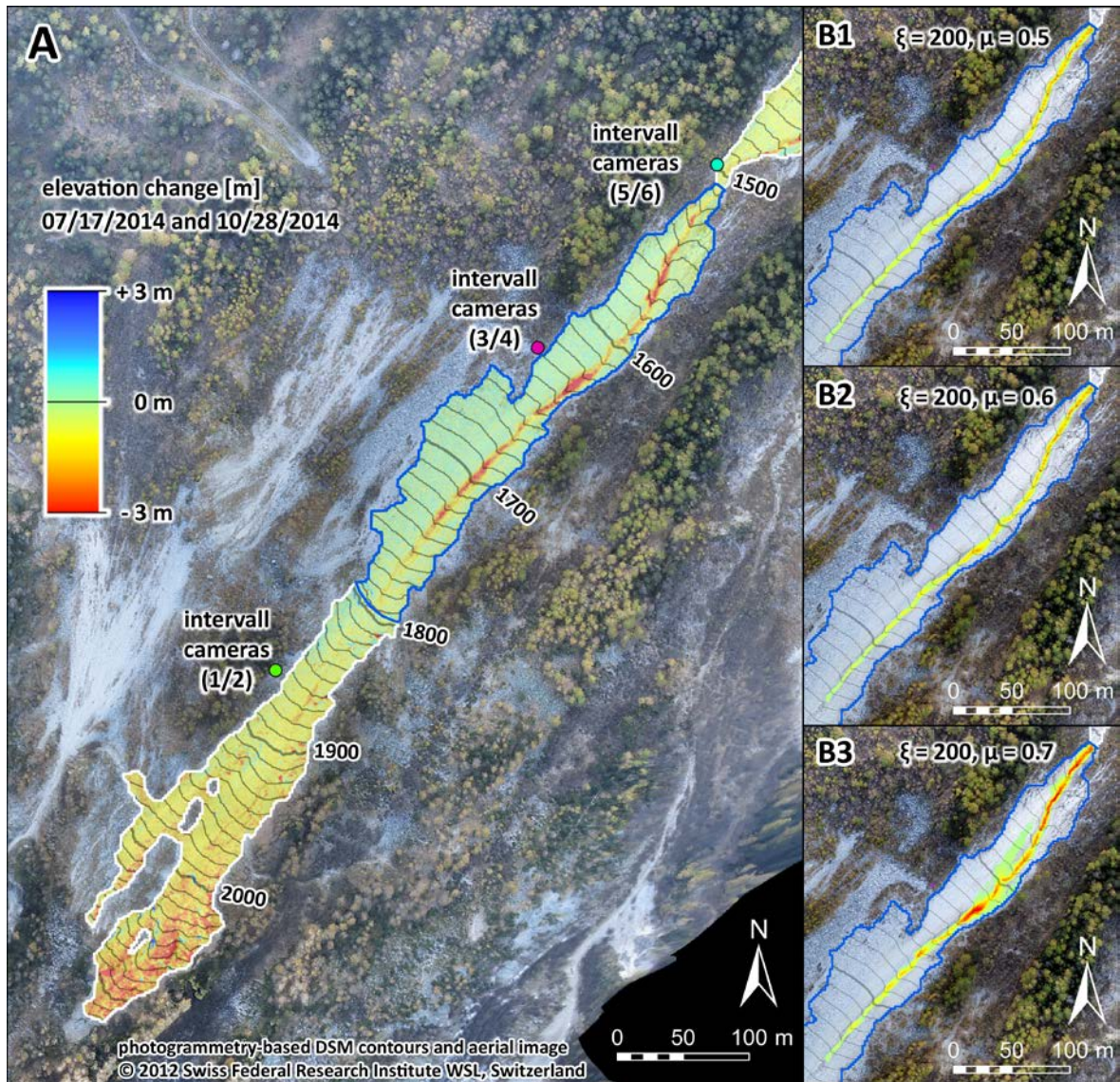
729 **Figure 1.** A. Location of the Meretschibach catchment in Southern Switzerland. B. Subcatchments
 730 of the Meretschibach and locations of the instrumentation site and data available for the erosion
 731 model analyses C. Initiation zone of the July 2014 events and camera positions. The main study

732 channel reach for the model testing is located in the middle part of “Bochtür” (black-white
733 rectangle), swissimage©2014, swisstopo (5704 000 000) (2014).

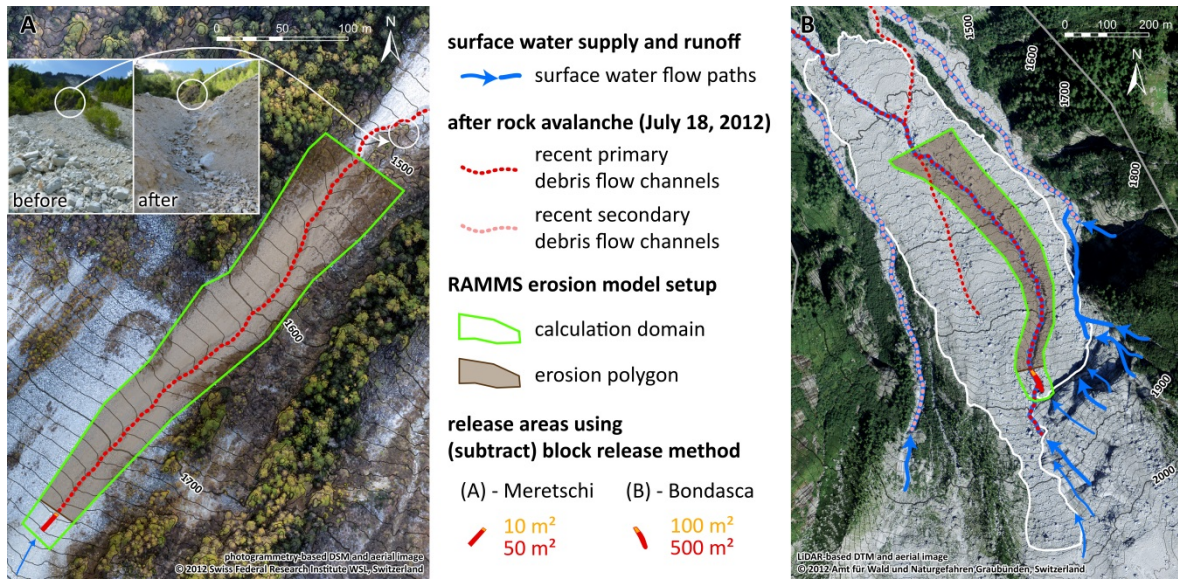


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735 **Figure 2.** A. Location of the Bondasca catchment in south-eastern Switzerland close to the border
736 to Italy. B. Perimeter of the 27 December 2011 rock avalanche deposit, including the main
737 deposition area (yellow polygon) and the deposits lower-elevation deposits which have been
738 partially exposed to erosion by debris flows in 2012 (red polygon). The 2012 post-event digital
739 elevation model (lidar, blue polygon) is from 18 July 2012 (data courtesy of the Amt für Wald,
740 Canton Graubünden). Pre-event digital elevation model (lidar) for 2009 is from the SwissAlti3D
741 (version 2012) data set from swisstopo, ©2012, swisstopo (5704 000 000) . The grey solid arrow
742 indicates the main debris-flow channel formed in 2012.

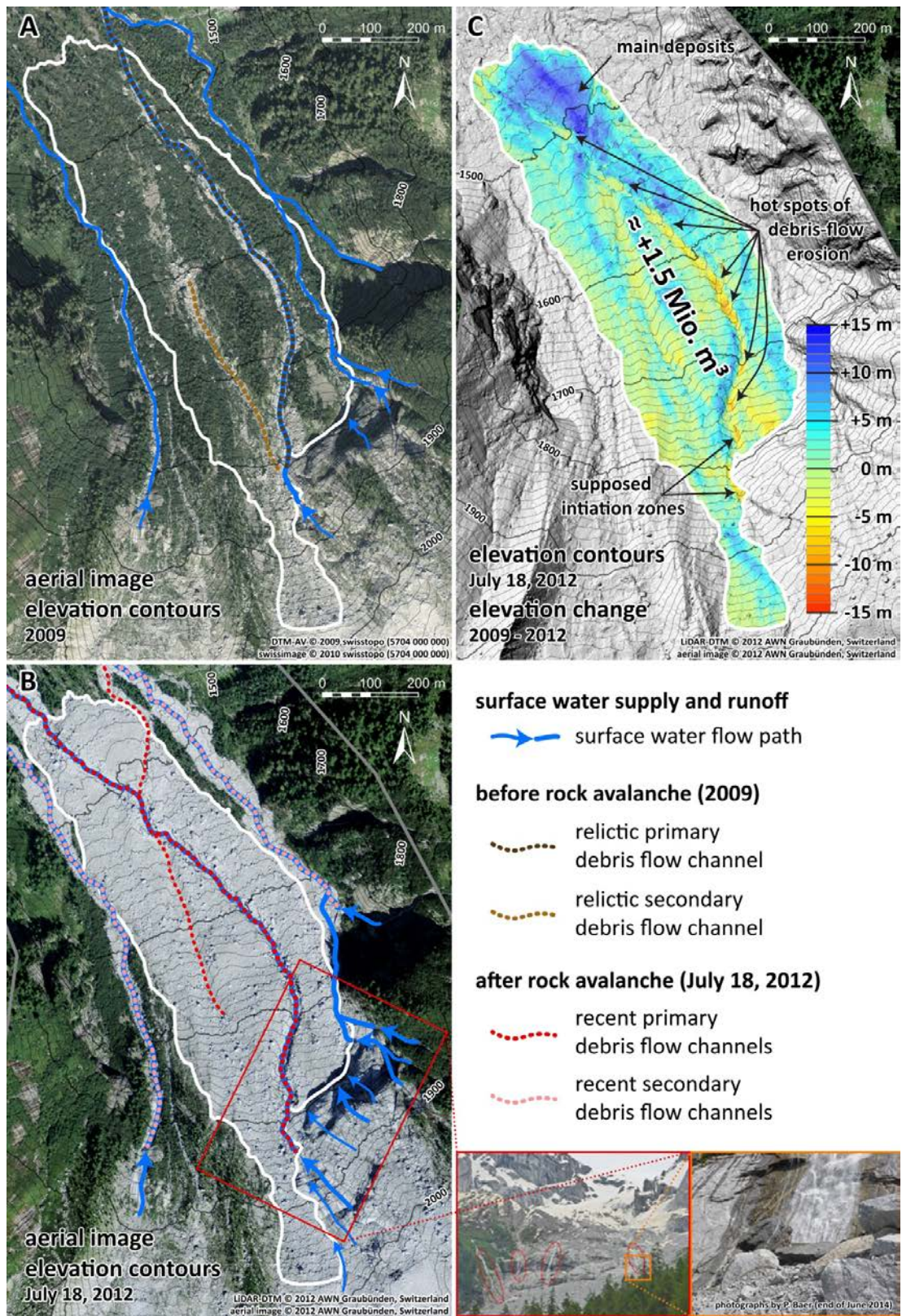


744 **Figure 3.** Calibration of modelled erosion patterns (**B1 to B3**) to the observed erosion depths (**A**) in
 745 the upper open debris slopes of the “Bochtür” catchment (Meretschibach) by varying values for the
 746 friction parameter μ . The blue polygon demarks the area where a differential DTM is available.



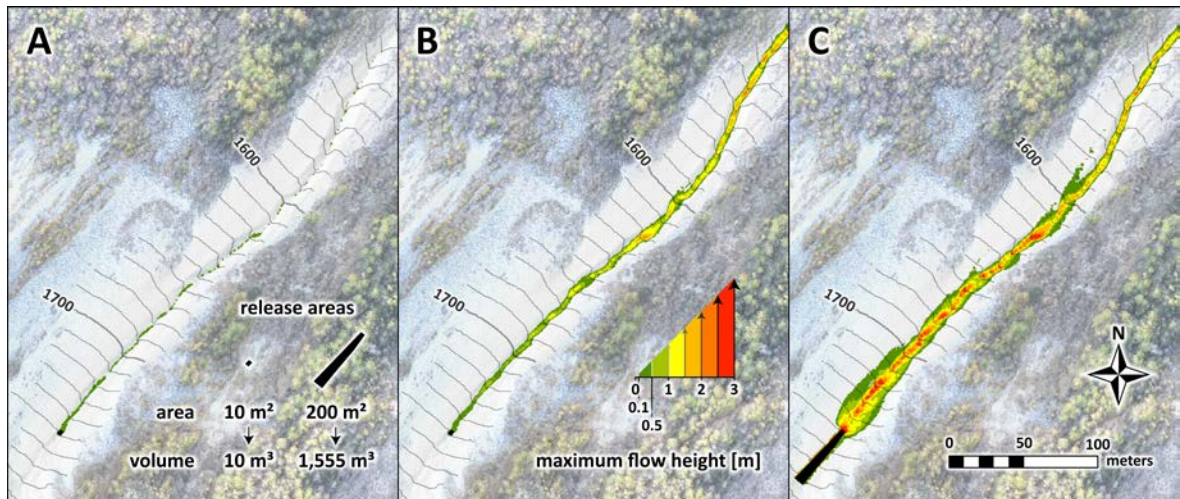
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748 **Figure 4.** Erosion model configuration for the model simulations showing the initial block release
 749 areas in the Meretschibach catchment (A) and the Bondasca catchment, Switzerland (B). The
 750 hillslope is erodible within the brown shaded polygon.



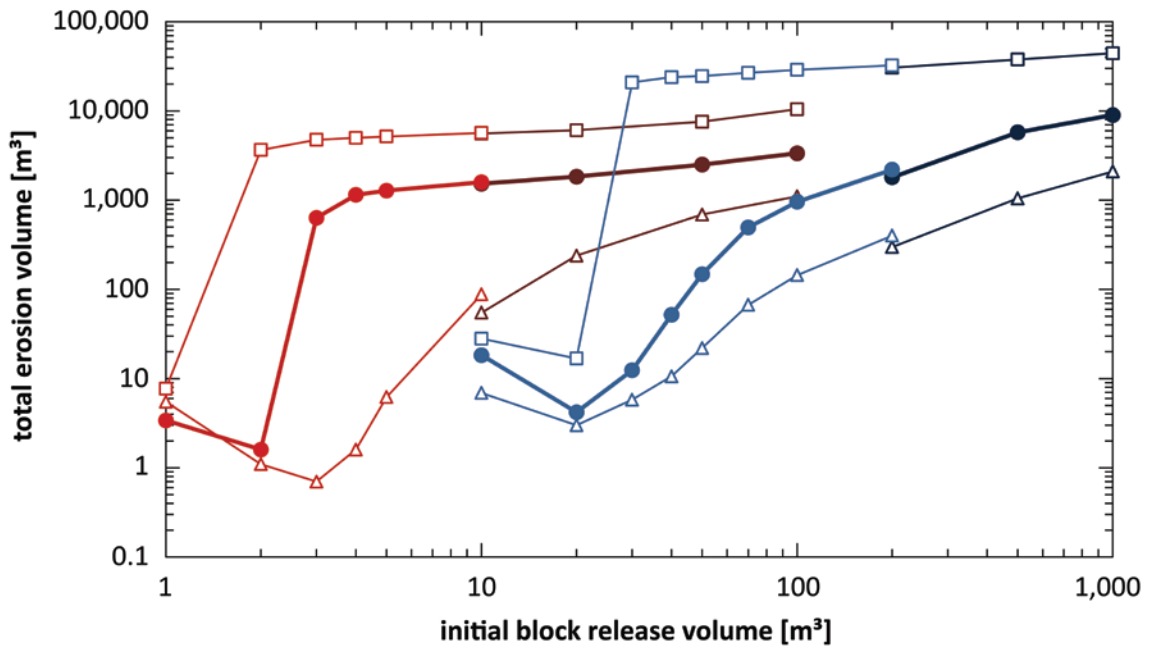
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752 **Figure 5.** Overview of rock avalanche deposits, subsequently formed debris flow channels, and the
 753 resulting overall elevation change in the Bondasca catchment (A, B). The elevation change map
 754 2009 to 2012 (C) includes both the rock avalanche ($\approx 1.5 \text{ Mio m}^3$ on 27 Dec. 2011) and the first
 755 two debris-flow events (5 and 14 July 2012).



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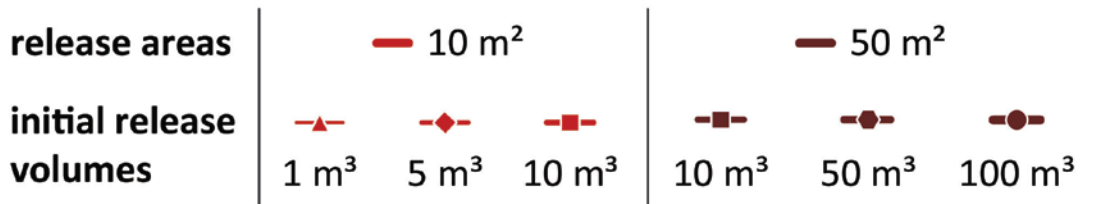
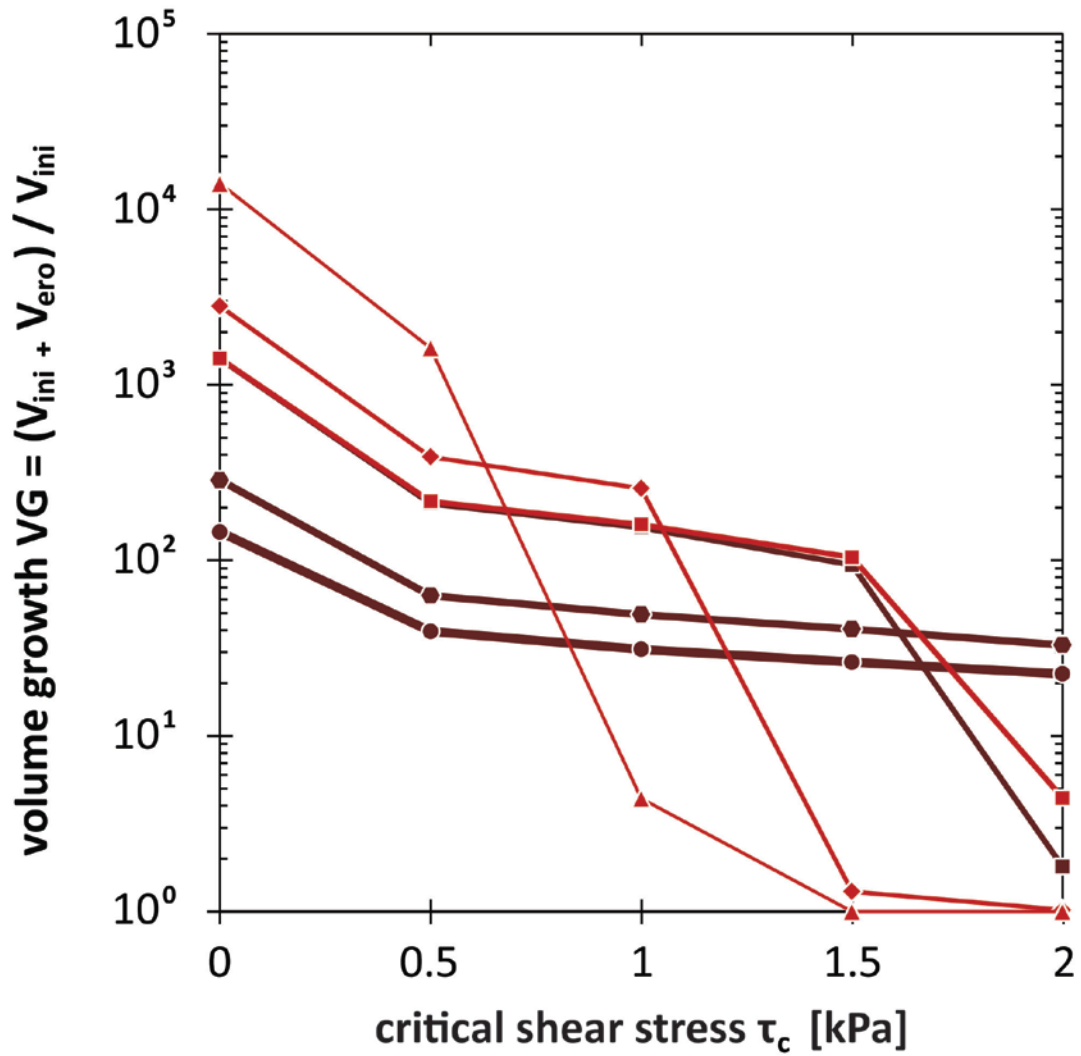
757 **Figure 6.** Comparison of runout patterns at “Bochtür” in the Meretschi catchment. The debris flow
 758 modeling is conducted using a (subtract) block release volume of (A) 10 m^3 and no-entrainment
 759 modeling, of (B) 10 m^3 and entrainment modeling as well as a total (subtract) block release volume
 760 of (C) $1,555 \text{ m}^3$ (sum of release and eroded volume from (B)) and no-entrainment modeling.



	Meretschi	Bondasca
calibrated parameters	$\xi = 200 \text{ m}^2\text{s}^{-1}, \mu = 0.60$	$\xi = 400 \text{ m}^2\text{s}^{-1}, \mu = 0.30$
release areas	— 10 m ² — 50 m ²	— 100 m ² — 500 m ²
erosion rates <i>(Frank et al., 2015)</i>	—△— 1.25 cm s ⁻¹ ● 2.5 cm s ⁻¹	—□— 5.0 cm s ⁻¹

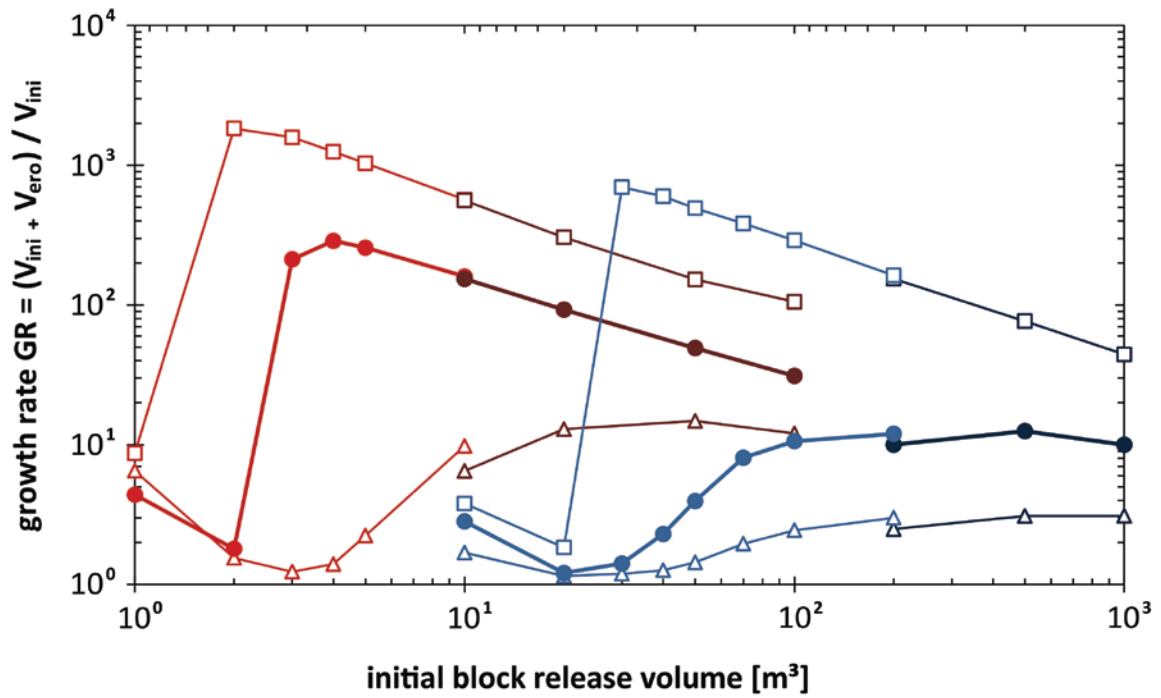
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762 **Figure 7.** Sensitivity of modeled erosion volume to initial block release volume in the
 763 Meretschibach and in the Bondasca catchments.



764

765 **Figure 8.** Sensitivity of the volume growth $VG = (V_{ini} + V_{ero}) / V_{ini}$ to the critical shear stress τ_c
 766 depending on 5 different initial (block release) volumes V_{ini} as set up based on two release areas in
 767 the Meretschibach catchment.



	Meretschi	Bondasca
calibrated parameters	$\xi = 200 \text{ m}^2\text{s}^{-1}, \mu = 0.60$	$\xi = 400 \text{ m}^2\text{s}^{-1}, \mu = 0.30$
release areas	— 10 m ² — 50 m ²	— 100 m ² — 500 m ²
erosion rates (Frank et al., 2015)	—△— 1.25 cm s ⁻¹ ●— 2.5 cm s ⁻¹	—□— 5.0 cm s ⁻¹

768

769 **Figure 9.** The volume growth $VG = (V_{ini} + V_{ero}) / V_{ini}$ consisting of the sum of the erosion volume
770 V_{ero} [m³] and initial block release volume V_{ini} [m³] per initial block release volume V_{ini} [m³] and
771 addressing three different erosion rates for the Meretschibach and Bondasca catchments.