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Discussions

A comparative assessment of two different debris flow propagation approaches – blind simulations on a real debris flow event

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Assessment of two different debris flow propagation approaches

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Abstract

A detailed comparison between the performances of two different approaches to debris flow modelling has been carried out. In particular, the results of a mono-phase Bingham model (FLO-2D) and these of a two phase model (TRENT-2D) obtained from a blind test have been compared. As a benchmark test the catastrophic event of 1 October 2009 which struck Sicily causing several fatalities and damages has been chosen. The predicted temporal evolution of several parameters of the debris flow (as the flow depths and the propagation velocities) has been analyzed in order to investigate the advantages and disadvantages of the two models in reproducing the global dynamics of the event. Analysis between the models results with survey data have been carried out, not only for the determination of statistical indicators of prediction accuracy, but also for the application of the Receiver Operator Characteristic (ROC) approach. Provided that the proper rheological parameters and boundary conditions are assigned, both models seem capable of reproducing the inundation areas in a fairly good way. However, the main differences in the application rely in the choice of such rheological parameters. Indeed, within the more user friendly FLO-2D model the tuning of the parameters must be done empirically, with no evidence of the physics of the phenomena. On the other hand, for the TRENT-2D the parameters are physically based and can be estimated from the properties of the solid material, thus reproducing more reliable results. A second important difference between the two models is that in the first method the debris flow is treated as homogeneous flow, in which the total mass is kept constant from initiation in the upper part of the basin up to the deposition on debris fan. On the contrary, the second approach is suite to reproduce the erosion and deposition processes and the displaced mass can be directly related to the rainfall event. Application of both models in an highly urbanized area evidence the limit of numerical simulation that are inadequate to describe some disturbances of the flows occurred during alluvial event (es. the cars, the volume of debris within buildings etc.) which have crucial influence on the evaluation of the maximum and final flow depths.

1   Introduction

Debris flow events are among natural phenomena which still produce damages and fatalities. Therefore in the last decades many efforts have been put in order to develop models able to simulate numerically the debris flow propagation, aiming at producing reliable hazard maps. It is possible to find several propagation models applicable to hyperconcentrated flows, which mainly differ for the adopted rheological schemes. In particular, they can be distinguish in: single phase models and two phase models. Single phase models assumes that a debris flow acts as a homogenous Bingham fluid composed by a mixture of water and sediment. From a rheological view point, such a mixture can be described by Herschel–Bulkley or even more complex models as, for example, that described by quadratic law which assumes that the total friction stresses could be divided into different terms: yield stresses, viscosity stresses and turbulent-dispersive stresses; all of them being functions of the sediment concentration in the mixture. In any case, the hypothesis of the Binghamian nature of the fluid is necessary in order to simulate the arrest of the flow.

When the debris flows are treated as a two phase model, the exchange of mass between the erodible bed and the flow is taken into account as well. The fundamental of such models were first developed by Bagnold (1954) and then applied to the debris flows by Takahashi (1978). In such models the solid concentration is an unknown variable which influences the global behavior of the flow that can be properly accounted by the model itself.

In particular, it has been noticed that the first type of models are more suitable for case characterized by fine sediments, when the viscous shear rate is high. The second type of models are more suitable in case in which the viscosity of the interstitial fluid is negligible and the solid fraction is composed by coarser material, so when the inertial share rate acts predominately due to the collisions between gravels.

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Besides, as was stressed by Iverson et al. (1997) the debris flow during the propagation does not behave with a fixed rheology, since it changes in space and time its rheological characteristics.

In this framework in order to understand the real behavior of the propagation of a debris flow at large scale, a real catastrophic debris flow event has been analyzed by means of two different models: the FLO-2D (O'Brien, 1986) which is a single phase model; the TRENT-2D (Armanini et al., 2009) which is a two phase models. Both models adopt depth-integrated flow equations, though they assume different mathematical descriptions of the phenomenon.

In order to highlight the advantages and disadvantages between the two methodologies, they have been applied to a real complex case, namely, the alluvial event of 1 October 2009, which struck the Messina Province (Italy) causing 37 fatalities and several damages to public and private buildings and infrastructures. In particular several debris flows in the Giampilieri village, which was the most stuck during the alluvial event, have been simulated.

The paper is organized as follows: Sect. 2 presents a brief description of the two models; Sect. 3 describes the case study; Sect. 4 presents the boundary conditions along with data adopted as input for the two models; Sect. 5 reports the performed simulations; Sect. 6 shows the application of the ROC procedure to the case being. The paper ends with some conclusions about the main strengths and weaknesses of the two codes.

2   Models description

As mentioned before, the adopted models are based on depth-integrated flow equations, though they differ for the mathematical descriptions of the phenomenon. Indeed, the FLO-2D model, which is not fully two-dimensional, is based on a monophasic Bingham scheme, modelled through the quadratic rheological law developed by O'Brien and Julien (1985). The TRENT-2D is a fully two-dimensional model and it is a two-phase

rheological model based on the *dispersive shear stresses* by Bagnold. Another important difference is that in the FLO-2D the concentration of the solid phase is kept constant along all the debris flow and for such condition the bed is fixed, while in the TRENT-2D the bed is mobile and completely coupled with the dynamic of the mixture. More in deep model descriptions are provided as follows.

2.1 FLO-2D

FLO-2D is a commercial code developed by O'Brien (1986) worldwide adopted for debris flow phenomena modelling and delineating flood hazards. It is a pseudo two-dimensional model in space which adopts depth-integrated flow equations. Hyper-concentrated sediment flows are simulated considering the flow as a homogeneous (monophasic) non linear Bingham fluid, based on an empirical quadratic rheological relation developed by O'Brien and Julien (1985). The basic equations implemented in the model consist mainly in the continuity equation:

$$\frac{\partial h}{\partial t} + \frac{\partial (hV)}{\partial x} = i \tag{1}$$

and the equation of motion:

$$S_f = S_o - \frac{\partial h}{\partial x} - \frac{V}{g}\frac{\partial V}{\partial x} - \frac{1}{g}\frac{\partial V}{\partial t} \tag{2}$$

where: h = flow depth; V = depth-averaged velocity; i = excess rainfall intensity (assumed equal to zero in the present application); x is the generic direction of motion; S_f is the total friction slope; S_o is the bed slope; g = gravitational acceleration.

The surface topography is discretized into uniform square grid elements. In order to solve the momentum equation the FLO-2D considers, for each cell, eight potential flow directions. Each velocity computation is essentially one-dimensional and solved independently from the other seven directions, so h and V are related to one of the eight flow directions x.

The total friction slope S_f can be expressed as follows:

$$S_f = \frac{\tau_B}{\rho g h} + \frac{K \mu_B V}{8 \rho g h^2} + \frac{n^2 V^2}{h^{\frac{4}{3}}} \qquad (3)$$

where τ_B = Bingham yield stress; ρ = mixture density; K = the laminar flow resistance coefficient; μ_B = Bingham viscosity; n = pseudo-Manning's resistance coefficient which accounts for both turbulent boundary friction and internal collisional stresses. In particular, the yield stress τ_B, the dynamic viscosity μ_B and the resistance coefficient n are influenced by the sediment concentration relationships and, therefore, can be described by the following equations (O'Brien, 2007):

$$\tau_B = \alpha_1 e^{\beta_1 C_v} \qquad (4)$$

$$\mu_B = \alpha_2 e^{\beta_2 C_v} \qquad (5)$$

$$n = n_t 0.538 e^{6.0896 C_v} \qquad (6)$$

where C_v is the volumetric concentration and α_1, β_1, α_2 and β_2 are empirical coefficients defined by laboratory experiments by O'Brien and Julien (1988) and n_t is the turbulent n value by Julien and O'Brien (1998). More detailed information about the numerical scheme and the general constitutive fluid equations adopted can be found in O'Brien (2007).

2.2 TRENT-2D

TRENT-2D is a code developed by Armanini et al. (2009) for the simulation of hyperconcentrated sediment transport and debris flows. It is based on a two-phase approach, in which the interstitial fluid is water and the granular phase is modelled according to the dispersive pressure theory of Bagnold (1954), applied to the debris flows according to the adaptations introduced by Takahashi (1978). Since the reference model is biphasic, the concentration is one of the unknowns of the model and the bed is movable.

Moreover, the dynamics of the mixture and the morphological evolution of the bed are solved in a completely coupled way. This is quite important because wave celerities change noticeably from the fixed to the movable bed case. The model is based on the balance equations for mass and momentum (related to solid and mixture):

$$
\begin{cases}
\dfrac{\partial(z_B + h)}{\partial t} + \dfrac{\partial(hV_x)}{\partial x} + \dfrac{\partial(hV_y)}{\partial y} = 0 \\[2mm]
\dfrac{\partial(c_B z_B + ch)}{\partial t} + \dfrac{\partial(chV_x)}{\partial x} + \dfrac{\partial(chV_y)}{\partial y} = 0 \\[2mm]
\dfrac{\partial(c_\Delta V_x h)}{\partial t} + \dfrac{\partial\left[c_\Delta(V_x^2 h + \frac{gh^2}{2})\right]}{\partial x} + \dfrac{\partial(c_\Delta V_x V_y h)}{\partial y} + c_\Delta g h \dfrac{\partial z_B}{\partial x} = -F_{V_x} \\[2mm]
\dfrac{\partial(c_\Delta V_y h)}{\partial t} + \dfrac{\partial(c_\Delta V_x V_y h)}{\partial x} + \dfrac{\partial\left[c_\Delta(V_y^2 h + \frac{gh^2}{2})\right]}{\partial y} + c_\Delta g h \dfrac{\partial z_B}{\partial y} = -F_{V_y}
\end{cases}
\tag{7}
$$

where: x and y are the two directions of motion; V_x, V_y = depth-averaged velocities components along the x and y coordinates; z_B = bed elevation; c = solid phase concentration; c_b = solid concentration into the soil; $c_\Delta = (1 + \Delta c)$ where $\Delta = (\rho_s - \rho_w)/\rho_w$ = relative submerged density of solid phase (ρ_w and ρ_s are the densities of the water and of the solid respectively); F_{V_x}, F_{V_y} = friction terms components along the x and y coordinates. The friction term $F = F(|v|, h)$ derives from the Bagnold's relation, modified by Takahashi (1978) on the basis of experimental data:

$$
F = 25/4(1 + \Delta)\sin\varphi\,\lambda^2 Y^2
\tag{8}
$$

where φ is the friction angle and:

$$
\lambda = \left[(c_b/c)^{1/3} - 1\right]^{-1}; \quad Y = h/(d_{50}\sqrt{a})
\tag{9}
$$

where d_{50} is the median grain size and $a = 0.32$ is a constant determined in laboratory setting by Takahashi (1978). The concentration is computed as a function of the flow variables as:

$$c = \beta c_b (V^2/gh) \tag{10}$$

where the transport capacity β is a dimensionless parameter. The numerical scheme, second order accurate both in space and in time, is based on a finite volume, Godunov approach over a Cartesian structured grid. The scheme follows a MUSCL-Hancock explicit time stepping approach. The numerical fluxes at cell interfaces are computed using the LHLL Riemann solver as in Fraccarollo et al. (2003), which is able to account for the non conservative terms due to bed discontinuities. More details on the mathematical scheme and on the numerical model can be found on the TRENT-2D User's Manual (2011).

3 Case study description

During the night of 1 October 2009 a heavy rainfall struck the Messina Province causing 37 fatalities and several damages to public and private structures. This area located along the coast is characterized by the presence of the Peloritan Arc, which determines a particular morphology characterized by narrow river valleys with high hillslope angles (ranging in a interval of 30–60°) and with catchment basins of small or moderate extensions (ranging in a interval of 0.5–12 km^2) (Fig. 1). The soil is composed by metamorphic material, easy to be eroded, also because of the semi-arid climate, which is characterized by short and very intense rainstorms that occur between October and March. The event interested an area of more than 50 km^2, generating over 600 landslides in few hours. One of the most damaged villages was Giampilieri, a little village in the Southern Messina Province, located on the left bank side of the Giampilieri river. The town is characterized by a high-density urban area with narrow streets, that during alluvial events become the bed over which the runoff flows. Upstream of the Giampilieri

urbanized area, there are three different tributaries named, from West to Est, Loco, Sopra Urno and Puntale creeks respectively (Fig. 2a). All the mentioned tributaries are characterized by catchments of small extension, respectively of $0.15\,\text{km}^2$ for the Loco basin, $0.07\,\text{km}^2$ for the Sopra Urno basin and $0.04\,\text{km}^2$ for the Puntale basin. During the alluvial event of 1 October 2009, all the three catchments produced debris flows that hit the village and caused 19 fatalities. The overall effect of the rainfall event on the slopeland surface is easy to be recognize by means of a simple comparison between orthophotos gathered respectively before and after the alluvial event (Fig. 2b and c respectively); at a glance it is possible to have an idea of the huge debris flow magnitude involved.

4   Data referring to simulations

In order to model the debris flows, three principal data sets are needed: a Digital Terrain Model (DTM), hydrological data, and rheological properties of the sediment-water mixture. While the first two datasets are the same for the two models, the rheological properties are very different. The geometry inputs consist in the definition of a flood plain area. For the case being the measure of terrain elevation has been established from data coming from a detailed on site survey. The different grid systems implemented by FLO-2D model and by TRENT-2D model have been designed in such a way that a grid of square cells with cell size $1.5\,\text{m} \times 1.5\,\text{m}$ has been adopted for both models. In order to simplify the comparison between the different outputs a coordinate system has been set equal for the two different simulations. The presence of buildings inside the flood plain has been considered in both models, thus obtaining the same final configuration, although the implementation of such a feature is performed in FLO-2D and TRENT-2D in different ways. Indeed, the FLO-2D model is able to consider such a feature, attributing a reduction factor that accounts for the loss of storage and redirection of the flow path, while within the TRENT-2D the presence of buildings is implemented by enhancing the elevation of the cell where the contour layer of buildings itself is presented. In

the case being for both models the cells where the buildings are located have been considered not able to be inundated and thus to store debris flow volume.

Regarding the hydrological input, the hydrological inputs relative to the three mentioned have been considered. The input hydrographs have been determined using the data obtained from hydrological analysis, using a 300 years return period rainfall rate. The duration of the each hydrograph has been estimated from the concentration time of each basin. In order to determine the discharge rate value of the debris flows for each basin, the formula derived by Armanini et al. (2009) has been used:

$$Q_{df} = Q_l \frac{c_b}{c_b - c} \tag{11}$$

where Q_{df} is the discharge of debris flow, Q_l is the liquid discharge rate (given by the hydrograph); c and c_b, as already described, are the concentrations of the solid phase in the debris flow and into the soil, respectively. The debris flow concentration is calculated according to the following expressions:

$$\begin{aligned} c &= \frac{1}{\Delta} \frac{\tan \alpha}{\tan \varphi - \tan \alpha} \quad &\text{for} \quad \alpha \leq 21° \\ c &= 0.9 c_b \quad &\text{for} \quad \alpha \geq 21° \end{aligned} \tag{12}$$

where α if the slope angle in the upstream section of the model. The above relationships are consistent with the rheological assumptions of the TRENT-2D model. From the above equations it is possible to argue that c cannot exceed $0.9 c_b$, when the bed slope is higher than a fixed threshold (that in the case is equal to 21°) and that the slope α cannot exceed the friction angle φ. In addition, from Eq. (11) we get: $Q_{df} = 10 Q_l$.

It should be noted that in general the upstream input discharge Q_{df} is rather sensitive to the value of the bed slope α, but the value of upstream input discharge has minor influence in the downstream features of the debris flows in the TRENT-2D model, because of its capability to reproduce the erosion and deposition processes that along the flow are able to change the local discharge according to the local geometry. On the

contrary, this is not possible with the FLO-2D model, in which the discharge, instead, can be changed only modifying the excess rainfall intensity, otherwise it remains constant along its entire course. Nevertheless we have used Eq. (11) also to assign the input discharge in the simulation with FLO-2D. However this assumption induces in the FLO-2D model an error in the volumes, because the Eq. (11) includes also the liquid discharge, that, reasonably, contributes only partly to deposited volumes.

While for the Puntale and Loco creeks only one hydrological input has been considered, for the Sopra Urno basin three inputs have been implemented. Such an assumption relies on the observed event dynamics and from the analysis of the orthophoto gathered just after the event, in which it is easy to distinguish the three sub-catchments. Also, it has been assumed that the debris-flows originated from the three sub-catchments did not develop at the same times; but separated each other by 6 min. Such hypotheses are supported by videos of the event as well as by the comparison between the volumes calculated from the hydrographs and those resulting from the surveys taken after the event.

As regards to the rheological parameters, the FLO-2D model relies on empirical parameters which have no evidence of the physics of the phenomena, while the TRENT-2D parameters have a more specific physical meaning. In the case being has been effected considering an acceptable physical range.

The mono-phase modelling approach suffers for a need of calibration with historical data; while the more physically based model, such as the adopted two phase one, it is easy to be implemented. Indeed in order to perform the simulations with the FLO-2D model, the coefficients α_1, β_1, α_2 and β_2 (Eqs. 4 and 5) need to be estimated. Due to the mono-phase rheology the model is based upon, the parameters cannot be directly evaluated, and must be estimated. In particular the following values have been assumed: $\alpha_1 = 0.006032$; $\beta_1 = 19.9$; $\alpha_2 = 0.000707$; $\beta_2 = 29.8$. The latter parameters have been chosen from the ones available on the literature, trying to select those that have similar geomorphological and lithological characteristic to the one present in the studied area (O'Brien and Julien, 1988; Bertolo et al., 2005; Boniello et al., 2010; Wu

et al., 2013). While in order to perform the simulations with the TRENT-2D model, it is necessary to estimate the parameters φ, Y and β (Eqs. 8–10). As regard the friction angle φ, it can be determined by laboratory tests on the soil material of the study site. Here, it has been assumed $\varphi = 38°$. As for the parameter Y, it has been set equal to 10, considering an average value of its expression throughout the flow field (Eq. 9). Finally, as regards the transport capacity β it has been determined as explained in the TRENT-2D User's Manual (2011), and therefore $\beta = 6$ has been assumed.

5 Performed simulations

Regarding the simulation performed by the FLO-2D, a reconstruction of the inundated area has been obtained as output of the model. In particular, it is easy to recognize the portion of the urbanized area interested by the debris flow, which fits fairly well with the surveys effected just after the event. The maximum flow depths during the event obtained from the FLO-2D simulation are presented in Fig. 3a. The highest predicted flow depths were generated by the Sopra Urno creek, with a maximum value of about 6 m. Figure 3b represents the final flow depths, i.e. the debris flow depths after a time of 3 h. Note that, according to the mono-phase approach, there are no bed variations nor settlement of sediments separated from the liquid phase. The whole fluid stops when the bed stress goes under a threshold that depends on the fluid, thus determining the final deposition. The highest values of the predicted final flow depths are found in the streets perpendicular to the main path followed by the debris flows, with a maximum value of 1.2 m. Finally, the predicted maximum velocities are shown in Fig. 3c. It is easy to recognize that the maximum velocities are registered in correspondence of the upper part of the basins, where the slope are the highest, with values ranging from 10 to 20 m s^{-1}, while inside the urbanized area the velocities range from 1.5 to 5 m s^{-1}, although some peaks over 10 m s^{-1} are also observed.

As regards the simulation performed by TRENT-2D, the maximum flow depths reached during the simulations of the event are shown in Fig. 4a. The values are

generally smaller than those predicted by the FLO-2D model for the same case. On the other side, considering the thickness of the final deposition of material (Fig. 4b), the TRENT-2D predicted values are greater than those provided by the FLO-2D, in particular along the main path followed by the debris flows. Finally, the maximum velocities given by the TRENT-2D model are shown in Fig. 4c. They are generally smaller than those obtained using the FLO-2D, with velocities around $1–1.5\,\mathrm{m\,s^{-1}}$ along the main paths and smaller values elsewhere.

The results of both FLO-2D and TRENT-2D have been compared with the data gathered from on-site investigations, i.e. videos recorded during the event and measurements of the depth of the sediment deposition gathered just after the event. For example in Fig. 5 is presented a picture gathered after the alluvial event from which it is possible to individuated the maximum depth occurred in a determined section during the event. The latter data belongs to the depositional map that have been made after the alluvial event, where the levels of sedimentation material along the streets of Giampilieri village are indicated (see the Supplement). In particular, Fig. 6 shows an orthophoto of the urban area of Giampilieri with the positions of measurements gathered on purpose for the present study.

In particular, two field data are available: the values of the maximum flow depth reached during the event (h_{max}) and the values of the thickness of the sediment deposit left by the debris flow (dz_b). In Table 1, field data are reported, along with the corresponding predicted data obtained from FLO-2D and TRENT-2D. In the Table 1 we have reported only the points for which both flow depth and deposit data are available.

Considering maximum flow depths (h_{max}), FLO-2D predicted values are, in general, higher than those observed. This can be explained by considering that buildings cannot store debris flow volume and the uncertainty relative of the magnitude event. On the other side, TRENT-2D results are slightly smaller than those observed, with more accurate results along the main paths and greater deviations in the smallest streets.

Predicted values obtained from FLO-2D simulations are smaller than survey data; the reasons can be the value of the viscosity parameter assumed and also the absence

of anthropic features in the simulated scenario, such as cars along the streets, which during the event influenced the flow to a large extent. Looking at the TRENT-2D results, predicted final depositions are smaller than those measured. As for the flow heights, results are better in the main streets and less accurate in the lateral narrow ones.

Considering the maximum velocities (v_{max}), there are no field data available. Using FLO-2D, all values belong to the range from 1.5 to 20 m s^{-1}, while using TRENT-2D velocities are much smaller (1–2 m s^{-1} along the main paths and smaller values elsewhere). Such a difference is clearly due to the different rheologies adopted by the two models.

Moreover in order to have a more clear view of the error distribution inside the flooded area, in Fig. 7a and b are indicated at each surveyed point (see Fig. 6) with dots of different color the range of error in evaluating the maximum surface elevation, respectively for FLO-2D and TRENT-2D, considering a threshold of 30 % of error. In such a case it is possible to individuate as the FLO-2D simulation overestimate for the upper part of the basin, while the TRENT-2D is more accurate in the upper part and underestimate for more distant locations.

Finally, some statistical analysis on the models performances have been conducted. It has been determined for each model the mean absolute error and the root mean square error, which are 1.2 and 1.5 respectively for the FLO-2D and 0.9 and 1.2 for the TRENT-2D.

6 Comparison of the models performances, application of the ROC approach

In order to compare, not only by means of simple statistical indicators, the results obtained by the two debris flow models, the Receiver Operator Characteristic (ROC) approach has been applied. Such a method was originally developed to assess the performances of models in signal detection theory and then applied in different fields such as epidemiology, weather forecasting, machine learning, and landslide susceptibility (Baum et al., 2010).

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In a ROC graph the true positive rate (Sensitivity) is plotted vs. the false positive rate (Specificity) for a cut-off points. The sensitivity and the specificity are determined as follows:

$$\text{Sensitivity} = \frac{TP}{TP + FN}; \quad \text{Specificity} = \frac{FP}{FP + TN}. \tag{13}$$

In literature it is possible to find several application of ROC in cases of regional landslide susceptibility models that rely on the limit equilibrium calculations to evaluate the slope stability. In such a framework, the application of ROC is based on two states (stable and unstable) and on the basis of a grid analysis at each cell to which is assigned just one of the four outcomes possible (true positive TP, false positive FP, false negative FN, true negative TN).

A point in the ROC graph represents a sensitivity/specificity pair corresponding to a particular decision threshold. A test with perfect discrimination is located in the ROC graph in the upper left corner (100 % sensitivity, 0 % specificity). Therefore the closer a test is located in the ROC graph near the upper left corner, the higher the overall accuracy of the test (Zweig and Campbell, 1993).

In this framework the ROC approach has been applied in order to evaluate the prediction of the two debris flow propagation models. The ROC graph have been determined considering a cut-off value of 1.0 m of h_{max} flow depth. In particular the two states for the ROC approach are: cell with max flow depth $\geq$ 1.0 m and cell with max flow depth < 1.0 m, so the four outcomes are: (i) if the cell in the simulation is affected by the max debris flow depth $\geq$ 1.0 m and during the event it has been observed a max flow depth $\geq$1.0 m, the outcome is true positive TP; (ii) if during the event it has been observed a max flow depth < 1.0 m it is a false positive FP; (iii) if the cell during the simulation is not interested by a max debris flow depth $\geq$ 1.0 m and the event observation gives max debris flow depth value < 1.0 m, it is a true negative TN; (iv) while if during the event the max depth flow value in the cell is $\geq$ 1.0 m, it is a false negative FN. It is worth to point out that for the ROC analysis the data set of maximum flow depths surveyed and calculated from the FLO-2D and TRENT-2D simulation, presented already in Table 1,

have been extended of other two surveyed points. Such a expedient has been taken into account in order to recreate a more realistic data set, so also considering true negative conditions. Figure 8 shows a ROC graph comparing the performance in terms of h_{max} obtained by means of the application of FLO-2D and the TRENT-2D models, in such a case a threshold of 1.0 m have been selected, showing high accuracy of prediction made for both tools adopted. Parameters of accuracy and precision of the model results have been determined, following the application of the ROC approach, as follows:

$$\text{accuracy} = \frac{TP + TN}{TP + FN + FP + TN} \qquad (14)$$

$$\text{precision} = \frac{TP}{TP + FP}. \qquad (15)$$

The accuracy values obtained are 0.88 and 0.96 respectively for the FLO-2D model and the TRENT-2D model, while the precision value are respectively 0.76 and 0.96.

7 Conclusions

The simulation of the alluvial event of 1 October 2009, in Giampilieri has been reproduced by means of two different models, which are the FLO-2D (O'Brien, 1986) and TRENT-2D (Armanini et al., 2009). The FLO-2D model is based on a mono-phase approach, modelled through an empirical quadratic rheological relation developed by O'Brien (1986); moreover, it is not a fully two dimensional model. The TRENT-2D model is fully two-dimensional and its two-phase rheology model is based on the dispersive pressure theory by Bagnold. Another important difference is that in FLO-2D the bed is fixed, while in TRENT-2D the bed is mobile and completely coupled with the dynamic of the mixture. On the other hand, such a model does not easily simulate the presence of unerodible zones inside the floodplain because it is not possible to fix the bed in some computational cells. Moreover, for the FLO-2D model, the tuning of the parameters must be done empirically, with no evidence of the physics of the phenomena. On

the other hand, since the TRENT-2D parameters have a more specific physical base, it is easier to choose the right calibration values looking for them within an acceptable physical range. The time evolution of several parameters (as the deposit, the velocities, the volume of mixture involved in the event, etc.) have been then systematically analyzed in order to highlight the differences in the global dynamic of the event as obtained from the two codes. The results showed that both models seem capable of reproducing the depositional pattern in the alluvial fan in a fairly good manner, provided that the rheological parameters and the correct boundary conditions are assigned. In particular, FLO-2D tends to overestimate the flow depths for the reasons previously explained, while TRENT-2D slightly underestimates them. As regards the final depositions, they are slightly underestimated by both models. This results is probably due to the fact that the models cannot reproduce some disturbances of the flows occurred during the event (es. the cars, see Fig. 9) which had, important influence on the flow as demonstrated by several videos. Finally, FLO-2D velocities are generally higher than those predicted by TRENT-2D, due to the different rheological models. It is important to point out that accurate representation of the topography in the grid system is an essential step to obtain a reasonable replication of the observed deposition patterns. A more detailed spatial resolution of floodplain strongly improves the model results. Moreover results may also improve if the effects of flow obstructions, such as buildings, is incorporated into the model in a proper way. Possible explanations for the inaccuracy of the model results include both systematic topographic errors or the simplification of the real multi-surge event by a single triangular hydrograph. Bed level changes either between successive surges or at the base of a flow within one surge may cause a local change in the direction of the flow. The comparative analysis on the models results (FLO-2D and TRENT-2D) has been shown as TRENT-2D model is more accurate then FLO-2D, although the simulation is affected by distortion effect in evaluating the flow depth in region of the flood area far from the main path. The determination of some statistical indicators states that TRENT-2D prediction is more accurate if compared with the one of the FLO-2D. Moreover, the application of the ROC approach confirm a

general higher accuracy and precision of both models adopted for the simulation of the Giampilieri event. Although a slightly higher sensitivity level is determined in case of the FLO-2D when compared with TRENT-2D. Finally it can be state that both models are good in the reproduction of the flooded area. In order to produce hazard and risk mapping though the FLO-2D is more user friendly and the TRENT-2D is more accurate.

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Table 1. Measured and predicted values of maximum flow depths and thickness of final sediment deposition for the Giampilieri event, acquired by means of surveys, FLO-2D results and TRENT-2D results. For lack of event data records value of maximum velocities are provided only in case of model simulations.

POS.	Survey data		FLO-2D			TRENT-2D		
	h_{max}	d_{zb}	h_{max}	h_{final}	v_{max}	h_{max}	h_{final}	v_{max}
(no.)	(m)	(m)	(m)	(m)	(m s^{-1})	(m)	(m)	(m s^{-1})
1	2.4	2.0	1.4	0.4	2.7	1.2	1.1	0.6
2	1.8	1.0	2.7	0.8	1.5	1.4	1.2	0.7
3	1.5	0.8	3.9	0.9	2.0	0.8	0.6	0.5
4	1.7	1.0	4.4	1.1	4.1	0.8	0.5	0.4
5	2.0	1.5	4.5	1.1	4.1	0.9	0.5	0.6
6	3.3	2.0	4.5	1.1	3.5	1.1	0.5	0.6
7	3.0	0.0	4.0	0.5	3.8	1.4	0.6	0.8
8	2.2	0.0	4.8	0.2	6.3	1.4	0.7	0.9
9	3.3	0.0	1.5	0.3	3.6	1.4	0.8	0.6
10	1.3	0.0	1.2	0.3	5.0	1.1	0.6	0.3
11	2.8	2.0	1.8	0.1	2.0	1.7	0.3	1.0
12	1.7	0.0	0.1	0.0	0.1	1.4	0.7	0.4
13	2.1	1.0	2.0	0.7	1.7	2.2	1.4	1.1
14	2.0	0.0	0.2	0.0	2.0	2.1	1.8	0.3
15	1.3	1.0	2.5	0.2	3.5	1.4	1.0	0.4
16	2.6	0.0	3.1	0.1	5.0	2.7	1.1	1.3
17	2.1	0.0	4.7	0.5	5.0	2.3	1.3	0.9
18	0.9	0.0	2.3	0.1	2.8	1.2	0.3	0.3
19	2.4	1.5	0.4	0.1	3.5	0.4	0.2	0.4
20	2.2	0.0	1.6	0.4	0.6	1.8	0.1	0.3
21	2.4	0.0	0.0	0.0	0.4	2.0	0.3	0.2
22	2.9	0.0	0.1	0.1	0.1	0.7	0.1	0.7
23	2.0	1.0	2.3	0.5	0.8	1.2	0.4	0.7
24	2.8	0.5	2.1	0.4	0.8	0.7	0.3	0.4
25	2.3	1.5	2.2	0.4	0.7	1.2	0.6	0.5
26	1.9	1.8	2.1	0.1	5.0	2.7	0.6	0.7
27	2.0	1.5	4.0	1.0	1.9	2.1	1.0	0.4
28	5.0	0.5	2.8	0.1	1.3	1.4	0.3	0.1
29	5.0	0.0	4.5	0.5	8.0	5.5	0.7	1.4
30	2.0	0.5	1.7	0.7	3.0	2.3	0.3	0.3
31	1.5	1.5	0.0	0.0	0.3	0.3	0.1	0.2

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Interactive Discussion

Figure 1. Map of North-East Sicily struck by the debris flows phenomena of 1 October 2009.
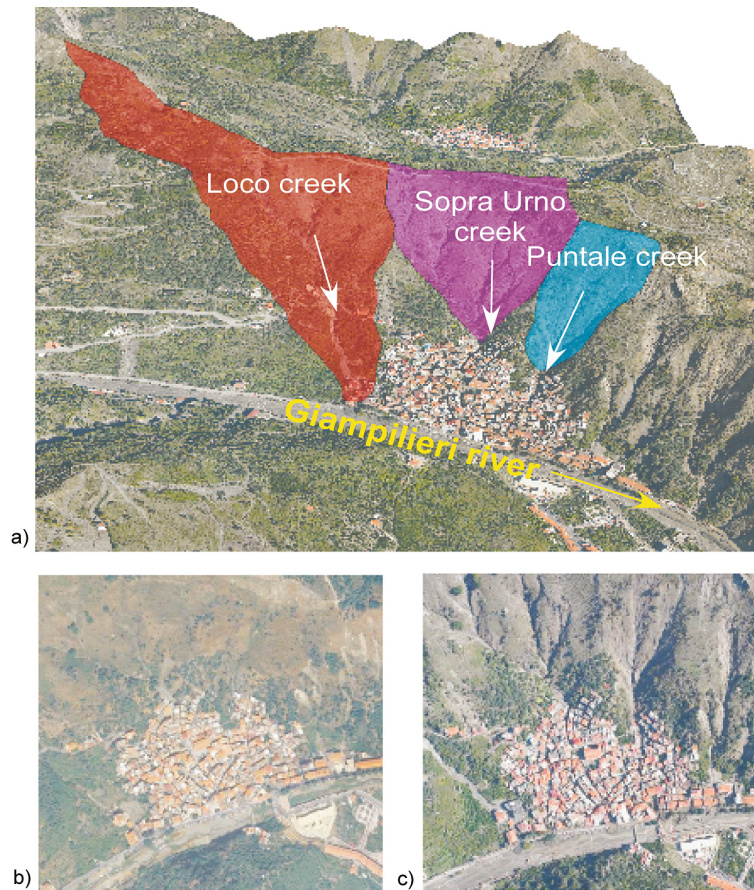
Figure 2. Ortophoto of Giampilieri village: **(a)** locations of Loco, Sopra Urno and Puntale Creeks and Giampilieri river are indicated; **(b)** the village previous the alluvial event; **(c)** the village after the alluvial event.
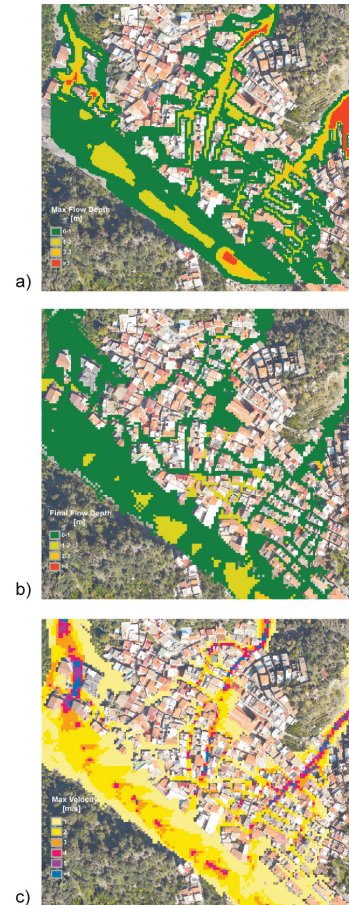
Figure 3. Scenarios simulated with the FLO-2D (hydraulic discharge relative to 300 years return period; rheological parameters: $\alpha_1 = 0.006032$; $\beta_1 = 19.9$; $\alpha_2 = 0.000707$; $\beta_2 = 29.8$): **(a)** maximum flow depth; **(b)** final flow depth; **(c)** maximum velocities.
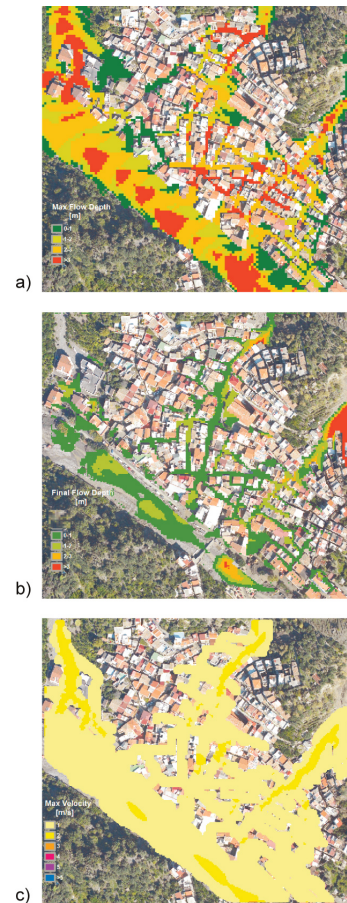
Figure 4. Scenarios simulated with the TRENT-2D (hydraulic discharge relative to 300 years return period; rheological parameters: $\varphi = 38°$; $Y = 10$; $\beta = 6$): **(a)** maximum flow depth; **(b)** depth of the final sediment deposition; **(c)** maximum velocities.

Figure 5. Evidences on the wall of the maximum depth observed during the event.

Figure 6. Orthophoto of Giampilieri urban area with positions where data of maximum flow depths and final sediment deposits are available from site surveys.
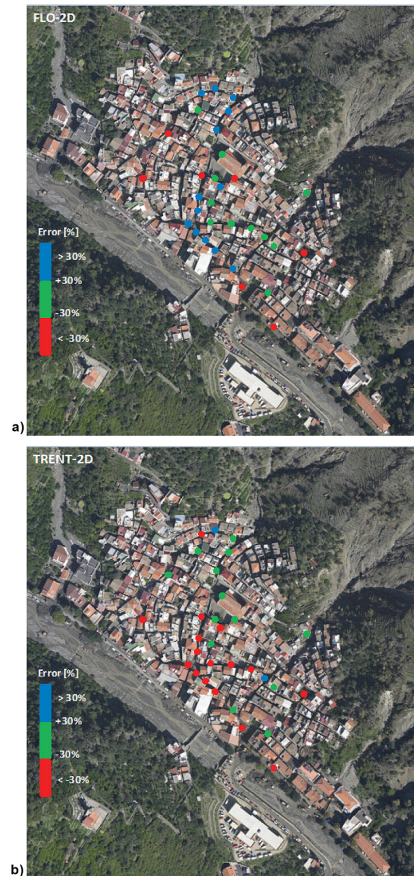
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Figure 7. Giampilieri village map where are indicated with colored dots the prediction error [%] respectively for: **(a)** FLO-2D and **(b)** TRENT-2D.
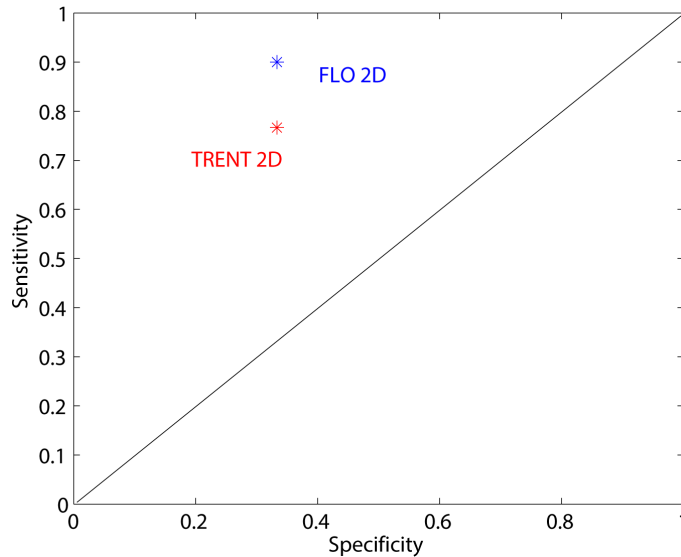
Figure 8. ROC graph related to a threshold value of 1.0 m, evaluating the performances of FLO-2D model and TRENT-2D model in prediction of the max flow depth.

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Figure 9. Street of Giampilieri village after the alluvial event of 1 October 2009, where the deposit material level has been influence by the presence of cars.