



- 1 Operative and reliable landslide forecasting and influence of
2 geology to predictability
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9 **ABSTRACT**

10 Forecasting a catastrophic collapse is a key element in landslide risk reduction, but also a very
 11 difficult task, owing to the scientific difficulties in predicting a complex natural event and also to
 12 the severe social repercussions caused by a false or a missed alarm. A prediction is always
 13 affected by a certain error, however when this error can imply evacuations or other severe
 14 consequences a high reliability in the forecast is, at least, desirable.

15 In order to increase the confidence of predictions, a new methodology is here presented.
 16 Differently from traditional approaches, it iteratively applies several forecasting methods based
 17 on displacement data and, also thanks to an innovative data representation, gives a valuation of
 18 how the prediction is reliable. This approach has been employed to back-analyse 15 landslide
 19 collapses. By introducing a predictability index, this study also contributes to the understanding
 20 of how geology and other factors influence the possibility to forecast a slope failure. The results
 21 showed that, contrarily to what is generally believed, geomechanics plays an indirect role in
 22 landslide predictability; instead kinematics, and all the factors influencing it, is the key feature.

23 *Keywords: landslides; forecasting; geomechanics; early warning; time of failure; slope failure*
 24

25 **INTRODUCTION**

26 Natural disaster forecasting for early warning purposes is a field of study that drew the media
 27 attention after events such as the 26th December 2004 tsunami of Sumatra. Predicting landslides,
 28 with respect to other natural hazards, is a complex task due to the influence of many factors like
 29 geomechanical properties, rainfall, ground saturation, topography, earthquakes and many others.
 30 So far, few empirical landslide forecasting methods exist (Azimi et al., 1988; Fukuzono, 1985a;
 31 Mufundirwa et al., 2010; Saito, 1969; Voight et al., 1988) and none furnishes a reliability degree
 32 about the prediction, making them unsuitable for decision making.

33 In our research we present an approach to perform probabilistic forecasting of landslides
 34 collapse. This has been achieved by reiterating several predictions using more forecasting
 35 methods at the same time on multiple time series. This approach may have important
 36 applications to civil protection purposes as it provides the decision makers with a level of
 37 confidence about the prediction. Furthermore, this study, performed on 15 different case studies,
 38 shows how the possibility or not to forecast the time of collapse of a landslide is not affected by
 39 geomechanical or geomorphological features, like usually believed, as much as by circumstantial
 40 conditions.

41 **The inverse velocity forecasting method**

42 Forecasting activity can be considered the fulcrum of early warning systems (Intrieri et al.,
 43 2013), i.e. cost-effective tools for mitigating risks by moving the elements at risk away. For
 44 many natural phenomena forecasting is common practice (for example for hurricanes;
 45 Willoughby et al., 2007), while for others is, at present, impossible (earthquakes; Jordan et al.,
 46 2011). Landslides lie in between. Their prediction is usually performed through rainfall
 47 thresholds (Baum and Godt, 2010), but a more reliable approach should make use of direct
 48 measures of potential instability, such as displacements (Lacasse and Nadim, 2010). A first issue
 49 is that only a small percentage of landslides in the world is appropriately monitored. This also
 50 causes an insufficient knowledge of the geomechanical processes leading to failure, which is
 51 another responsible for our deficiencies in predicting landslides.

52 In spite of this, few empirical methods for predicting the time of failure based on movement
 53 monitoring data have been developed (Azimi et al., 1988; Fukuzono, 1985a; Mufundirwa et al.,
 54 2010; Saito, 1969) and further investigated on a physical basis (Voight et al., 1988). They are all



55 based on the hypothesis that if a landslide follows a peculiar time-dependant geomechanical
 56 behaviour (called creep; Dusseault and Fordham, 1994), it will display a hyperbolic
 57 acceleration of displacements before failure; by extrapolating this trend from a displacement time
 58 series through empirical arguments, it is possible to obtain the predicted time of failure. However
 59 such methods do not always produce good results.

60 One of the most famous methods is Fukuzono's (1985a), which derives from Saito's (1969),
 61 from here on simply called F and S method, respectively. It requires that during the acceleration
 62 typical of the final stage of the creep (tertiary creep), the inverse of displacement velocity (v^{-1})
 63 decreases with time. The collapse is forecasted to occur when the extrapolated line reaches the
 64 abscissa axis (corresponding to a theoretical infinite velocity). Such line may either be convex,
 65 straight or concave (Fukuzono, 1985a). When it is straight this phenomenon is sometimes
 66 referred to as Saito effect (Petley et al., 2008).

67 The possibility to find landslides showing the Saito effect has been related to the mechanical
 68 properties of the sliding mass. However there is no general consensus on this issue.

69 According to some authors (Petley, 2004; Petley et al., 2002), in order to display the Saito effect,
 70 landslides need to display a brittle behaviour (which indicates a drop from peak strength to
 71 residual strength value, deformation which is concentrated along a well defined shear surface,
 72 sudden movements and catastrophic failure, usually associated with crack formation in strong
 73 rocks); furthermore only brittle, intact rocks evolve in catastrophic landslides and therefore can
 74 be predicted; for others (Rose and Hungr, 2007), on the opposite, landslides displaying the Saito
 75 effect must have ductile failures in order to be forecasted (i.e. slower, indefinite deformation
 76 along a shear zone and under a constant stress, typical of sliding on pre-existing surfaces of soft
 77 rocks), as brittleness is characterized by sudden, impossible to anticipate, ruptures.

78 This complex subject is made even more difficult due to the influence of external factors
 79 (rainfall, earthquakes, excavations), structural constraints (joints, faults, contacts with different
 80 lithologies) and sometimes unknown elements within the mass (the conditions of the shear
 81 surface, the history of the landslide, the presence of rock bridges). Therefore it is often hard to
 82 establish the mechanical behaviour and even more to find an exact correlation between the
 83 mechanical behaviour of a landslide and the possibility to predict its failure.

84 **The concept of predictability**

85 Before assessing the influence of geomechanics on the predictability of a landslide it is first
 86 necessary to address the concept of predictability. In literature (Azimi et al., 1988; Hutchinson,
 87 2001; Mufundirwa et al., 2010; Rose and Hungr, 2007) there are papers that deal with
 88 "predictions" made in retrospect, that is thorough post-event analyses showing the signs of a
 89 critical pre-collapse acceleration; however whether such signs would have been unambiguous or
 90 would have granted a sufficient lead time is often neglected. On the other hand in our research
 91 we consider an operational definition of predictability (integrating the one of early warning
 92 system; UNISDR, 2009) as the feature possessed by a landslide which allows one to forecast its
 93 collapse with reasonable confidence and sufficiently in advance, permitting the dispatch of
 94 meaningful warning information to enable individuals, communities and organizations
 95 threatened by the hazard to prepare and to act appropriately and in sufficient time to reduce the
 96 possibility of harm or loss. Therefore, displaying the Saito effect is not the only prerequisite for
 97 an operational prediction, there is also the need for repeated time of failure forecasts fluctuating
 98 around a constant time value placed not too close in the future. This has been achieved through
 99 the reiterative approach and the graphical representation described in the following paragraph.

100 **METHODS**



101 The usual way to apply landslide forecasting methods based on displacements, is to obtain a
 102 single predicted time of failure (t_f) and to update such prediction as soon as new data are
 103 gathered (Rose and Hungr, 2007). This is a deterministic approach, since the real time of failure
 104 (T_f) is predicted through a single inference.

105 On the other hand, in order to account for the uncertainty of the methods and complexity of the
 106 phenomena, predictions should have a certain confidence (for example given by the standard
 107 deviation of t_f). This is especially important for operative early warning systems. We achieved
 108 this probabilistic approach by reiterating the equations from Saito (1969), Fukuzono (1985a) and
 109 Mufundirwa et al. (2010) for finding t_f , using continuously new data. The latter method will be
 110 called M method from here on.

111 The predictions are plotted versus the time when they have been made (time of prediction, t_p).
 112 We call these diagrams prediction plots (Figure 1). A prediction is considered reliable when the
 113 inferences oscillate around the same t_f . Figure 1 also shows that since reliable predictions usually
 114 display an oscillatory trend, the most updated one is not necessarily the most accurate, contrarily
 115 to what is usually believed (Rose and Hungr, 2007) in fact, the length of the dataset is more
 116 important, from which T_f can be estimated through simple statistical analyses (like mean and
 117 standard deviation).

118 Since in some cases a single forecasting method can fail to give satisfactory results, in order to
 119 improve even more the confidence in the predictions, a multi-model approach is adopted together
 120 with the probabilistic approach. In fact, according to the Diversity Prediction Theorem (Page,
 121 2007; Hong and Page, 2008), diversity in predictive models reduces collective error. The highest
 122 confidence, of course, is reached when all the employed method independently converge towards
 123 the same result. For this research we confronted the results from S and F methods and from the
 124 method by Mufundirwa et al. (2010). The equations used for the iteration are obtained from the
 125 respective authors and are:

$$126 \quad t_r = \frac{t_2^2 - (t_1 \cdot t_3)}{2t_2 - (t_1 + t_3)}, (1)$$

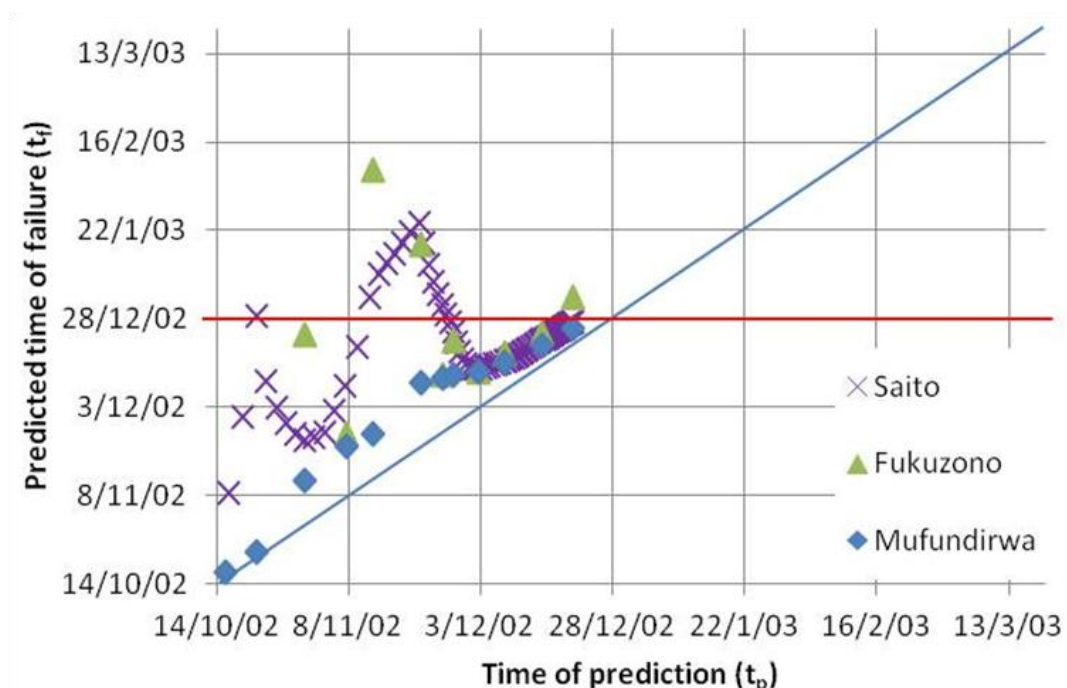
127 for S method, where t_1, t_2, t_3 are times taken so that the displacement occurred between t_1 and t_2
 128 is the same as between t_2 and t_3 .

$$129 \quad t_r = \frac{t_2 \frac{1}{v_1} - t_1 \frac{1}{v_2}}{\frac{1}{v_1} - \frac{1}{v_2}}, (2)$$

130 for F method, where v_1 and v_2 are the velocities at arbitrary times t_1 and t_2 .

$$131 \quad t \frac{dD}{dt} = t_r \frac{dD}{dt} - B, (3)$$

132 for M method, where D is the displacement and t_r is the angular coefficient of the line
 133 represented in a $t \frac{dD}{dt} = f\left(\frac{dD}{dt}\right)$ space having B as the intercept.



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135 **Figure 1.** This graph represents probabilistic predictions performed with 3 different forecasting
 136 methods (Fukuzono, 1985a; Mufundirwa et al., 2010; Saito, 1969) applied to the MB34-35'
 137 displacement time series of Mount Beni landslide (Gigli et al., 2011). The black dashed line
 138 indicates the observed time of failure (T_f) and the grey diagonal line the equality between t_f and
 139 t_p . Therefore the vertical distance between a point and the black dashed line indicates the
 140 prediction error. The vertical distance between the blue line and a prediction above it is the life
 141 expectancy of the landslide at the time of prediction. In this case the predictions obtained through
 142 S and F methods give a good estimation of T_f , while the one from Mufundirwa et al. (2010)
 143 consistently forecasts the collapse few days ahead.

144 TIME OF FAILURE PREDICTION

145 In order to find a relation between the predictability of a failure and the geological features of the
 146 landslide, S, F and M methods have been applied to a number of different real case studies. Some
 147 geological features of interest relative to such cases are reported in TABLE 1, when they were
 148 known or applicable.

149 A few representative examples of prediction plots are showed in Figure 1 and Figure 2. Mount
 150 Beni landslide is a 500.000 m³ topple that evolved as a rockslide (Gigli et al., 2011). It developed
 151 on a slope object of quarrying activity. The predictions oscillate quite regularly around the
 152 observed time of failure (T_f , black dashed line in Figure 2). It is this convergence that permits to
 153 correctly forecast the collapse a priori at least since late November, i.e. a month before the
 154 failure. The three methods are similar to the point that S and F previsions can be partially
 155 overlapped. M previsions overlap as well but only in the final part. The M method alone would



not be sufficient for spreading a reliable alarm as the single forecasts do not converge but move forward to a different time of failure as the time passes by. Similar behaviours can be observed also for the cases of Figure 2 that display landslides with a different array of geological features (as seen in TABLE 1). The best results are obtained when the forecasts oscillate around T_f with sufficient time in advance (as for Vajont and, limited to F method, for Liberty Pit) or when they consistently give the similar t_f (as for the artificial landslide E). In other cases (Avran valley and, limited to S and M method, for Liberty Pit) the predictions are too scattered or simply never converge toward a single result, thus making it impossible to foresee a reliable time of failure.

The results of the prediction plots can be roughly summarized reporting the mean and standard deviation of the forecasts for each method (Figure 3).

TABLE 1. LANDSLIDE CASE HISTORIES

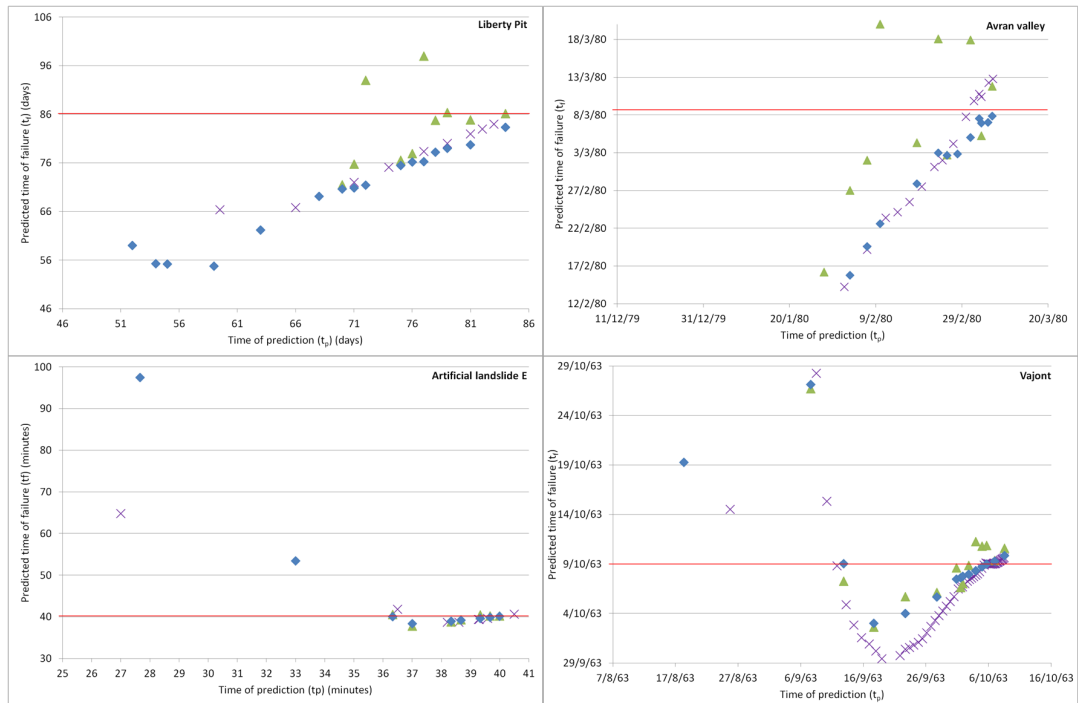
Name	Material	Type	Brittleness	Volume (m ³)	Predisposing factor	Trigger	History	Basal geometry	Ref. *
Liberty Pit	Weathered quartz monzonite	Rockslide?	Medium/high	6x10 ⁶	N.D.	Blasts, pore water pressure	First time failure	Planar?	1, 2
Landslide in mine	Consolidated alluvial sediments, weathered bedrock	Deep-seated toppling in bedrock	Medium	10 ⁶	Blasts, pore water pressure	N.D.	First time failure?	N.D.	1
Betze-Post	Weathered granodiorite	Rockslide?	Medium/high	2x10 ⁶	N.D.	Rainfall	First time failure?	Wedge intersections?	1
Vajont	limestone and clay	Rock avalanche	High	2.7x10 ⁸	N.D.	Pore water pressure	Reactivated	Concave	1, 3
Stromboli †	Shoshonitic basalts	Bulging (not a landslide)	Medium/high	N.D.	N.D.	Sill intrusion	First time failure	N.D.	4
Monte Beni	Ophiolitic breccias	Topple/rock slide	High	5x10 ⁵	Rainfall, structure, basal excavation	N.D.	First time failure	Stepped	5
Cerzeto	Weathered metamorphic rocks on top, cataclastic zone and Pliocene clays	Debris slide-earth flow	Medium/low	5x10 ⁶	Tectonized area, permeability differences	Prolonged rainfalls	Reactivated ?	Compound (steeper and irregular in the upper zone and gentler in the clays)	6
Rock mass failure Japan	Clayey limestone	Rockslide?	High (within limestone)?	5x10 ²	“Structural complexity” (?)	Intense rainfall	First time failure?	Planar?	7
Asamushi	Liparitic tuff, jointed and weathered. Clay in the joints		Medium/low	10 ⁵	N.D.	N.D.	N.D.	Concave?	7, 8
Avran valley	Chalk	Rockslide	Medium/low	8x10 ⁴	N.D.	N.D.	First time failure?	Convex	9
Giau Pass	Morainic material	Complex slide	Medium/low	5x10 ⁵	N.D.	Pore water pressure	Preexisting shear surface	Composite	10, 11
Artificial landslide A	Loam	Earth slide	Low	N.D.	N.D.	Prolonged rainfall	First time failure	Planar	12
Artificial landslide B	Sand	Earth slide	Low	N.D.	N.D.	Prolonged rainfall	First time failure	Planar	12



Artificial landslide C	Sand	Earth slide	Low	N.D.	N.D.	Prolonged rainfall	First time failure	Convex	12
Artificial landslide D	Sand	Earth slide	Low	N.D.	N.D.	Prolonged rainfall	First time failure	Planar	12

*The references used are numbered as follows: 1: Rose and Hungr, 2007; 2: Zavodni and Broadbent, 1980; 3: Semenza and Melidoro, 1992; 4: Casaghi et al., 2009; 5: Gigli et al., 2011; 6: Iovine et al., 2006; 7: Mufundirwa et al., 2010; 8: Saito, 1969; 9: Azimi et al., 1988; 10: Petley et al., 2002; 11: Angeli et al., 1989; 12: Fukuzono, 1985b.
† The case of Stromboli is not relative to a landslide, rather to a volcanic bulging preceding a vent opening that was forecasted in a similar fashion of a landslide and therefore here included.

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Figure 2. These graphs show how iterating forecasts performed through multiple forecasting methods increases the confidence when estimating the actual time of failure (T_f , black dashed line). The purple crosses represent forecasts performed with S method, the green triangles with F method and the blue squares with M method. Note that F forecasts for Avran valley landslide include other less accurate values not showed in the graph as they are out of scale.

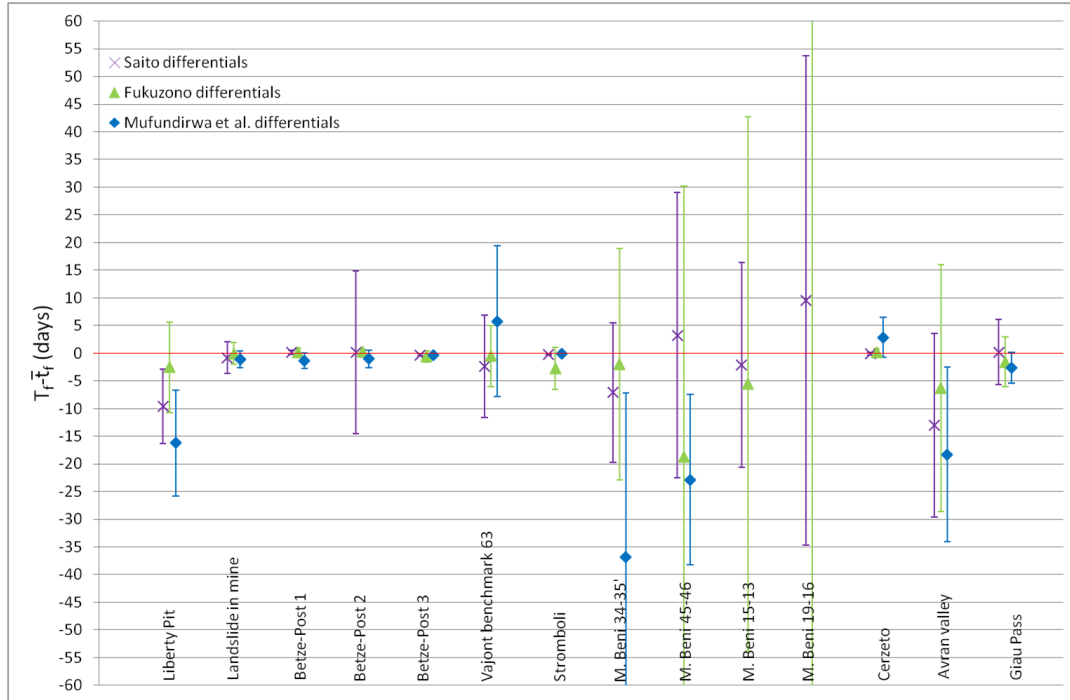


Figure 3. This graph represents for each method the differential between the mean of the forecasts (\bar{t}_f) and the actual time of failure (T_f). Negative values are safe predictions as anticipate the time of failure. The standard deviations of the forecasts are represented as error bars. For Betze-Post and Mount Beni landslides, time series from different measuring points are reported. The rock mass failure, Asamushi landslide and the artificial landslides are not shown as were monitored in a different time scale (hours or minutes).

PREDICTABILITY INDEX

In order to evaluate the performance of S, F and M methods and to relate it to the characteristics of the reported examples, an arbitrary scoring system has been implemented and attributed to each prediction plot (considering that every time series has a prediction plot for each forecasting method and that for some case studies more than one time series was available). This permits to quantify the predictability of a collapse based on the prediction plot. A score from 1 to 5 has been assigned according to the following criteria:

- 1 point: the prediction plot never converges on a single t_f (typically t_f increases at every new datum available).
- 2 points: the predictions vary considerably at every new iteration. An average time of failure (\bar{t}_f) can be extracted but with high uncertainty.
- 3 points: the predictions oscillate around T_f , although with a certain variance.
- 4 points: the predictions have a low variance although \bar{t}_f is slightly different than T_f . Note that when the variance was low, \bar{t}_f and T_f never differed greatly.
- 5 points: the prediction plot is clearly centred on T_f therefore the reliability of \bar{t}_f is high.



198 By summing the scores obtained from S, F and M prediction for each time series, what we call
 199 the Predictability Index (P_i) is obtained (TABLE 2). By using 3 forecasting methods, P_i ranges
 200 from 3 (impossible to predict the time of failure) to 15 (the time of failure can be predicted in
 201 advance and with a high reliability). Though a certain degree of subjectivity is unavoidable when
 202 assigning the scores, what matters here is the relative difference of P_i between the case studies.
 203 In such a way it is possible to understand in which conditions a landslide is more or less
 204 predictable.

TABLE 2. PREDICTABILITY INDEX

Name	S	F	M	P_i	Inverse velocity trend	Notes
Liberty Pit	1	5	1	7	Asymptotic (linear at the end)	Open pit mine, structural control of 2 intersecting faults
Landslide in mine	5	5	5	15	Linear	Open pit mine
Betze-Post 1	3	3	1	7	Linear	Open pit mine
Betze-Post 2	4	5	4	13	Linear	Open pit mine
Betze-Post 3	5	4	1	10	Linear	Open pit mine
Vajont benchmark 63	5	5	5	15	Linear	Air pressure and cementation caused catastrophic collapse
Stromboli	1	2	2	5	Asymptotic	Volcanic context
Mount Beni 12-9	4	5	1	10	Concave	Back fracture
Mount Beni a'b'	1	3	1	5	Linear	Short time series
Mount Beni 15-13	5	3	1	9	Linear	Internal fracture
Mount Beni 34-35'	5	3	1	9	Linear	Lateral fracture, short time series
Mount Beni 45-47	2	3	1	6	Linear	Back fracture, short time series
Mount Beni 3-2	5	2	1	8	Concave	Back fracture
Mount Beni 4'-6	1	4	1	6	Linear	Back fracture, short time series
Mount Beni 24-23	4	2	1	7	Linear	lateral fracture
Mount Beni 49-24	5	1	1	7	Linear	Lateral fracture, short time series
Mount Beni 35'-36	2	5	1	8	Linear	Lateral fracture, short time series
Mount Beni 33-35'	3	3	1	7	Linear	Lateral fracture, short time series
Mount Beni 36-37	4	3	1	8	Linear	Lateral fracture
Mount Beni 19-16	2	2	1	5	Linear	Lateral fracture
Mount Beni 19-17	1	2	1	4	Linear	Lateral fracture, short time series
Mount Beni 33-34	4	2	1	7	Linear	Internal fracture
Mount Beni 43-44	3	2	1	6	Asymptotic (constant velocity at the end)	Internal fracture, short time series
Mount Beni 40-41	3	2	1	6	Asymptotic (constant velocity at the end)	Internal fracture, short time series
Mount Beni 40-42	3	3	1	7	Linear	Internal fracture, short time series
Mount Beni 45-46	3	2	2	7	Linear	Back fracture, short time series
Mount Beni 1-2	4	2	1	7	Linear	Back fracture
Cerzeto	5	5	1	11	Linear	N.A.
Rock mass failure Japan	2	2	1	5	Convex	Open pit mine, very small landslide
Asamushi	5	3	1	9	Linear	N.A.
Avran valley 5	1	2	1	4	Concave	N.A.
Avran valley 6	1	1	1	3	Asymptotic	N.A.
Avran valley 7	1	2	1	4	Concave	N.A.



Giau Pass	3	3	1	7	Asymptotic /concave	N.A.
Artificial landslide A	5	5	5	15	Convex	40° artificial slope
Artificial landslide B	2	2	3	7	Concave	40° artificial slope
Artificial landslide C	1	2	3	6	Linear (slightly convex)	40° artificial slope
Artificial landslide D	5	5	5	15	Linear	30° artificial slope

DISCUSSION

TABLE 2 shows how the most predictable events ($P_i > 8$) can display very different features and are quite irrespective of the shape of the inverse velocity plot, the volume, the brittleness of the material, the history of the landslide and so on (see also TABLE 1).

A comparison between Figure 3 and TABLE 2 illustrates how the mean and standard deviation of the forecasts alone are not enough to represent the quality of predictions and, consequently, the predictability of a landslide. In fact the importance of a single forecast strongly depends on the time when it is made; for example, given the same set of forecasts ($t_{f,i}$), a higher P_i is obtained if the first predictions done are the farthest from T_f while the final ones tend to converge to it; in this way the prediction plot assumes an oscillatory shape (as for S and F forecasts in Figure 1). Conversely, if the same forecasts are made with a different order so that they get closer and closer to T_f as time passes by (that is $|t_{f,i} - T_f| < |t_{f,i-1} - T_f|$), then there is no $t_{f,i}$ prevailing on the others and it is not possible to define a more probable time of collapse (as for M forecasts in Figure 1). However the average and standard deviation of t_f are the same for both cases and this explains why these two statistics alone are not as informative as a prediction plot. From TABLE 2 it is also possible to assess which method gives the best results. The sum of the scores for S, F and M is 119, 115 and 63 respectively. Overall S and F perform similarly, but for a specific case study their effectiveness can be very different, therefore their result are independent and not redundant; there is no indisputable clue suggesting when F method is more performing than S and vice versa; nonetheless it appears that S is negatively influenced when the displacement curve is not regularly accelerating (Liberty Pit, Stromboli), whereas for F a few aligned points in the final tract in the inverse velocity plot are sufficient for predicting the failure; however F forecasts are more disturbed when displacement data are noisy, since they use their derivative (velocity) as input. Eventually M forecasts generally perform more poorly and rarely (i.e. artificial landslides B and C) surpass those obtained from S and F methods.

Interestingly, different displacement time series belonging to the same landslide can display different behaviours. This is a strong evidence that, even though the geological features do influence the predictability of a landslide, assuming that they keep the same for the whole landslide, other factors must determine the quality of the predictions. The last column of TABLE 2 shows for each time series what such factors could be, such as lithology (the asymptotic trends of the cases of Avran valley and Giau Pass can be explained as consequences of a lowly brittle material according to Petley's experiments; Petley, 2004), external forces (excavation in open pit mines, volcanic activity, rainfall), local effects (structural constraints, displacement measured relative to internal or lateral fractures not representing the general instability of the landslide), quality of data (length of the time series, frequency of the observations, level of noise, representativeness of the monitored point) etc.

All these case histories show that the main responsible for the predictability of a landslide, and secondary also for the presence or not of the "Saito effect", is connected to geology but not simply and directly. Instead both depend on the kinematics of the landslide, which in turn depends on the geological conditions. In the complex relation between geology and kinematics



the aforementioned factors may intervene and asymptotic trends in the inverse velocity plot have been encountered also for first failure ruptures (as found in some time series of Mount Beni landslide).

Finally, the prediction plots clearly show that, contrarily to what is generally believed (Rose and Hungr, 2007), the last forecasts are not necessarily the most accurate and that past ones (starting from the initiation of the tertiary creep) are essential to estimate the correct time of failure. In fact older forecasts can be more accurate and in any case furnish precious information about the general reliability of the final prediction, as explained above. The integration of more forecasting methods further raises reliability of the predictions, which is of great importance for early warning systems, in particular when evacuations are envisaged.

CONCLUSIONS

In conclusion, the results of the study are the following:

- Prediction plots are introduced as graphs showing the evolution of collapse forecasts with time. Such plots provide more information than simple average and standard deviation of the forecasts and improve the reliability of the final prediction.
- A predictability index (P_i) has been introduced as a scoring system based on the description of the prediction plot.
- The predictability of a landslide depends firstly on its kinematics and then on what determines it (geology, external forces, local effects etc.).
- Landslide collapses can be forecasted whether they are in highly or lowly brittle materials, in rock or in earth material, of different types, with different sliding surface geometries, volumes and triggers.
- Contrarily to what is generally assumed (Voight, 1988; Rose and Hungr, 2007), landslides can be forecasted also with external forces acting.
- The asymptotic behaviour of the inverse velocity curve does not imply that the landslide cannot be correctly forecasted, even though it can hinder the prediction.
- The asymptotic behaviour may be induced by external factors, lithology and local effects, rather than only by crack propagation. In fact asymptotic trends have been found in first time failures and in both brittle and lowly brittle materials. The crack propagation explanation is not neglected, but it may not represent the general rule.
- Most recent displacement monitoring data increase the confidence when estimating the time of failure but do not necessary provide more accurate predictions than the older ones (provided that they start from after the initiation of the tertiary creep).
- The developed approach integrates more forecasting methods to further improve the reliability of the prediction.

AUTHOR CONTRIBUTION

E. Intrieri developed the idea and performed the analyses. G. Gigli supervised and improved the manuscript.

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