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Evaluation of shallow landslide triggering scenarios through a physically-based approach: an example of application in the southern Messina area (north-eastern Sicily, Italy)

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Abstract

Rainfall-induced shallow landslides are a widespread phenomenon that frequently causes substantial damage to property, as well as numerous casualties. In recent years a wide range of physically-based models has been developed to analyze the triggering process of these events. Specifically, in this paper we propose an approach for the evaluation of different shallow landslide triggering scenarios by means of TRIGRS numerical model. For the calibration of the model, a back-analysis of the landslide event occurred in the study area (located SW of Messina, north-eastern Sicily, Italy) on 1 October 2009 was performed, by using different methods and techniques for the definition of the input parameters. After evaluating the reliability of the model through the comparison with the 2009 landslide inventory, different triggering scenarios were defined using rainfall values derived from the rainfall probability curves, reconstructed on the basis of daily and hourly historical rainfall data. The results emphasize how these phenomena are likely to occur in the area, given that even short-duration (3–6 h) rainfall events having a relatively low return period (e.g. 10 years) can trigger numerous slope failures. On the contrary, for the same rainfall amount, the daily simulations overestimate the instability conditions. The tendency of shallow landslides to trigger in this area agrees with the high number of landslide/flood events occurred in the past and summarized in this paper by means of archival researches. Considering the main features of the proposed approach, the authors suggest that this methodology could be applied to different areas, even for the development of landslide early warning systems.

1 Introduction

Landslides triggered by rainstorms occur in many part of the world and cause significant damage and loss to affected people, organizations and institutions as well as to the environment (Glade, 1997; Nadim et al., 2006; Petley, 2012). Within this category of natural disasters, shallow landslides (in particular debris-flows) pose a serious threat

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to life or property, in particular due to their high velocity, impact forces and long runout, combined with poor temporal predictability (Jacob and Hungr, 2005). These phenomena consist in sudden mass movements of a mixture of water and granular material that rapidly develop downslope eroding the soil cover and increasing their original volume (Iovine et al., 2003). Due to their high destructiveness, the study of these processes is an important research topic that can support decision makers in developing more detailed land-use maps and landslide hazard mitigation plans.

However, it is not simple to predict the probability of occurrence and magnitude of shallow landslides, considering the complexity of the phenomenon, mostly related to the variability of controlling factors (e.g. geology, topography, climate and hydraulic conditions, etc.). In this respect, a relation between triggering events (i.e. rainfall) and landslide occurrences is needed. To evaluate this cause–effect relationship, an approach widely used in the literature relies on the definition of empirical thresholds (Caine, 1980; Reichenbach et al., 1998; Wieczorek and Glade, 2005; Guzzetti et al., 2007). An empirical threshold defines the rainfall, soil moisture or hydrological conditions that, when reached or exceeded, are likely to trigger landslides (Reichenbach et al., 1998). Rainfall intensity-duration thresholds for the possible occurrence of landslides are defined through the statistical analysis of past rainfall events that have resulted in slope failures, and can be classified on the basis of the geographical extent for which they are determined (i.e. global, national, regional or local thresholds) and the type of rainfall information used to establish the threshold (Brunetti et al., 2010).

Nonetheless, the reliability of empirical thresholds is generally affected by the historical data quality and availability. In fact, adequate historical data on landslides and simultaneous rainfall are in most cases available only for a relatively short period, which may not be sufficiently significant from a statistical point of view. Furthermore, rainfall intensity and duration alone may not be able to capture most of the uncertainty related to landslide triggering (Peres and Cancelliere, 2014). Another drawback of the empirical rainfall thresholds is a general lack in spatial resolution. This aspect cannot be neglected if we consider that the terrain factors which control the onset of instability



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during a rainfall event can vary spatially to such an extent that, from a theoretical point of view, the rainfall threshold can be different for each landslide (Lo et al., 2012). Finally, further criticisms are based on the observation that it is not the amount of precipitation but the (largely unknown) amount of water that infiltrates and moves into the ground to cause failure (Guzzetti et al., 2008).

For this reason, in recent years different models have been developed to define physical (process-based) thresholds. Specifically, these models can determine the amount of precipitation needed to trigger slope failures, as well as the location and time of expected landslides, using spatially variable characteristics (e.g. slope gradient, soil depth and shear resistance) with a simplified dynamic hydrological model that predicts the pore pressure response to rainfall infiltration (Montgomery and Dietrich, 1994; Wilson and Wieczorek, 1995; Terlien, 1998; Frattini et al., 2009). These models, although they are challenging to apply over large areas where a detailed knowledge of input parameters is very difficult to acquire (Berti et al., 2012), are usually calibrated using rainfall events for which rainfall measurements and the location and time of slope failures are known.

In this paper we propose an approach based on TRIGRS (Baum et al., 2002), a physically-based model that predicts the timing and distribution of shallow, rainfall-induced landslides combining an infinite slope stability calculation with a transient, one-dimensional analytic solution for pore-pressure response to rainfall infiltration. This model has been used in order to define different shallow landslide triggering scenarios in the study area (located SW of Messina in north-eastern Sicily, Italy) by varying the rainfall input on the basis of the results deriving from the analysis of the historical rainfall data. Prior to this stage, the model has been thoroughly calibrated through the back-analysis of the disaster occurred in the same area on 1 October 2009. On that day, a heavy rainstorm triggered several hundreds of shallow landslides, causing 37 fatalities and severe damage to buildings and infrastructure. Given the nature of the event, it can be considered particularly representative of the studied phenomenon and, thus, suitable for testing the reliability of the physically-based model.



The paper is organized as follows: after a brief description of the study area and the 1 October 2009 event (Sect. 2), a summary of the landslide/flood events occurred in the past is reported (Sect. 3). Then, the methods used for the analysis of the historical rainfall data and the parameterization of TRIGRS model are outlined (Sect. 4). Afterwards, the results of the back-analysis of 2009 event and the evaluation of possible future triggering scenarios are provided (Sect. 5) and discussed, along with the main features of the proposed approach (Sect. 6). Finally, in Sect. 7 the main conclusions are summarized.

2 General features of the study area and the 1 October 2009 event

The study area (Fig. 1) is located south of Messina (north-eastern Sicily, Italy), at the NE termination of the Peloritani Mountain Belt, that represents the southern border of the Calabrian–Peloritani arc. This chain is composed by different metamorphic units (Kabilo–Calabride Complex) of pre-alpine age and later involved in Hercynian and Alpine orogenic processes, and tectonically overlapping the sedimentary Maghrebide units (Lentini et al., 2000). Since the Late Miocene, the opening of the Tyrrhenian Basin led to the formation of an extensional fault system that involved and re-oriented some of the former structures. These faults, generally oriented NE–SW, have influenced the development of this region during the Pleistocene–Holocene (Antonioli et al., 2003; Di Stefano et al., 2012), resulting in a landscape characterised by steep slopes eroded by torrent-like straight watercourses, with alluvial conoids and debris-flow fans along the valleys. A thin (0.5–2 m) layer of colluvial deposits or coarse-grained regolith overlies the majority of the slopes, where small outcrops of marine terraces, documenting the different uplift-rates during the Late Pleistocene, can be found. Three orders of terraces can be distinguished at approximately 185, 135 and 95 m a.s.l. (Catalano and De Guidi, 2003), whereas there is no evidence of fluvial terraces. Catchments generally have small dimensions and markedly elongated shapes, with short time of concentration and direct discharge into the sea. In particular the study area, which has an exten-

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to declare the state of natural disaster for the villages of southern Messina area. However, even though the largest number of events recorded during the last twenty years also depends on the increasing number of sources of information, it is important to emphasize that, in the same period, the landslide risk exposure of the area has increased, substantially due to the enlargement of the urban area as a consequence of poor land-use planning (Del Ventisette et al., 2012). Finally, recent studies (Bonaccorso et al., 2005; Arnone et al., 2013) indicate an increasing trend for extreme, short-duration rainfall events over the last few decades in Sicily, especially in coastal areas.

4 Methodology

4.1 Analysis of historical rainfall data

In order to depict different shallow landslide triggering scenarios in the study area, firstly it is necessary to evaluate the recurrence of specific rainfall events, which can be used as input for the physically-based model. Therefore, a statistical analysis of historical rainfall data has been performed. The hydrological-statistical model is based on the analysis of the maximum values assumed by the chosen hydrological variable (i.e. cumulative rainfall at different time intervals). Once the significant hydrological variables are identified, the recurrence of the rainfall event can be expressed in terms of return period. In this study the probability model relied on the Generalized Extreme Value (GEV) distribution introduced by Jenkinson (1955). This distribution is a generalized version of the more known Gumbel distribution, which is largely used in the study of extreme events. The variables of “rainfall cumulated” (PC_n) i.e., in 1, 2, 5, 10, 30, 60, 90, 120 and 180 days are computed from daily rainfall data by means of the expression:

$$PC_{n,j} = \sum_{i=j-n+1}^j P_i \quad \text{with } n = 1, 2, 5, 10, 30, 60, 90, 120, 180 \quad (1)$$

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where j is the progressive number of days that form the analyzed time interval and P_i is the rainfall value recorded the i th day. The maximum values of each variable are extracted, year by year, from the datasets so generated and the parameters of the GEV function are determined from the above values, by applying the Probability Weighted Moments (PWM) method introduced by Greenwood et al. (1979) and subsequently modified by Hosking et al. (1985). Finally, the inversion of the probability function yields the values of cumulated rainfall x for each of the variables (1, 2, 5, 10...180 days) and for different return periods. Then, these values are interpolated with a view to build the rainfall probability curves.

To yield reliable results, this type of analysis requires sufficiently long and continuous time series of rainfall data (at least 20 years of recorded data according to Houghton et al., 2001 and Serrano, 2010). For this reason, use was made of daily rainfall data from Santo Stefano di Briga and Messina Istituto Geofisico rainfall stations, that are operational since 1925 and 1952, respectively. However, if we consider that the extreme rainfall events which periodically affect the study area are usually of short duration, as in the case of the 1 October 2009 event, it would be extremely interesting to analyze the historical data of maximum hourly rainfall intensity. Unfortunately, these data are not available for the above mentioned stations. For this reason, use was made of hourly rainfall data (i.e. cumulated in 1, 3, 6, 12 and 24 h) from Alì Terme station, that is located approximately 4 km SE of Fiumedinisi station (see Fig. 8b); thus, this station has been considered sufficiently close to be used to assess the recurrence of the rainfall events recorded in Fiumedinisi station.

4.2 Theoretical basis of TRIGRS model

TRIGRS (Transient Rainfall Infiltration and Grid-based Slope Stability model) is a Fortran program designed for modelling the timing and distribution of shallow, rainfall-induced landslides. It combines a transient, one-dimensional analytic solution for pore-pressure response to rainfall infiltration with an infinite slope stability calculation. In the original version (Baum et al., 2002), the infiltration model was based on Iverson's (2000)

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linearized solution of Richards' equation, with implementation of complex storm histories, an impermeable basal boundary at finite depth and a simple runoff routing scheme (Savage et al., 2003; Salciarini et al., 2006). Introducing a time-varying rainfall input on the ground surface $I_Z(t)$, the pressure head response $\Psi(Z, t)$ can be computed using the following input parameters (variable from cell to cell throughout the model): slope, soil layer depth d_{lb} , depth of the initial steady-state water table d_{wt} , long term (steady-state) surface flux I_{ZLT} and saturated hydraulic conductivity K_s . However, this solution is appropriate for initial conditions where the hillslope is tension-saturated (Fig. 5a). In the second version (Baum et al., 2008), TRIGRS model was expanded to address infiltration into a partially unsaturated surface layer above the water table by using an analytical solution of the Richards' equation for vertical infiltration (Fig. 5b). TRIGRS uses four hydrodynamic parameters (θ_s , θ_r , α_G and K_s) to linearize the Richards' equation through the unsaturated zone, according to Gardner (1958) hydraulic model. If the amount of infiltrating water reaching the water table exceeds the maximum amount that can be drained by gravity, TRIGRS simulates the water-table rise comparing the exceeding water quantity to the available pore space directly above the water table or capillary fringe and then, for each time step, applies the water weight at the initial top of the saturated zone to compute the new pressure head (Baum et al., 2010). For the calculation of the Safety Factor in the unsaturated configuration, the pressure head is multiplied by the effective stress parameter:

$$\chi = (\theta - \theta_r) / (\theta_s - \theta_r) \quad (2)$$

as suggested by Vanapalli and Fredlund (2000). This approximation has application to a generalized effective stress law and represents a simplified form of the suction-stress characteristic curve (Lu and Godt, 2008; Lu et al., 2010).

4.3 Parameterization of the numerical model

In order to define the input parameters of TRIGRS model, use was made of different methods and techniques. To estimate the spatial variation of soil thickness, the model

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proposed by Saulnier et al. (1997), which correlates soil depth to the local slope angle, has been applied to the study area (Fig. 6a). The maximum and minimum values of slope and soil thickness, which were measured within the source areas of the shallow landslides triggered during the 1 October 2009 event, have been used as constraints of the model. These values, equal to 58–17° and 1.5–0.5 m respectively, can be considered reliable since the 2009 landslides mostly involved the entire soil profile. Although this model relies heavily on geomorphological simplifications, it is frequently used to estimate a spatially distributed soil depth field in basin scale modelling (e.g. Salciarini et al., 2006).

To reproduce the spatial rainfall distribution of the 1 October 2009 rainstorm, the conditional merging technique (Ehret, 2002; Pegram, 2003) has been chosen as interpolating method. In this approach, the information from the satellite radar is used to condition the spatial rainfall field obtained by the interpolation of rain gauge measurements. Although there are numerous deterministic methods for estimating spatial rainfall distribution (e.g. Thiessen polygon, Inverse Distance Weighted, polynomial interpolation, etc.), geostatistical methods are commonly preferred because they allow not only to account for spatial correlation between neighboring observations to estimate values at ungauged locations, but also to include more densely sampled secondary attributes (i.e. weather radar data) with sparsely sampled measurements of the primary attribute (i.e. rainfall) to improve rainfall estimation (Mair and Fares, 2011). In particular, meteorological satellite radars give a large-scale vision of precipitation fields compared to scattered point estimates from rainfall gauges. In this study, use was made of the precipitation rate maps deriving from the processing of EUMETSAT (European Organisation for the Exploitation of Meteorological Satellites) satellite data. These maps, that were made available by the National Center of Aeronautical Meteorology and Climatology (CNMCA) of the Italian Air Force, are generated from blending of PMW (passive microwave) measurements and IR (Infrared) brightness temperatures, coupled with the NEFODINA (DYNAMIC NEFOanalysis) software, that allows the automatic detection and classification of convective cloud systems reducing the un-

derestimation of precipitation (Mugnai et al., 2013). Ten-minute rainfall records of six stations (Antillo, Colle San Rizzo, Fiumedinisi, Ganzirri, Messina Istituto Geofisico and Santo Stefano di Briga) have been used as input data, after conveniently converting them into fifteen-minute data for the comparison with the corresponding radar rainfall maps. Thus, sequential rainfall maps (Fig. 6b) have been obtained referred to the time period between 13:00 and 21:00 UTC.

The hydraulic properties of the colluvial deposit, the steady-state water-table depth and the initial soil moisture conditions have been estimated using HYDRUS 1-D model (Šimůnek et al., 1998), a USDA (United States Department of Agriculture) Salinity Laboratory software which can simulate the water flow into unsaturated porous media resulting from a rainfall event. The software describes infiltration in vadose zone using a modified version of Richards' equation. In this paper, numerical simulations have been performed for the period 1–30 September 2009 in order to quantify the effect of the 1 month antecedent rainfall on soil moisture conditions. As hydraulic model, van Genuchten–Mualem model (van Genuchten, 1980) was chosen to simulate the water flow, whereas the hydrodynamic parameters θ_s , θ_r , α_G and K_s are predicted from soil grain size distribution using the ROSETTA Lite module (Schaap et al., 2001). This module uses a database of measured water retention and other properties for a wide variety of media. For a given grain size distribution and other soil properties the model estimates a retention curve (i.e. the relationship between soil water suction Ψ and the amount of water remaining in the soil θ) with good statistical comparability to known retention curves of other media with similar physical properties (Nimmo, 2005). Daily rainfall data have been used as input for the model, whereas evapotranspiration is accounted for by inserting into the Hargreaves equation (Jensen et al., 1997) the maximum and minimum temperature values recorded during the investigated period. As lower boundary, a zero-flux condition was assumed due to the presence of an impermeable bedrock below the soil cover. A 80 cm soil profile inclined of 38° (i.e. the average soil thickness and slope observed within the landslide source areas) was cho-

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sen as the most representative geometric configuration of the slope prior to 1 October event.

Laboratory tests have been performed to measure physical and mechanical properties of the colluvial deposit (Table 1). The grain size distribution analysis shows a soil composed mainly of gravel (45.4%) and sand (38.1%) with minor components of silt and clay (12 and 4.5% respectively). With regard to the mechanical parameters, drained triaxial tests have been conducted on three large reconstituted specimens ($H = 200$ mm, $D = 100$ mm). To reconstitute each specimen, the soil was compacted inside a mould in different layers of decreasing depth, in order to account for under-compaction. The tested material was sieved leaving the maximum grain size of 10 mm and imposing 35% of porosity (i.e. the average porosity obtained from different soil samples) and 8% of initial water content. The latter value can be considered representative of the investigated soil on the basis of the results of HYDRUS 1-D model (see Sect. 5.2). For the same material other authors (Aronica et al., 2012; Peres and Cancelliere, 2014; Penna et al., 2014) reported values which vary between 30 and 40° for the friction angle and between 0 and 5 kN m⁻² for the cohesion; thus the resulting internal friction angle (i.e. 36.3°), obtained by assuming a null cohesion, substantially agrees with these values. However, it is important to stress that this difference can depend on both the natural spatial variability of soil shear strength parameters and the type of deposit, characterized by an extremely variable texture resulting from erosion and weathering areas.

Finally; as regards the 1 October 2009 landslide inventory, it is important to note that for each of more than 700 mapped landslides, identified through analysis of high resolution aerial orthophotos integrated by field surveys in the days after the event, the landslide deposit has been distinguished from the source area. Therefore, in order to achieve more accurate assessment, only the latter data has been used for the comparison with the numerical simulations.

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5 Results

5.1 Rainfall probability curves and return period of the 1 October 2009 event

Figure 7a–c shows the graphic comparison between cumulative frequency (symbols) and GEV probability function (continuous line), obtained by using the daily rainfall records from Santo Stefano di Briga and Messina Istituto Geofisico stations. As it can be observed, the good fitting between data and probability function confirms the reliability of the applied method. With regard to the probability curves (Fig. 7b–d), the comparison reveals that the highest rainfall values are attributed to the Santo Stefano di Briga curves for the same return period. This finding emphasizes that, in the past, this station (the most representative of the sector most severely hit by the 2009 event) has recorded more intense and severe rainfall events than the other one. On the basis of the same curves, the return periods of the rainfall accumulated up to 1 October 2009 have been estimated (Table 2). An estimation has been made also for the rainfall accumulated up to 30 September (i.e. the day prior to the event), but the obtained values infer that the rainfall amount, at both stations, is far from exceptional (estimated return periods of 1 year); thus, rainfall prior to the event practically lies within the standard range, in contrast with rainfall accumulated up to 1 October. In this case, while rainfall recorded at Messina Istituto Geofisico continues to be unexceptional (estimated return period of 4–5 years), rainfall accumulated in a single day (1 October) at Santo Stefano di Briga has a return period of 47 years. This means that the event under review was not only strongly localized in space, but also particularly severe in that specific sector. This finding is also substantiated by what has been previously pointed out, i.e. the highest return periods have been obtained for the station with the highest rainfall probability curves.

With regard to the analysis of the historical data of maximum hourly rainfall intensity from Ali Terme station, the results shows that the fit between cumulative frequency and probability (Fig. 8a) is not as good as in the preceding analyses. However, it is worth stressing that data about intense precipitation are generally scantier than daily

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ones and that the resulting statistical analyses are usually less reliable. The resulting rainfall probability curves (Fig. 8b) define a return period of 78 years for the 1 h rainfall recorded on 1 October 2009 in Fiumedinisi station, a value greater than that estimated for the 1 day rainfall recorded in Santo Stefano di Briga station (47 years). Therefore, the 1 h rainfall event can be classified as an extreme event. Nevertheless, it is particularly interesting to analyze the sub-event of maximum duration equal to three hours, a time after which major damage was observed in the area. In this case the estimated return period is equal to 26 years (Table 3): this value infers that, even if the return period of the 1 h rainstorm allows to assert that it was an extreme event, the 3 h sub-event is characterized by a return period much lower, which suggests its classification as not severe. However, it is worth noting that the probabilistic analysis is affected by several uncertainties, related to the type of probabilistic model and the definition of the parameters of the model itself. Generally speaking, the uncertainty tends to increase with decreasing the sample size (i.e. the number of measurement years) and increasing the considered return period.

5.2 Back-analysis of the 1 October 2009 event

As previously mentioned, before applying the TRIGRS model to back-analyze the 1 October 2009 event, it was necessary to evaluate the soil moisture conditions prior to the event through HYDRUS 1-D model. According to the simulation results, the absence of a steady-state water table within the soil cover can be assumed, whereas in Fig. 9a the resulting volumetric water content trend with depth at four different times (1, 24, 25 and 30 September) is reported. The initial soil moisture ($\theta = 0.049$) is assumed near to the residual water content value considering the hot, dry conditions during the preceding summer months. The effect of the September rainfall (Fig. 9b) results in a progressive increase in soil water content, that is equal to 0.202 on 25 September (the day after the second rainfall event, see Table 4). It is worth noting that the water content values are averaged approximately for the first 30 (24 September) and 50 (25 September) cm of soil, considering the evident non-homogeneous due to the advance of the wetting

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process. On the other hand, the water content trend is much more homogeneous in the first 70 cm of soil on 30 September, resulting in an average value of 0.145, which corresponds to a gravimetric water content (w) of approximately 8% and a degree of saturation (S_r) equal to 41.5% (on the basis of the physical properties reported in Table 1).

Once the initial soil moisture conditions are estimated, all the input parameters required by TRIGRS can be defined (Table 5). As digital elevation model, use was made of a detailed (2×2 m) pre-event DEM, resampled at the 4×4 m resolution substantially due to limitations on computing time. Soil thickness (H) and rainfall intensity (I_Z) vary from cell to cell on the basis of maps obtained through the methods described in Sect. 4.3. According to the available data an average friction angle and cohesion of 35° and 2.5 kN m^{-2} have been used whereas γ_n , which represents the depth-averaged soil unit weight, is equal to 18.8 kN m^{-3} given the porosity (35%), the degree of saturation (41.5%) and the unit weight of soil solids (26.73 kN m^{-3}), θ_s (saturated water content), θ_r (residual water content) and K_s (saturated hydraulic conductivity) are directly predicted using HYDRUS-1D model. Given the absence of an initial water table, its depth (d_{wt}) so corresponds to the bedrock–soil interface. To evaluate α_G parameter, that is typical of Gardner hydraulic model, use was made of the conversion formula introduced by Ghezzehei et al. (2007) which defines a correspondence between Gardner and van Genuchten–Mualem models through the capillary length approach (Warrick, 1995). On the basis of the results of the same simulations the I_{ZLT} parameter, that represents the long-term background rainfall rate, was assumed equal to the cumulative actual surface flux value ($5.3 \times 10^{-8} \text{ ms}^{-1}$). Finally, the saturated hydraulic diffusivity (D_0) has been calculated according to:

$$D_0 = \frac{(K_s H)}{S_y} \quad (3)$$

where K_s is the saturated hydraulic conductivity, H the average soil thickness (80 cm) and S_y the specific yield (Grelle et al., 2014). If we consider that the investigated soil

can be classified as loamy sand, the specific yield has been assumed equal to 0.26, on the basis of typical values given by Johnson (1967) (also reported in Loheide II et al., 2005) for each soil textural class.

With regard to the comparison between the numerical simulations and the landslide inventory map, Table 6 reports, as well as the number and relative percentage (P_U) of predicted unstable pixels (i.e. $FS \leq 1$), the percentage of correctly predicted landslide (P_L) and stable (P_S) pixels between 14:00 and 21:00 UTC, whereas Fig. 10 shows the temporal evolution of slope instability at the catchment scale. As can be noted, only 11.7 % of pixels are indicated as “unstable” at 14:00 UTC. After a slowly increase in the following three hours, the instability rapidly rose between 17:00 and 18:00 UTC (P_U : +25.1 %) in correspondence of a rainfall peak, and the critical stage continued until 21:00 UTC, given that P_U passed from 47.7 to 100 % in just 3 h. This temporal evolution of the phenomenon substantially agrees with both the witnesses and amateur videos, although during the real event no particular increase of slope instability has been recorded after 20:00 UTC. To fully evaluate the accuracy of the model, a ROC (Receiver Operating Characteristics) curve analysis has been performed by comparing the final safety factor map (21:00 UTC) with the landslide inventory. The ROC curve measures the goodness of the model prediction plotting, for different threshold values, the True Positive rate, i.e. the proportion of correctly predicted positive values (“landslide presence”) and the False Positive rate, i.e. the proportion of negative values (“landslide absence”) erroneously reported as positive. The Area Under Curve (AUC), which varies from 0.5 (diagonal line) to 1, quantifies the predictive capability of the model.

According to the results, the FS map correctly classifies 47.0 % of source areas (True Positive) and 80.4 % of stable areas (True Negative) with $FS = 1$, whereas the AUC is equal to 0.740 (Fig. 11a). Although the model predicts only about the half of real landslide areas, it is worth noting that the main ones, i.e. those occurred on the slopes above Giampileri village, are correctly identified by the model, whereas an

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overestimation of slope instability characterizes in particular the NW sector of the study area (Fig. 11b).

5.3 Evaluation of different triggering scenarios

Once the physically-based model was calibrated through the back-analysis of the 1 October 2009 event, different TRIGRS simulations have been performed by varying the rainfall input, on the basis of the daily rainfall probability curves obtained for the rain gauge closest to the study area, i.e. Santo Stefano di Briga monitoring station. With regard to the other input parameters of the model, those used for the analysis of the 1 October event have been kept. Table 7 shows the number of unstable pixels depending on rainfall event. The four rainfall values used in the analyses correspond to four different return periods (RP): 2, 4, 10 and 20 years. In this way, it is possible to evaluate the effect of more frequent rainfall events with respect to the 2009 one, whose RP has been estimated equal to 47 years. Given the short duration of the 1 October event, only 1 day rainfall events have been simulated. According to the results, a significant level of instability may be also caused by events much less severe than the 1 October one. For instance, an event with RP = 10 years (such as the 25 October 2007 one, see Sect. 3) would cause approximately 62.7% of the slope instability computed with the 1 October rainfall amount. Nevertheless, in the case of the 2009 event, if we compare the simulation results obtained by using the 1 day rainfall value with those gained with the 15 min rainfall maps, it can be noted how the first one identifies a much higher number of unstable pixels (203 778 rather than 95 748), overestimating the instability phenomena. For this reason, hourly simulations have also been performed on the basis of the rainfall probability curves obtained for Alì Terme station (Table 8). In this case, the comparison shows that the 6 h rainfall simulation depicts a triggering scenario substantially similar to the one simulated with the 15 min rainfall maps. In fact, 76 936 pixels are predicted as “unstable” by the model, corresponding to 80.4% of the pixels identified in the 15 min rainfall simulation. With regard to the other results, it can be noted the importance, in terms of produced instability, of the 1 h rainfall amount even for

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infiltration rate is very low, often insufficient for trigger numerous shallow landslides. Therefore, landslide phenomena similar to the 1 October one are related to extreme but sufficiently long rainfall events. Significantly, it results a clear increase in slope instability in the 3 h and 6 h simulations, even with rainfall values characterized by relatively low return periods (e.g. 10 years). These results emphasize the severity of recurring rainfall events in the study area and explain the high number of landslide/flood events occurred in the past (see Sect. 3). In fact, considering the high values which characterize the rainfall probability curves of Santo Stefano di Briga and Ali Terme rain gauge stations (see Sect. 5.1), we can assert that short duration rainfall events frequently have a high intensity in this specific area. Therefore the combination of recurring and heavy rainfall events, probably due to specific geomorphological and climatic features that influence the development of localized severe storms, justifies the approximately 40 landslide/flood events that would have occurred since the last century in this area.

From a methodological point of view, it is worth noting how the approach proposed in this paper (Fig. 12) combines various techniques and methods optimizing different types of data, depending on their availability. For instance, the parameterization of the physically-based model can be performed both in the absence and presence of preceding reference events. In the former case, only the geotechnical parameters and the soil thickness are needed, while in the latter the process used for the back-analysis of the 1 October 2009 event can be applied to any other event. Here, in particular, it is important to stress that the comparison with a preceding landslide event allows to increase the reliability of the model, as long as a comprehensive and detailed event-based landslide inventory exists. With regard to the evaluation of the initial soil conditions, in this study the HYDRUS 1-D model has been used considering the 1 month antecedent rainfall. However, some recent studies have investigated the linkage between soil moisture and landslide occurrence by using soil moisture data derived by in situ (Baum and Godt, 2009; Hawke and McConchie, 2011) and satellite sensors (Ray and Jacobs, 2007; Ray et al., 2010), and this type of measurements, if available, can be used to define the input parameters of the model. The last step of the approach concerns the definition

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of the rainfall input to be used for the evaluation of a specific triggering scenario. By means of a statistical analysis of hourly rainfall data, different rainfall values having different return periods may be used to depict different scenarios, to changing the initial soil conditions and the duration of the rainfall input. However, as emphasized in the discussion of the results, establishing which is the critical rainfall duration that triggers the shallow landslides cannot be straightforward, because completely different slope stability conditions can be obtained with different combinations of single rainfall inputs within the same rainfall event, even with the same initial soil conditions. For this reason, and considering the chance to use also soil moisture data, a possible application of the here-proposed approach could be to develop an early warning system based on rainfall thresholds, identified by the physically-based model calibrated according to the above-described process. In this way, a model in which the initial conditions are constantly updated, could depict more consistent and reliable triggering scenarios, by using any rainfall event forecasted for the next hours.

7 Conclusions

In this study, we introduce an approach for the analysis of shallow landslide triggering scenarios that uses the TRIGRS code, a physically-based model which describes the stability conditions of natural slopes in response to specific rainfall events. As a first step, the model has been calibrated through the back-analysis of a reference landslide event, i.e. the disaster occurred in the southern Messina area on 1 October 2009. Comparing the results of the numerical simulation with the 2009 landslide inventory, it turns out the model is able to reproduce quite well the reference event, both in terms of temporal evolution and spatial distribution of slope instability, identifying the areas mostly affected by shallow landslides. It is worth stressing that the model has been accurately calibrated through different methods and techniques, with specific focus on the evaluation of the spatial pattern of the triggering storm and initial soil moisture conditions.

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Table 1. Physical and mechanical properties of the colluvial deposit.

Physical properties	
Unit weight of soil solids (kN m^{-3})	26.73
Porosity (%)	35
Granulometric characteristics	
Gravel (%)	45.39
Sand (%)	38.12
Silt (%)	11.99
Clay (%)	4.5
Mechanical properties	
Friction angle ($^{\circ}$)	30–40
Cohesion (kN m^{-2})	0–5

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Table 2. Estimated return periods (in years) for rainfall accumulated in the 1, 2, 5, 10, 20, 30, 60, 90, 120 and 180 days prior to 1 October 2009 for Santo Stefano di Briga (SSB) and Messina Istituto Geofisico (MIG) rainfall stations.

Cumulated days	Return period SSB	Return period MIG
1	47	4
2	22	2
5	8	1
10	12	5
20	7	4
30	4	2
60	1	1
90	1	1
120	1	1
180	1	1

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Table 3. Estimated return periods (in years) for rainfall accumulated in 1, 3, 6, 12 and 24 h during the 1 October 2009 event.

Cumulated hours	Return period
1	78
3	26
6	40
12	39
24	19

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Table 4. Resulting average volumetric-gravimetric water content (θ and w , respectively) and degree of saturation (S_r) at four different simulation times (1, 24, 25 and 30 September).

Date (day-month)	θ (–)	w (%)	S_r (%)
1 September	0.049	2.8	14.0
24 September	0.260	14.7	74.3
25 September	0.202	11.4	57.8
30 September	0.145	8.2	41.5

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Table 5. Input parameters for TRIGRS model.

Parameter	Attributed value
H (m)	Spatial map
I_z (m s^{-1})	Spatial-temporal maps
φ' ($^\circ$)	35
c' (kNm^{-2})	2.5
γ_n (kNm^{-3})	18.8
θ_s (-)	0.3904
θ_r (-)	0.0485
K_s (ms^{-1})	1.22×10^{-5}
d_{wt} (m)	H
α_G (m^{-1})	9.09
I_{ZLT} (ms^{-1})	5.3×10^{-8}
D_0 ($\text{m}^2 \text{s}^{-1}$)	3.75×10^{-5}

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Table 6. Results of TRIGRS simulation at different times: number and relative percentage of pixels predicted as “unstable” (P_U); percentage of correctly predicted landslide (P_L) and stable (P_S) pixels.

Time	n. “unstable” pixels	P_U (%)	P_L (%)	P_S (%)
14:00 UTC	11 163	11.7	5.0	97.7
15:00 UTC	13 357	13.9	6.0	97.2
16:00 UTC	17 897	18.7	8.5	96.3
17:00 UTC	21 668	22.6	10.4	95.5
18:00 UTC	45 719	47.7	22.1	90.6
19:00 UTC	61 736	64.5	30.6	87.4
20:00 UTC	77 939	81.4	38.5	84.0
21:00 UTC	95 748	100	47.0	80.4

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Table 7. Results of TRIGRS simulations for different daily rainfall scenarios. P_U represents the relative percentage of pixels predicted as “unstable” by the model.

Rainfall duration	Return period (year)	Rainfall amount (mm)	n. “unstable” pixels	P_U (%)
1 day	2	70	61 771	30.3
	4	95	93 168	45.7
	10 (2007 event)	130	127 733	62.7
	20	165	160 063	78.5
	47 (2009 event)	225	203 778	100

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Table 8. Results of TRIGRS simulations for different hourly rainfall scenarios. P_{2009} represents the percentage of unstable pixels compared to those predicted in the back-analysis of the 1 October 2009 event.

Rainfall duration	Return period (year)	Rainfall amount (mm)	n. “unstable” pixels	P_{2009} (%)
1 h	2	30	35 447	37.0
	4	45	38 011	39.7
	10	55	38 011	39.7
	20	65	38 011	39.7
	78 (2009 event)	86	38 011	39.7
3 h	2	45	37 416	39.1
	4	60	39 311	41.1
	10	85	44 129	46.1
	20	105	47 325	49.4
	26 (2009 event)	115	48 574	50.7
6 h	2	55	44 749	46.7
	4	70	50 792	53.0
	10	95	61 068	63.8
	20	120	68 515	71.6
	40 (2009 event)	142	76 936	80.4
12 h	2	60	62 075	64.8
	4	85	81 576	85.2
	10	110	98 328	102.7
	20	130	110 896	115.8
	39 (2009 event)	153	125 259	130.8

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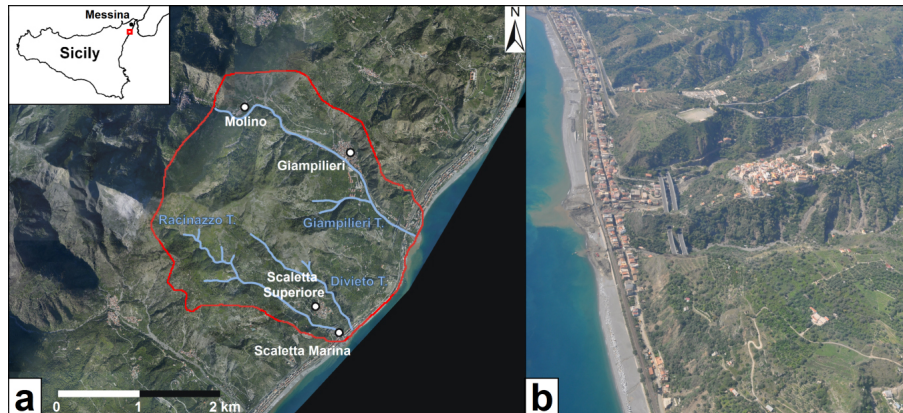


Figure 1. (a) The study area; (b) aerial view of Giampileri area a few days after the 1 October 2009 event.

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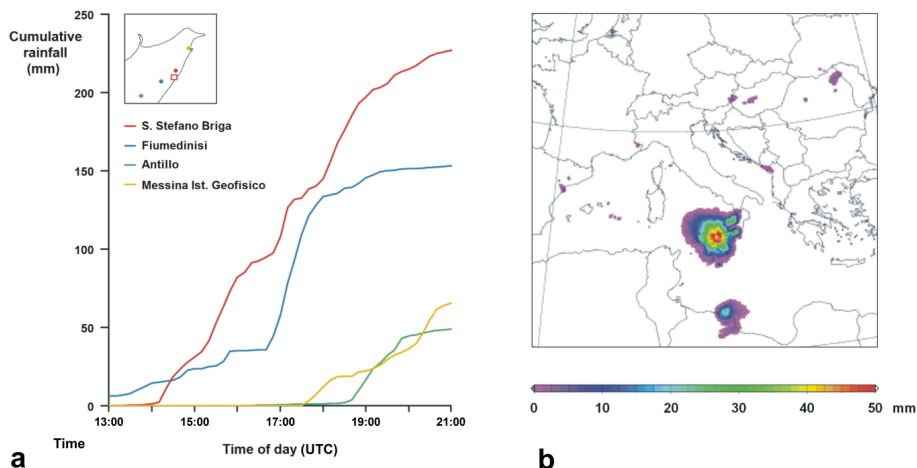


Figure 2. (a) Cumulative hyetographs recorded at the 4 rain gauge stations (Santo Stefano di Briga, Fiumedinisi, Antillo and Messina Istituto Geofisico), whose location is shown in the upper left sketch (the red square represents the study area); (b) accumulated rainfall between 18:00 and 21:00 UTC, based on radar (satellite) data (from Melfi et al., 2012).

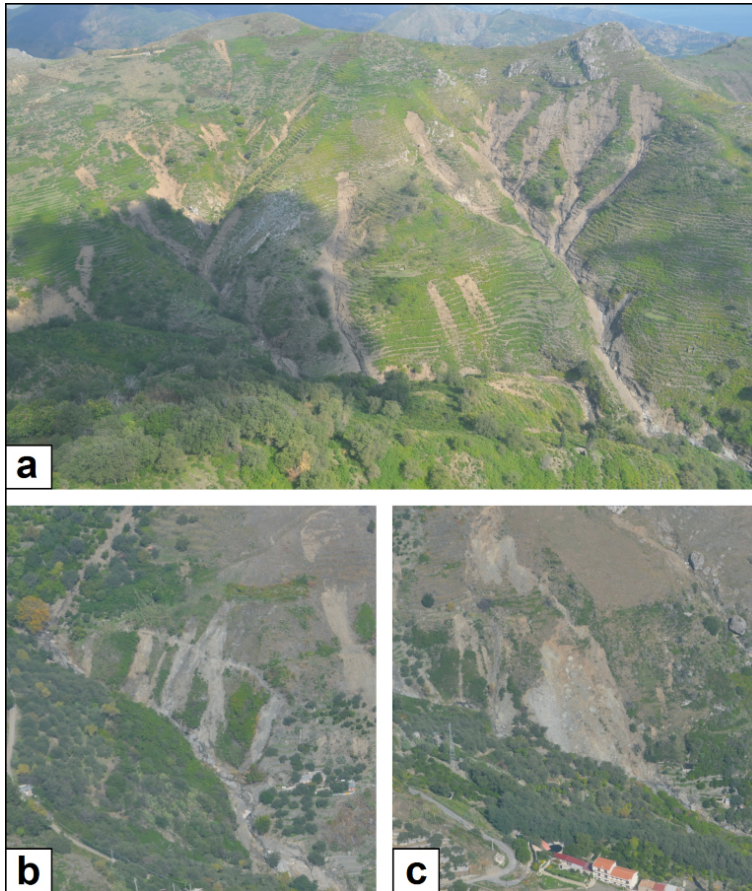


Figure 3. Different types of shallow landslides occurred on 1 October 2009 **(a)** debris-flows; **(b)** debris-slides; **(c)** debris-avalanches.

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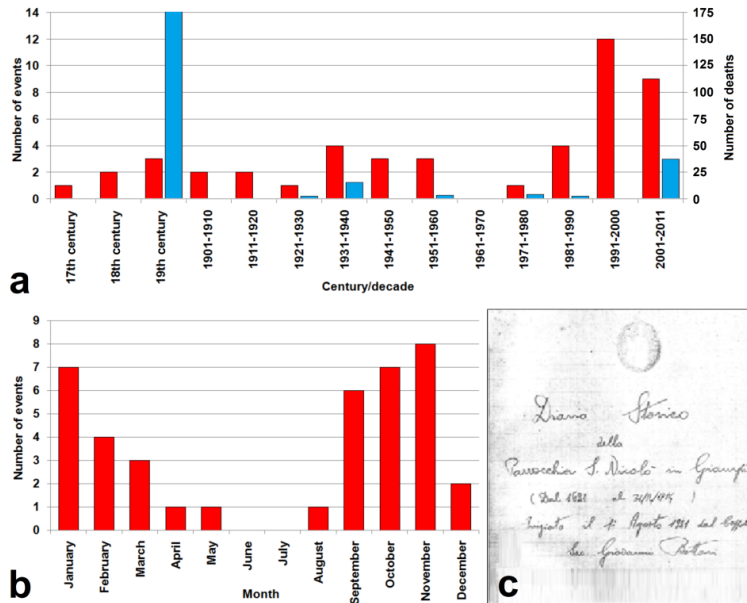


Figure 4. (a) Number of landslide/flood events (red bars) and related fatalities (blue bars) recorded in the southern Messina area per century/decade; (b) number of recorded landslide/flood events per month of occurrence; (c) frontispiece of the historical diary of S. Nicolò di Giampileri church.

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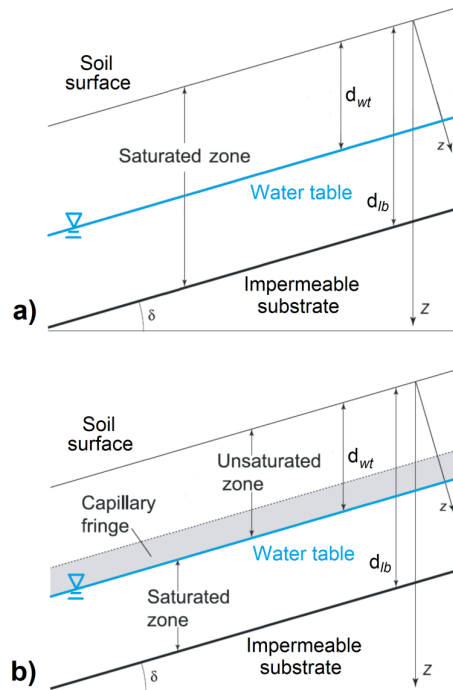


Figure 5. Conceptual sketch of the hydrological model in TRIGRS simulating tension-saturated **(a)** and unsaturated **(b)** soil conditions (from Baum et al., 2010 mod.)

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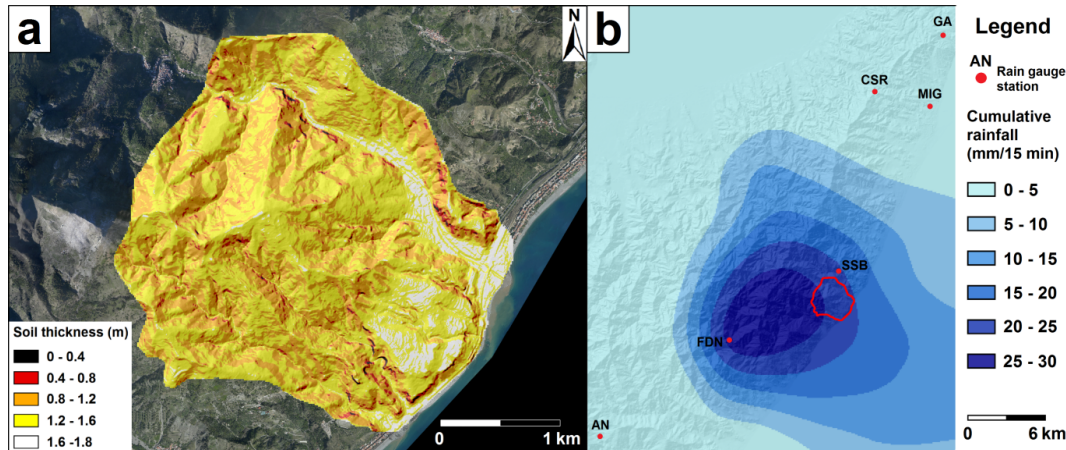


Figure 6. (a) Soil thickness map for the study area; (b) an example of rainfall map resulting from the application of conditional merging technique: in this case, the cumulative rainfall between 17:00 and 17:15 UTC is reported. Points indicate the location of the six rain gauge stations used in the method (AN: Antillo, CSR: Colle San Rizzo, FDN: Fiumedinisi, GA: Ganzirri, MIG: Messina Istituto Geofisico, SSB: Santo Stefano di Briga).

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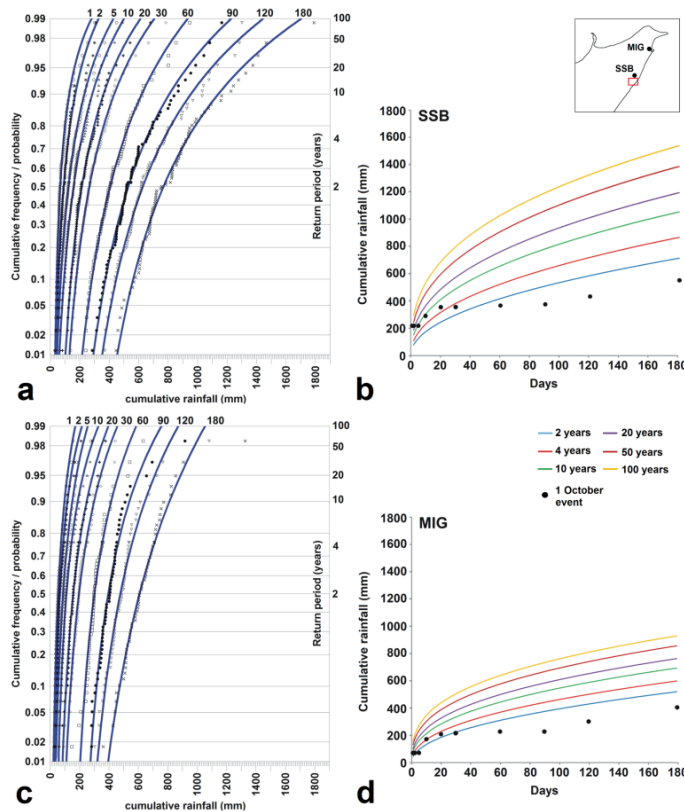


Figure 7. Cumulative frequency and probability according to GEV distribution for 1, 2, 5, 10, 20, 30, 60, 90, 120 and 180 days cumulative rainfall for **(a)** Santo Stefano di Briga (SSB) and **(c)** Messina Istituto Geofisico (MIG) station; **(b–d)** rainfall probability curves for return periods of 2, 4, 10, 20, 50 and 100 years for the same stations, whose location is shown in the upper right sketch (the red square represents the study area).

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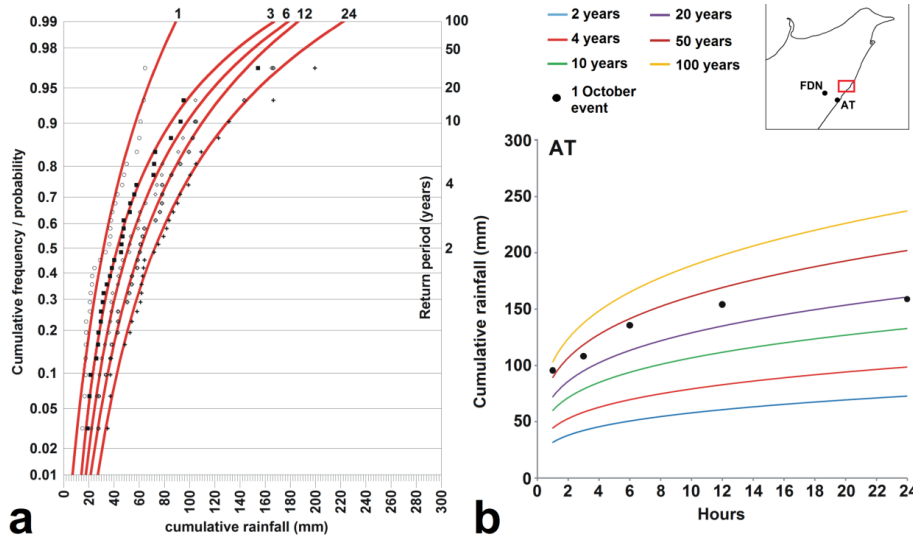


Figure 8. (a) Cumulative frequency and probability according to GEV distribution for 1, 3, 6, 12 and 24 h cumulative rainfall for Ali Terme (AT) station; (b) rainfall probability curves for return periods of 2, 4, 10, 20, 50 and 100 years for the same station. The location of this station and Fiumedinisi (FDN) one is shown in the upper right sketch (the red square represents the study area).

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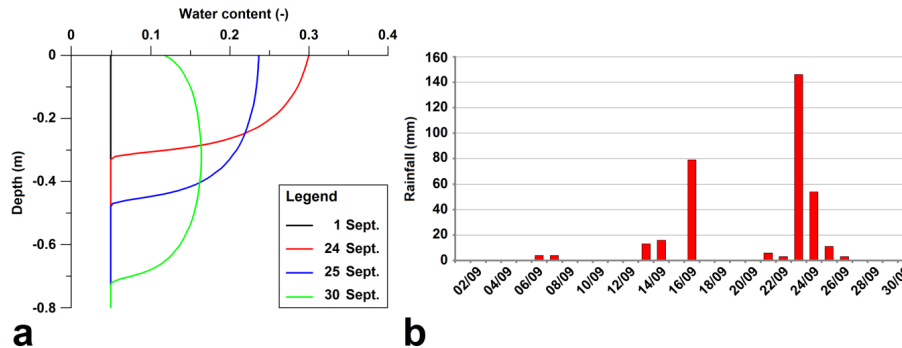


Figure 9. (a) Resulting water content trend vs. depth at four different simulation times (1, 24, 25 and 30 September); (b) September 2009 daily rainfall data recorded at the Fiumedinisi rain gauge station.

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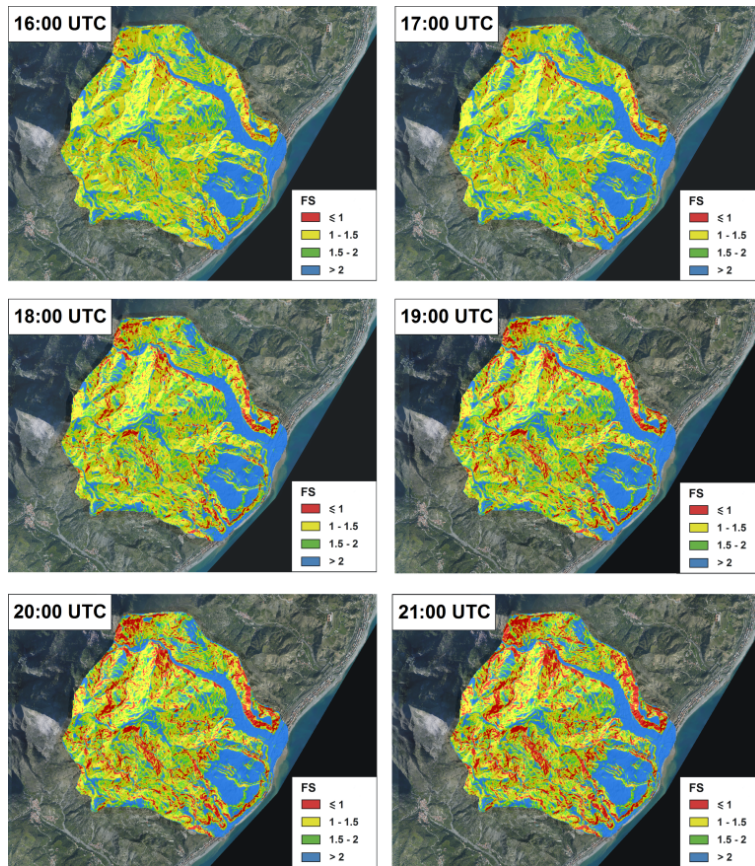


Figure 10. Slope stability conditions, expressed in terms of safety factor (FS), during the 1 October 2009 event according to TRIGRS model.

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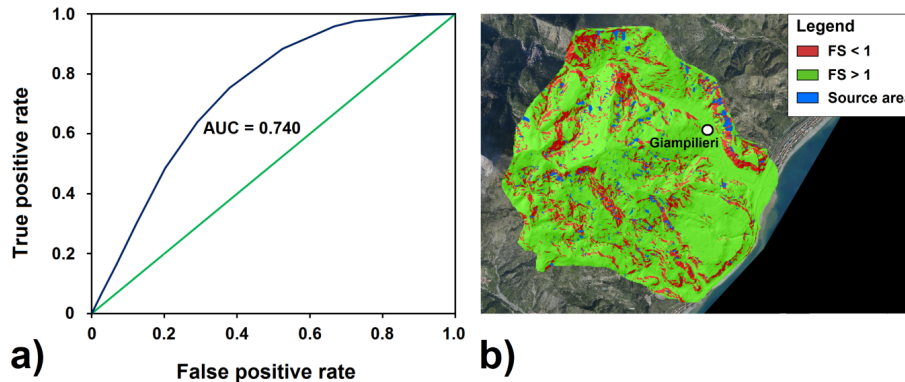


Figure 11. (a) ROC curve carried out by the comparison between TRIGRS final safety factor map and 1 October 2009 landslide inventory map; (b) graphic comparison between the two maps.

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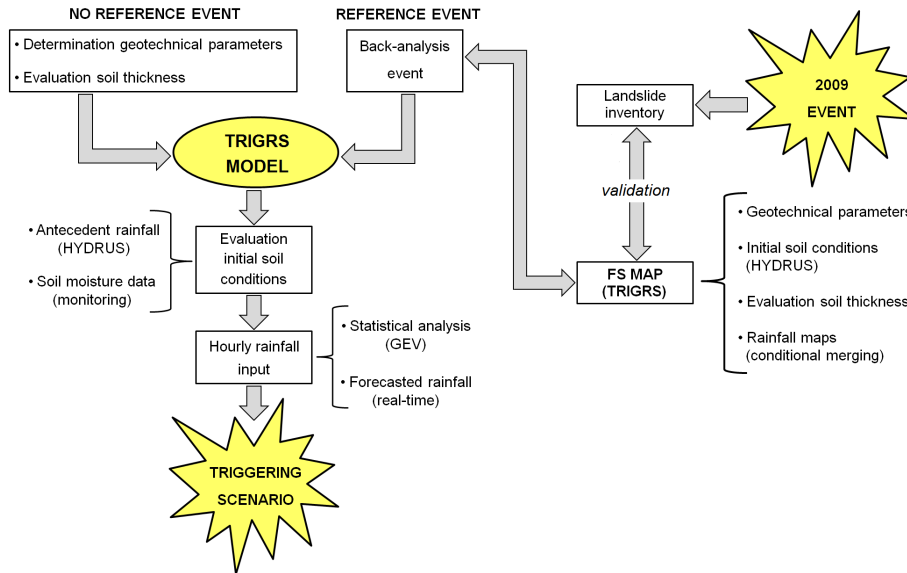


Figure 12. Flux diagram describing the proposed approach for the evaluation of shallow landslide triggering scenarios.

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