Nat. Hazards Earth Syst. Sci. Discuss., 1, 2547–2587, 2013 www.nat-hazards-earth-syst-sci-discuss.net/1/2547/2013/ doi:10.5194/nhessd-1-2547-2013 © Author(s) 2013. CC Attribution 3.0 License.



This discussion paper is/has been under review for the journal Natural Hazards and Earth System Sciences (NHESS). Please refer to the corresponding final paper in NHESS if available.

Landslide and debris flow susceptibility zonation using TRIGRS for the 2011 Seoul landslide event

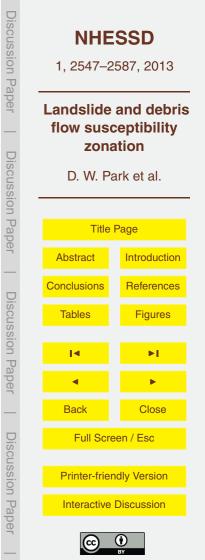
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Received: 23 April 2013 - Accepted: 22 May 2013 - Published: 5 June 2013

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Published by Copernicus Publications on behalf of the European Geosciences Union.



Abstract

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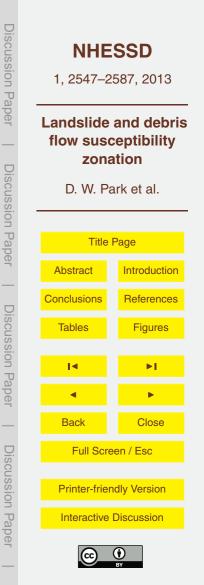
This paper presents the results from application of a regional, physically-based stability model: Transient Rainfall Infiltration and Grid-based Regional Slope-stability analysis (TRIGRS) for a catchment on Woomyeon Mountain, Seoul, Korea. This model couples

an infinite-slope stability analysis with a one-dimensional analytical solution to predict the transient pore pressure response to the infiltration of rainfall. TRIGRS also adopts the Geographic Information Systems (GIS) framework for determining the whole behaviour of a slope. In this paper, we suggest an index for evaluating the results produced by the model. Particular attention is devoted to the prediction of routes of debris
 flow, using a runoff module.

In this context, the paper compares observed landslide and debris flow events with those predicted by the TRIGRS model. The TRIGRS model, originally developed to predict shallow landslides, has been extended in this study for application to debris flows. The results predicted by the TRIGRS model are presented as safety factor (FS) maps corresponding to transient rainfall events, and in terms of debris flow paths using methods proposed by several researchers in hydrology.

In order to quantify the accuracy of the model, we proposed an index called LR_{class} (landslide ratio for each predicted FS class). The LR_{class} index is mainly applied in regions where the landslide scar area is not well defined (or is unknown), in order to avoid over-estimation of the model results. The use of the TRIGRS routing module was proposed to predict the paths of debris flow, especially in areas where the rhe-ological properties and erosion rates of the materials are difficult to obtain. Although an improvement in accuracy is needed, this module is very useful for preliminary spa-

- tiotemporal assessment over wide areas. In summary, the TRIGRS model is a powerful
- tool of use to decision makers for susceptibility mapping, particularly when linked with various advanced applications using GIS spatial functions.



1 Introduction

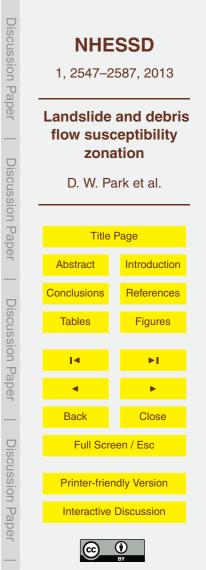
Shallow landslides involving colluvium are generally the most common in Korea and often mobilize into destructive debris flows. Shallow landslides are typically 1–3 m deep and often occur at boundaries between the colluvium and the underlying more solid

- ⁵ parent rock (Salciarini et al., 2008). In most parts of Korea, including Seoul, the thickness of the colluvium is generally less than 2 m because of the relatively shallow depth of the bedrock, and hence shallow landslides are frequent. Furthermore, the climate of Korea is typical of the Indian Ocean Monsoon, with pronounced seasonal precipitation (Kim et al., 2010). Thus, rainfall-triggered landslides are a recurring problem in 10 Korea. Due to the mountainous terrain with a shallow layer of colluvium, and associ-
- ated weather conditions, landslides have proven a hazard across most of the country. The socio-economic impact, moreover, has become much higher than before because of the current population levels in the hazardous zones.

During 26–27 July 2011, in particular, a heavy rainfall (470 mm in two days) occurred

- in Seoul, an amount approximately equal to 20% of the total annual rainfall for that region. During this precipitation event, 147 catastrophic landslides occurred on Mt. Woomyeon. Most of the landslides were accompanied by debris flows, and these mixtures of debris flowed down roads into the surrounding communities. Sixteen people were killed and ten buildings damaged by these debris flows. During the storm, shallow
- ²⁰ landslides on steep mountainous terrain were mostly triggered by heavy rainfall that increased the pore pressure of soil in the near-subsurface, with an attendant decrease in its shear strength. Under these conditions, precipitation-induced landslides caused translational mass movements that occurred suddenly.

In order to understand when and where rainfall-induced landslides have occurred in mountainous regions, and how topographic, geotechnical and hydraulic parameters affect the initiation of landslides and might be used to predict them, models adopting both empirical and deterministic approaches have been used.



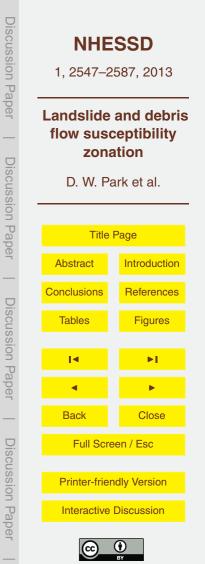
SMORPH (Shaw and Johnson, 1995), which stands for Slope MORPHology Model, is an empirical model adapted to include the contributing area with creeping process. One advantage of this model is that it only uses parameters derived from the Digital Elevation Model (DEM) to calculate susceptibility, and does not require field mapping.

In contrast, physically based or deterministic models are more frequently used for specific catchments, because there are physical descriptions that can be used to inform mathematical equations about slope failure processes. Five such models are introduced below, and compared with TRIGRS, the model upon which this work is focused.

LISA (Hammond et al., 1992) stands for Level I Stability Analysis. It identifies the effects of the tree root strength and tree surcharge on slope stability as an important parameter of forested, hill-slope areas. LISA enables the user to compute the probability of slope failure using up to 1000 iterations of a Monte Carlo simulation, by varying input values involved in the infinite slope equation. The Monte Carlo simulation estimates the probability of failure rather than a single factor of safety value.

- ¹⁵ SHALSTAB (Dietrich et al., 1993, 1995; Montgomery and Dietrich, 1994; Montgomery et al., 1998) stands for Shallow Land-sliding Stability Model. It is a coupled, steady-state runoff and infinite-slope stability model which can be used to map the relative potential for shallow sliding. This model correctly predicts the observed tendency for soils to be thick in the un-channelled valleys and thin on ridges.
- The dSLAM (Wu and Sidle 1995; Dhakal and Sidle, 2003), distributed Shallow Landslide Analysis Model, is a distributed, physically based model that combines an infinite slope model, a kinematic wave groundwater model, and a model simulating continuous changes in vegetation root strength, to analyze shallow, rapid landslides. This also includes results on the spatial distribution of safety factors in steep, forested terrain. This
- ²⁵ model is characterized by its focus on the stochastic influence of rainfall on pore water pressure.

SHETRAN (Ewen et al., 2000; Birkinshaw et al., 2010), which stands for System Hydrology European TRANsport, provides a hydrological and sediment transport framework for simulating landslides triggered by rain and snowmelt, along with sediment



yield. In this model, the occurrence of shallow landslides is predicted as a function of the time- and space-varying soil saturation conditions, using an infinite slope model for safety analysis.

SINMAP (Pack et al., 1998, 2001), for Stability INdex MAPping, was developed in
 British Columbia with the support of the Canadian government. Compared to other models, the slope stability model SINMAP has merit in that it calculates the potential slide risk for shallow translational slides via the specific-slope water-balance (Chinnayakanahalli, 2004).

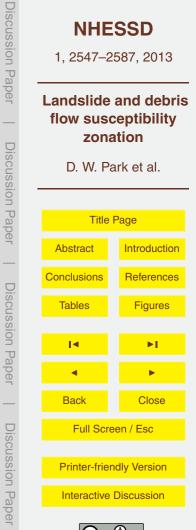
TRIGRS (Baum et al., 2002, 2008) stands for Transient Rainfall Infiltration and Grid based Regional Slope-stability. It is written in FORTRAN code and based on Iverson's (2000) linearized solution of the Richards equation, and the extension of that solution. The TRIGRS model, used for either saturated or unsaturated soils, is able to improve the effectiveness of susceptibility analysis by accounting for the transient effects of varying rainfall on conditions affecting slope stability. It has been used suc ¹⁵ cessfully around the world for quantitatively evaluating rainfall-triggered landslides, and a number of those applications follow.

 TRIGRS was used in a case study to account for the transient effects of rainfall on shallow landslide initiation, and verified with pilot catchments. Some examples being the Seattle area, Washington, USA (Godt et al., 2008); Mt. Tenliao, Taipei, Taiwan (Chen et al., 2005); and Mt. Gaemyung, Yangjoo, Korea (Kim et al., 2010).

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2. TRIGRS was used to evaluate and compare other physically based models including SLIP (Montrasio, 2011), SHALSTAB (Sorbino, 2010), SINMAP and LISA (Morrisey et al., 2001). The latter mentioned that Iverson's model, which is the basis of TRIGRS, would be preferred among the three models described above, because only Iverson's transient response model can assess stability conditions as a function of time and depth, on a regional scale, in areas prone to rainfall-induced landslide.



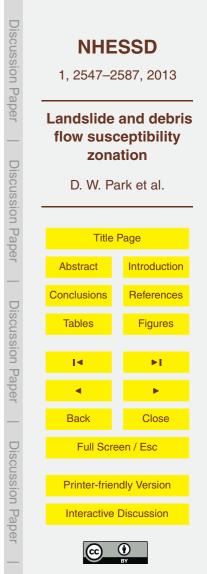
- 3. Some research has focused on parametric analyses to estimate material properties (Salciarini et al., 2006; Vieira, 2010). They proved that reasonable approximations of soil parameters, based on a limited number of measurements in the study area, were able to produce satisfactory results.
- The TRIGRS model was augmented with a statistical technique. In the probabilistic approach with TRIGRS, the simulated landslide potential map created was generally comparable to field observations when using the Monte Carlo simulation (Liu and Wu, 2008) and the General Extreme Value probability distribution (Salciarini et al., 2008).
- 5. TRIGRS code has been revised and converted for specific purposes. There is a probabilistic version, TRIGRS-P (Raia et al., 2013) and a Matlab version, Ma-TRIGRS (Liao et al., 2011). TRIGRS-P adopts a stochastic approach to compute and input parameters to be sampled randomly from a given probability distribution. MaTRIGRS offers unique computational efficiency in multi-dimensional matrix data and in real-time visualization of the simulation during modelling.

The main objective of this study was to predict shallow, rainfall-triggered landslides using TRIGRS, in the region of Woomyeon Mountain. The landslide ratio of each predicted class of safety factors was employed in evaluating the performance of the landslide model. Finally, this paper discusses the applicability of the flow routing model; then concludes with a discussion of the effectiveness of this approach and the potential for further research.

2 Study area

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The study area was Woomyeon Mountain, which is located in the Seocho district of Seoul City, South Korea (Fig. 1). It is located at $37^{\circ} 27'23''-37^{\circ} 28'55''$ N latitude and $126^{\circ} 59'20''-127^{\circ}01'28''$ E longitude. The elevation of Woomyeon Mountain is 293 m



above sea level. This area is completely encircled by buildings and roads amounts to $5\,104\,162\,\text{m}^2$ and is predominantly covered by forest, mostly oak trees.

The Mt. Woomyeon range is basically composed of Pre-Cambrian banded biotite gneiss and granitic gneiss as depicted in Fig. 2. The banded biotite gneiss was mod-

⁵ erately weathered and has stripes called gneissic banding, which develops under conditions of high temperature and pressure. Because of the gneissic banding, it is clear that the study area has been exposed to extreme shearing.

The soil profile can be divided into three main layers (Korean Geotechnical Society, 2011):

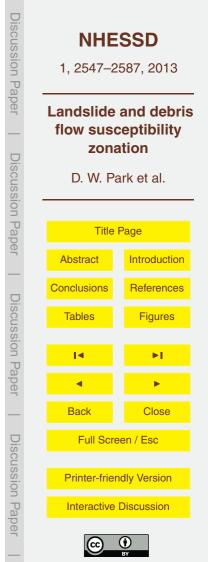
- A colluvium layer extends to a maximum depth of 3.0 m from the ground level and the upper part of this layer was formed from previously transported soil. This layer is generally loose material composed of gravel and silty-sand, according to the Unified Soil Classification System (USCS), a heterogeneous, incoherent and permeable soil.
- A transition zone is composed of mainly a clay layer (thickness is 0.2 m to 0.5 m below colluvium layer) characterized by the colors taupe and dark brown. It was anticipated that landslides would be generated by conditions in this layer between the colluvium and bedrock.
 - A subsoil of stiff weathered bedrock is followed by a clay layer. This subsoil layer can be considered impervious according to the low hydraulic conductivity indicated by the modelling that follows.

3 Landslide event

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In the area of Mt. Woomyeon, about 147 catastrophic, shallow, landslide events were triggered by a localized torrential rainfall from 26 July to 27 July in 2011. Most of the landslides were accompanied by debris flows, and mixtures of debris flowed down the



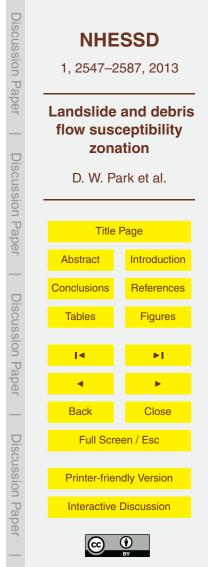
roadways into local communities. Sixteen people were killed and ten buildings were damaged by the debris, leading to economic losses of about US\$15 million. Figure 3 depicts the locations of the damaged districts (deaths, buildings, inundated areas, land-slide scarps and landslide areas) after the disaster.

- Figure 4 contains all the landslides and debris flows documented for this event on 27 July 2011. These have been registered in an official archive of disaster survey reports and publications for the government of Seoul by the Korean Society of Civil Engineers. To recognize shallow landslides, satellite images and aerial photographs taken after landslide events, as well as during field surveys, were used to spatially describe the geometry is features of the landslide area. Figure 4a deniate landslide and debria
- the geomorphic features of the landslide area. Figure 4a depicts landslide and debris flow locations, and this image is suitable for identifying and mapping the landslides of Fig. 4b. The landslide map identified 147 individual landslides with a landslide density of 29 landslides km⁻². Most of the landslides were transformed into translated debris flows as plotted in Fig. 4.

15 4 Theoretical basis

4.1 Description of the TRIGRS model

TRIGRS (Baum et al., 2008) models rainfall infiltration, resulting from storms that have durations ranging from hours to a few days. To do so, it uses analytical solutions of partial differential equations that represent one-dimensional, vertical flow in isotropic,
 homogeneous materials for either saturated or unsaturated soil conditions (Fig. 5). This combines the theoretical bases of the models for infiltration and subsurface flow of storm water, routing of runoff, and slope stability, to calculate the effects of rainfall on the analysis of stability over large areas. Following is a brief description of the models and formulas used by TRIGRS to represent these processes.



4.1.1 Infiltration model

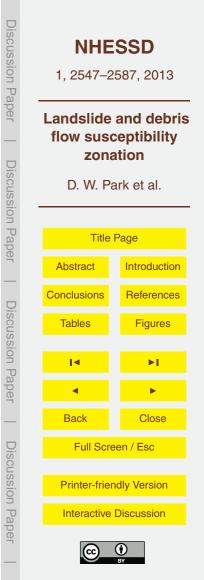
The infiltration models in TRIGRS for initial wet conditions are based on Iverson's linearized solution of the Richards equation and extensions by Baum et al. (2002, 2008) to that solution. TRIGRS also uses a series of Heaviside step functions to implement ⁵ Iverson's suggested summation of his original solution for rainfall of constant intensity, to represent a general time-varying sequence of surface fluxes of variable intensities and durations. As an alternative to the solution with an infinitely deep basal boundary, Baum et al. (2002, 2008) added to TRIGRS a solution for pore pressure in the case of an impermeable basal boundary at a finite depth d_{LZ} . The pore pressure for an impermeable basal boundary at a finite depth is given by:

$$\psi(Z,t) = (Z-d)\beta + 2\sum_{n=1}^{N} \frac{l_{nZ}}{K_S} H(t-t_n) [D_1(t-t_n)]^{\frac{1}{2}} \sum_{m=1}^{\infty} \left\{ \operatorname{ierfc}\left[\frac{(2m-1)d_{LZ}-(d_{LZ}-Z)}{2[D_1(t-t_n)]^{\frac{1}{2}}}\right] + \operatorname{ierfc}\left[\frac{(2m-1)d_{LZ}+(d_{LZ}-Z)}{2[D_1(t-t_n)]^{\frac{1}{2}}}\right] \right\} - \frac{1}{2} \left[- \frac{1}{2} \sum_{n=1}^{N} \frac{l_{nZ}}{K_S} H(t-t_n) \left[- \frac{1}{2} \sum_{n=1}^{N} \frac{l_{nZ}}{2[D_1(t-t_n)]^{\frac{1}{2}}} \right] \right] + \operatorname{ierfc}\left[\frac{(2m-1)d_{LZ}+(d_{LZ}-Z)}{2[D_1(t-t_n)]^{\frac{1}{2}}} \right] \right]$$

$$+2\sum_{n=1}^{N}\frac{l_{nZ}}{K_{\rm S}}H(t-t_{n+1})[D_{1}(t-t_{n+1})]^{\frac{1}{2}}\sum_{m=1}^{\infty}\left\{\operatorname{ierfc}\left[\frac{(2m-1)d_{\rm LZ}-(d_{\rm LZ}-Z)}{2[D_{1}(t-t_{n+1})]^{\frac{1}{2}}}\right] +\operatorname{ierfc}\left[\frac{(2m-1)d_{\rm LZ}+(d_{\rm LZ}-Z)}{2[D_{1}(t-t_{n+1})]^{\frac{1}{2}}}\right]\right\}$$
(1)

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where ψ is the ground-water pressure head; $Z = z/\cos\delta$, where Z is the vertical coordinate direction (positive downward) and depth below the ground surface, z is the slope-normal coordinate direction (also positive downward), and δ is the slope angle; d is the steady-state depth of the water table measured in the vertical direction; d_{LZ} is the depth of the impermeable basal boundary measured in the vertical direction; $\beta = \cos^2 \delta - (I_{ZLT}/K_S)$; K_S is the saturated hydraulic conductivity in the Z direction; I_{ZLT} is the steady (initial) surface flux; I_{nZ} is the surface flux of a given intensity for 2555



the *n*th time interval; $D_1 = D_0 / \cos^2 \delta$, where D_0 is the saturated hydraulic diffusivity $(D_0 = K_S / S_S)$, where K_S is the saturated hydraulic conductivity and S_S is the specific storage); *N* is the total number of time intervals; $H(t - t_n)$ is the Heaviside step function and t_n is the time at the *n*th time interval in the rainfall infiltration sequence; $\operatorname{erfc}(\eta)$ is the complementary error function: $\operatorname{ierfc}(\eta) = \frac{1}{\sqrt{\pi}} \exp(-\eta^2) - \eta \operatorname{erfc}(\eta) m$ is index of infinite series displaying odd term in complementary error function.

4.1.2 Slope stability model

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The model of slope stability, using an infinite-slope stability analysis, is characterized by the ratio of resisting friction to gravitationally induced downslope driving stress. $F_{\rm S} < 1$ denotes unstable conditions and the depth *Z* where FS first drops below "1" will be the depth of landslide initiation. The equation to calculate the safety factor of the slope according to the infiltration of rainfall for an infinite slope model is given by:

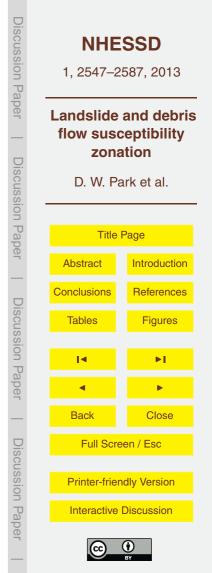
$$F_{\rm S}(Z,t) = \frac{\tan\phi'}{\tan\delta} + \frac{c' - \psi(Z,t)\gamma_{\rm W}\tan\phi'}{\gamma_{\rm S}Z\sin\delta\cos\delta}$$

where c' is soil cohesion for effective stress; ϕ' is the soil friction angle for effective stress; γ_W is unit weight of groundwater; γ_S is unit weight of soil.

4.1.3 Runoff model

TRIGRS computes the infiltration of each cell. The amount of infiltration, *I*, is the sum of the precipitation, *P*, and runoff from adjacent cells, R_u , if the infiltration cannot exceed the saturated hydraulic conductivity, K_s , as:

$${}_{20} \quad I = \frac{P + R_u}{K_{\rm S}} \quad \frac{P + R_u \le K_{\rm S}}{P + R_u > K_{\rm S}}$$



(3)

The runoff, R_{d} , is calculated by the following equation:

 $R_{d} = \frac{P + R_{u} - K_{S}}{0} \qquad \frac{P + R_{u} - K_{S} \ge 0}{P + R_{u} - K_{S} < 0}$

Further theoretical details of the model have been fully described in TRIGRS open file reports (Baum et al., 2002, 2008).

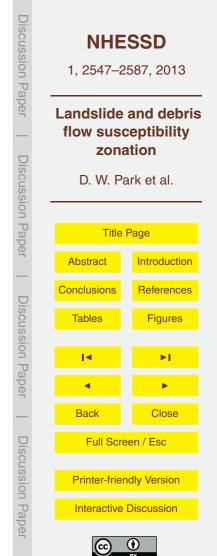
5 4.2 Description of the flow routing model

The model TRIGRS, uses a method for routing runoff flow cell-by-cell in the mass balance calculations. Several methods for the representation of flow directions, using rectangular grid digital elevation models, are presented later, along with flow routing features. Figure 6 shows the designation of the eight flow directions used by following flow routing model, and the numbering scheme according to ESRI direction codes in ArcGIS. This approach is commonly referred to as an eight-direction (D8) flow model because of the eight valid output directions relating to the eight adjacent cells into which flow could travel. Based on the Grid DEM, there are many models for predicting the flow.

15 **4.2.1 The D8 method**

The earliest and simplest method for estimating flow directions, is to distribute flow from each cell to one of its eight neighbors, on the steepest downslope path. That is, flow is diverted only to the one neighboring cell that is on the steepest direction. This model, named the D8 method, was suggested by O'Callaghan and Mark (O'Callaghan

²⁰ and Mark, 1984), and has been widely used. In the D8 method approach, however, the resulting flow distribution is irregular and somewhat unrealistic, because flow can occur in only steepest direction, either adjacent or diagonally, of eight possible directions.



4.2.2 The Multiple flow direction method

Multiple flow direction method (Quinn, 1991) offers advancement over the D8 method (restricts flow to one among eight possible directions). The fraction of the flow through each grid cell to each downslope direction is proportional to the gradient of each down-

⁵ hill flow path, so that steeper gradients will naturally attract more of the stream area. Every cell surrounding the central point can be flow directions if they have lower elevations than the initial one. The pattern, of course, becomes more strongly concentrated toward the steepest downslope path. Equation (5) expresses the relative amounts for the downhill directions.

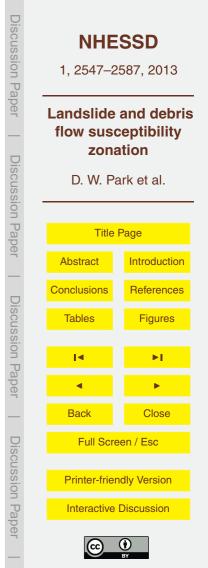
¹⁰
$$\Delta A_i = A(\tan \beta_i L_i) / \sum_{j=1}^n (\tan \beta_j L_j)$$

where *n* is the total number of downhill directions; ΔA_i is the amount passed onto the *i*th downhill cell; *A* is the total upslope area accumulated in the current cell; $\tan \beta_i$ is the gradient (difference in elevation/distance between the elevation values) in the *i*th downhill directions; L_i is the contour length of the *i*th direction either cardinal or diagonal;

4.2.3 The D-infinity method

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Tarboton's D-infinity method (Tarboton, 1997) assumes that water flows down one or two cells by partitioning the flow between the two cells nearest to the steepest slope direction. Figure 7 illustrates the calculation of flow directions. The single flow direction is determined by the steepest downwards slope among the eight triangular facets. This direction is calculated by apportioning flow between two downslope pixels according to how close the flow direction is to the direct angle to the downslope pixel. In other words, the procedure is based on representing the flow direction as a single angle taken as the steepest downwards slope among the eight triangular facets. Like for the



(4)

Multiple flow direction method, some dispersion is generated by the proportioning of flow between downslope cells, but this is minimized since flow is never diverted to more than two downslope cells. Compared to other models, the D-infinity method was preferred because it is physically more realistic. The D-infinity method is also used in the program SINMAP.

Table 1 presents a summary of the flow routing methods used in simulations. The TRIGRS runoff module is used to compare the flow direction routing models above, with the observed debris flow routes in the study area.

5 Application of the model

10 5.1 Rainfall characteristics

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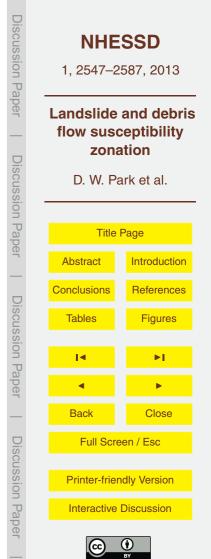
There are two meteorological monitoring stations (Namhyun and Seocho) near the Mt. Woomyeon region. All weather stations are operated by the Korea Meteorological Administration.

The climate in the study area is mainly characterized by an average annual rainfall of 1400–1500 mm; highest in July and lowest in January. The climatic conditions occurring in July 2011, however, were significantly different from the average. During July alone, Mt. Woomyeon received about 55 % of its total annual precipitation of 2039 mm. Hourly maximum rainfalls were 114 mm h⁻¹ (07:44 ~ 08:44 UTC on 27 July) and 87 mm h⁻¹ (07:41 ~ 08:41 UTC on 27 July). The first record was at Namhyun Station, the second

²⁰ one at Seocho Station. From the Intensity-Duration-Frequency (IDF) curves for Seoul City, the rainfall recurrence intervals were 120 and 20 yr, respectively.

Figure 8 shows the hourly rainfall history from 25 July to 27 July in 2011. Shallow landslides were triggered by the localized torrential rainfall during this period, characterized by a cumulative rainfall of about 350 mm, of which 42 % poured down during the location hours (2000) 1000 1100 hours the failures. The located data at 0.5 mm

last two hours (06:00 ~ 09:00 UTC) before the failures. The landslides started at 9 a.m. on 27 July.



5.2 Input parameters

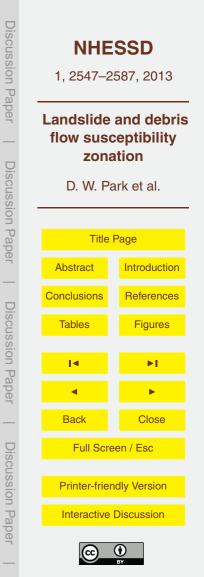
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Many important parameters are involved in the TRIGRS model, for example, topographic factors, soil thickness, as well as strength properties and hydraulic parameters of the soil. Accuracy and reliability of the results depend mainly on a detailed knowledge of the study site, and on the quality of the input parameters.

For Mt. Woomyeon, topographic analyses for elevation, slope angle and aspect were calculated from 1 : 5000 maps developed by the National Geographic Information Institute. The ArcGIS was used to create grids with 10 m cells and to quantify the aforementioned information above for each cell of the DEM.

- ¹⁰ All available data were obtained from the engineering geological investigation for landslides hazards restoration work conducted by the National Forestry Cooperative Federation, Korean Society of Civil Engineers and Korean Geotechnical Society. After the landslides occurred on 27 July, a total of 58 geotechnical investigation boreholes were drilled for collecting soil, hydrologic and geological information. Among these,
- available data from 13 boreholes and 19 soil samples were used in this analysis. The locations of the investigation boreholes and profiles are depicted in Fig. 9. Determination of the soil water characteristic curve was accomplished by using pressure plate extractor and filter paper method as shown in Fig. 10. Using van Genuchten formula was best fits among several fitting equation. From the soil-water retention curves, saturated and residual volumetric water content were 50 % and 18 %, respectively.
 - Hydraulic parameters including hydraulic saturated conductivity (K_S), diffusivity (D_0) and steady infiltration rate (I_Z) were obtained from laboratory tests, and derived according to soil classes and empirical references. The values of D_0 and I_Z

were not well defined, since they had wide ranges according to the complex prop-²⁵ erties of soil (e.g., void, fine content, and soil density). These parameters are quite different for various samples, even though they were collected from the same site. In the literature review about colluvium soil, D_0 was discovered to have a value about 10–500 times the value of the hydraulic conductivity. For this reason, the D_0 value was



assumed to be 200 times of the K_S (Liu and Wu, 2008). The information about the I_Z rate, however, is rare in the literature. The I_Z value is affected by soil characteristics including porosity, storage capacity, and transmission rate through the soil. The soil texture and structure, vegetation types and cover, water content of the soil and soil tem-

- ⁵ perature also play a role in controlling the infiltration rate. If the soil is saturated, I_Z can be the same as hydraulic conductivity, while it can be zero for dry soil. In this research, the reasonable value 0.01 of the K_S (Salciarini et al., 2008; Liu and Wu, 2008; Kim et al., 2010) was selected for I_Z , because of the hot, dry conditions during the summer of this event. In the simulations carried out in this study, uniformly homogeneous soil
- ¹⁰ conditions were considered present due both to similar soil properties in reports, and the presence of almost the same geological conditions. The input values, and units of the parameters for analysis, are listed in Table 2.

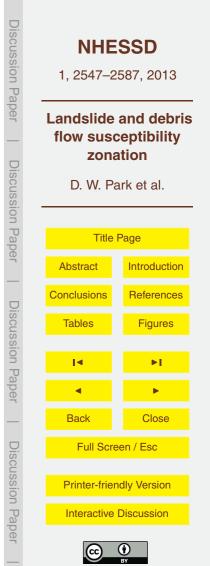
6 Results and discussion

6.1 Elevation, slope, aspect and curvature

¹⁵ The simulations described below, were carried out at two levels: (i) considering landslides of the study area where 147 shallow landslides occurred during an intense rainfall event in July 2011, and (ii) taking into account the debris flow routes, but not debris mass, velocity and deposits.

In the study area, elevation, slope, aspect and curvature, all of which are relevant to landslides, were calculated from topographic information. These results for both the study area, and the landslide occurrence points, are shown in Fig. 11. Bar graphs represent the percentage of the area of each category, in relation to the total study area. Polygonal lines are the ratio of the number of landslides in each category, in relation to the total number of landslides.

²⁵ Seventy percent of the study area was between 50 m and 150 m elevation, and 67 % of the slope angles were between 10 and 25 degrees. Most of the landslides were



triggered on terrain with mid-to-high altitudes ranging from 100–250 m (average 119 m), and on steeper slopes (> 25° , average: 19°). The aspect ratios in each category were similar to each other while the largest landslide orientation was east with 18%. Curvature graph showed nearly the same number of concave and convex profiles in the study area, but, concave was predominant in failure spots.

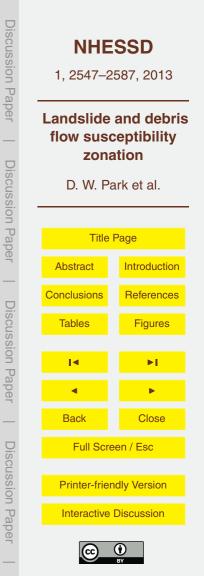
6.2 Prediction of landslides

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One main objective of this research was to evaluate the spatiotemporal predictability of landslide events in Mt. Woomyeon, using the TRIGRS model for regional landslide hazard assessment. The factor of safety (hereafter FS) over the entire study area was calculated for each cell, and plotted over time during this severe storm. Figure 12a, b and c show the spatial distributions of FS in different periods of time. In other words, these depict the temporal and spatial dynamics of FS values induced by heavy rainfall during the 48 h duration (from 9 a.m., 25 July to 9 a.m., 27 July). The three FS maps are for the 0, 46, and 48 h, and 46 h corresponded to the start of extraordinarily heavy rain-

- fall for two hours. The TRIGRS model correctly simulated the time that the landslides were triggered. The areas characterized as having a safety factor close to FS = 1.0 progressively expanded when the rainfall became more intense. This implies that large numbers of the landslides were triggered by the intense rainfall. The Korean peninsula has a lot of curvy and steep nature of mountainous terrain. This is why FS maps are very complex and tortuous. Nevertheless, the performance of the TRIGRS model for prediction, which has been evaluated by field investigation, can be considered reason-
- prediction, which has been evaluated by field investigation, can be considered reasonably applicable as shown in Fig. 12c.

The success of landslide prediction models has been typically evaluated by comparing locations of measured landslides with the predicted results. Thus, a proper index or an estimator for measuring performance is essential. Most previous studies (Crosta et al., 2003; Salciarini et al., 2006; Kim et al., 2010; Vieira et al., 2010) used agreement of parts (cells) between the predicted and the actual landslides to evaluate the performance of their models.



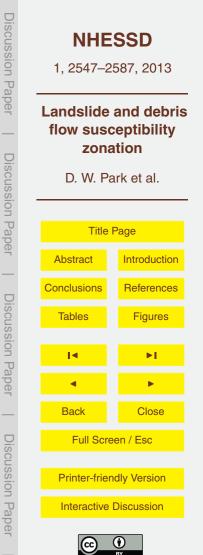
However, it can be seen that the model output with more unstable areas are better than the underestimated results, since it covers more landslides. An ideal landslide assessment model simultaneously maximizes the agreement between known and predicted locations of landslides, and minimizes predicted unstable area to give useful information for prediction. In order to overcome the disadvantages and limitations of such comparisons, various indices have been proposed: SR and MSR stand for Success Rate and Modified Success Rate (Huang and Kao, 2006), ROC stands for Receiver Operating Characteristic using confusion matrix (Godt et al., 2008; Montrasio, 2011; Raia et al., 2013), SI and EI stand for Success Index and Error Index (Sorbino et al., 2010), SC and LP stand for Scar Concentration and Landslide Potential (Vieira et al., 2010), POD, FAR and CSI stand for Probability Of Detection, False Alarm Ratio

and Critical Success Index (Liao et al., 2011) and the *D* index (Liu and Wu, 2008). Although the performance factors above are useful for quantifying the accuracy of a model, the precise area of known landslides is necessary for applying them. In the Mt.

- ¹⁵ Woomyeon event, it is difficult to know the size of the landslides that occurred, since a number of the debris flows occurred after the landslides. Figure 4a depicts this problem associated with unclear boundaries of landslides. Most landslide areas are connected with debris flow channels. This is the reason why counting landslide sites, instead of calculating landslide area, was used in the following.
- In this paper, the landslide ratio of each predicted FS class (hereafter LR_{class}) was employed for evaluating the performance of the landslide model. LR_{class} was based on the ratio of landslide sites contained in each FS class, in relation to the total number of actual landslide sites (total 147 spots), according to the predicted percentage of area in each class of FS category.

 ${}_{^{25}} LR_{class} = \frac{\% \text{ of contained landslide sites in each class of FS}}{\% \text{ of predicted landslide areas in each class of FS}}$

Note that in the numerator, the number of landslide sites, instead of the number of landslide cells, is used because of the difficulty in defining boundaries between landslides



and debris flows. The performance value derived by LR_{class} enables consideration of predicted stable areas as well as predicted unstable areas, and thus substantially reduces the over-prediction of landslide potential. Unlike the numerator, the number of predicted and total cells is used in denominator. The numerator, also, is the same as the SR (Success Ratio) index.

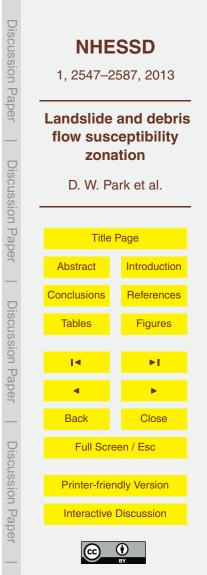
Table 3 and Fig. 13 show that 2.99% of the area was classified as unstable (FS \leq 1.0), and that 33.33% of the actual landslides were correctly localized within this predicted unstable area. LR_{FS<1} was about 11 with 33.33% over 2.99%. By calculating the % of LR_{class}, we can get quantitative result. The % of LR_{FS<1} is 70.30%, in other words, if a landslide happens, then predicted unstable area (FS < 1) has 70.30% chance of including the landslide. In other words, lower safety factor classes showed higher values of LR_{class} percentages. The results show significant agreement between the simulated and known landslide map from a quantitative point of view, despite missing information of landslide area.

15 6.3 Prediction of debris flow routes

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During severe storms, the failed soil mass rapidly propagates downslope and increases its initial volume through erosion of in-place soils producing a dangerous mobilized volume called a debris flow. A large number of landslides evolved into debris flows during the torrential rainfall from 26 to 27 July 2011, in Seoul, Korea. The average length of debris flows in the study area was about 317.0 m, with an average volume of 269 m³. The biggest debris flow mapped has a length of 632 m while the smallest is less than several tens of meters (Korean Society of Civil Engineers and Korean Geotechnical Society, 2012).

In this study, we suggested a debris flow routing method for the TRIGRS runoff mod-²⁵ ule. This concept aims at giving a prompt assessment of debris flow path analysis at a regional scale with minimum data requirement. Most debris flow susceptibility models obviously depend on a lot of information about the area of interest. Due to the complexity of debris flows relative to the modelling parameters, a simplified model was required



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The second interesting issue arises from the comparison of two runoff schemes in the module as shown in Fig. 14c and d. The runoff scheme in TRIGRS can calculate the amount of flow through each cell, and use various hydrological routing models. The results from the two model applications shown in Fig. 14 reveals the model outputs

module has the advantage by predicting susceptible zone for future extreme storm.

results of the debris flow routing module. 15 First, comparing the results with the debris flow inventory map, good agreement can be found between the predicted debris flow paths and the inventory, despite the paucity of parameters for rheological properties and erosion rate. It was shown that the topography of the DEM is an important factor to determine debris flow propagation (Horton et al., 2011). In the framework of debris flow mapping, the predicted results 20

have more routes than the observed debris flow routes shown in Fig. 14a. This is because the predicted results should be representative of the worst cases; i.e., flow

in every potential route even where a landslide did not occur. To solve this problem, a coupled analysis with landslide and debris flow is needed. On the other hand, this

flow susceptibility by different models: Flow-R, D-infinity method and Multiple flow direction method, respectively. Figure 14c and d indicate two interesting points about the

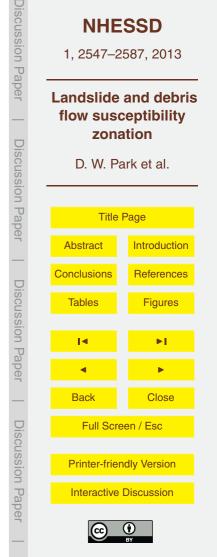
tograph and satellite imagery. Figure 14b, c and d show spatial distributions of debris

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et al., 2013) has been chosen as the reference method. Flow-R is a distributed, semi-5 empirical model for susceptibility assessments of debris flow developed at University of Lausanne. It assesses propagations using three critical factors of sediment availability, water input and gradient (Takahashi, 1981). The following calibrated parameters were used in the model: (1) flow accumulation and slope relationship - extreme event threshold, (2) Holmgren's exponent -4, (3) slope angle algorithm of the energy loss function -11° , and (4) velocity threshold: 15 m s^{-1} (Horton et al., 2011). 10

Figure 14a depicts debris flow mapping developed by field investigation, aerial pho-

for predicting flow paths on a regional scale. Thus, by using this module, it was possible to provide a quick, simple preliminary debris flow assessment. In order to obtain a qualitative comparison of the model presented, Flow-R (Horton



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for the same topographic input, without any data related to sediment availability using geological, lithological information or land use. In Fig. 14, the width of the flow paths calculated by the Multiple flow direction method is wider than that produced by the D-infinity method, due to the routing of flow to all the adjacent pixels of lower elevation.

⁵ The module in TRIGRS allows the user to control the width using a weighting factor proportional to the slope raised to an exponent. However, in the scheme to predict debris flow susceptibility, it is not important to consider the flow width.

7 Conclusions

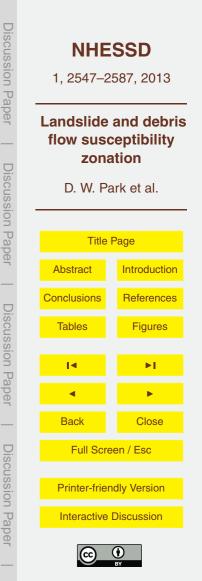
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This paper presented an approach to assess rainfall induced shallow landslides and debris flows in a mountainous region in Seoul, Korea. Model simulations resulted in reasonable estimates of the mountain hazards based on a deterministic approach.

One purpose of this paper was to suggest and verify a factor LR_{class} (landslide ratio of each predicted FS class) for a pilot study. The biggest strength of this index is that it can estimate model performance quantitatively by minimizing the overestimated area, even in landslide-debris flow regions where the area of landslide scars is unknown or ambiguous. According to the results, the percentage of LR_{class} of unstable area is 70 %, and well reflects the effect of transient rainfall.

Another purpose was to introduce a GIS based TRIGRS runoff module for predicting debris flow routes. By applying hydrological routing models, the results provide approximate information about debris flow routes. This means that the model, originally developed for landslide assessment, has also been proved applicable for susceptibility analysis of debris flow in catchments with limited data availability. It is noteworthy that the proposed approach is useful when there are only DEM and its derivative.

In summary, though TRIGRS output is dependent on the resolution of DEM and precision of the geotechnical parameters used, this paper provides a practical approach for mapping the susceptibility to landslide and debris flow of each pixel in an area. This kind of approach has advantages in that: (1) it considers "dynamic" (i.e., time varying)



analysis regarding the transient rainfall, (2) it provides workability with very fast computation, and (3) it gives useful results for preliminary assessments of hazards over large areas. Further research can be carried out to improve the TRIGRS model by coupling the landslide and debris flow runoff parameters.

⁵ Acknowledgements. This study was supported by the National Research Foundation of Korea under the Ministry of Education, Science, Technology (under grant No. 2012M3A2A1050974).

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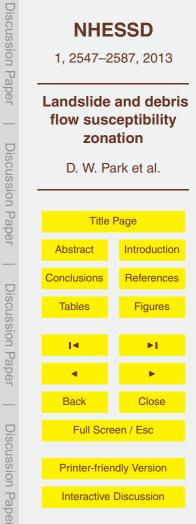
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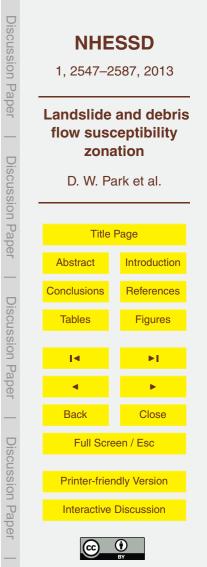
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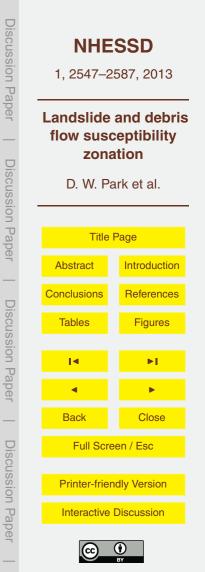
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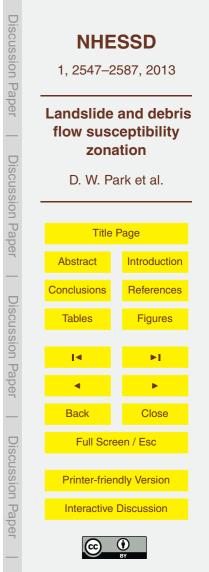
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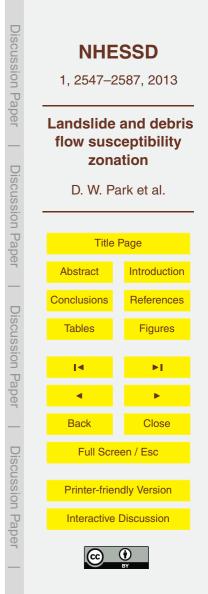
Table 1. Summary of flow routing models.

Flow routing model	Weighting factor in TRIGRS	Symbol	
D8 method (O'Callaghan and Mark, 1984)	$w_{ij} = 1$; cell on steepest path $w_{ij} = 0$; other downslope cells	 <i>i</i>: grid cell <i>j</i>: each neighboring downslope cell <i>n</i>: the number of neighboring grid cells <i>w_{ij}</i>: weighting factors <i>s_{ij}</i>: a function of the slope <i>δ_{ij}</i>: angle between the D8 flow direction and the steepest slope 	
Multiple flow direction method (Quinn, 1991)	$W_{ij} = s_{ij}^{w} / \sum_{j=1}^{j=n} s_{ij}^{w}$		
D-infinity method (Tarboton, 1997)	$\begin{split} w_{i1} &= \left(\frac{\pi}{4} - \delta_d\right) / \left(\frac{\pi}{4}\right) \\ , w_{i2} &= \delta_d / \left(\frac{\pi}{4}\right) \\ ; \text{ if two cells} \\ w_{i1} &= 1 \\ ; \text{ if only one cell} \end{split}$		

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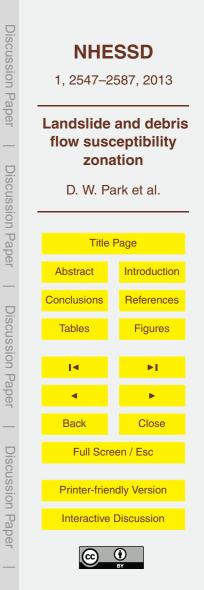
Table 2. Summary of values used in simulations.

Parameter (unit)	Value
Friction angle, ϕ (°)	29.63
Cohesion, <i>c</i> (kPa)	10.17
Total unit weight of soil, γ_{s} (KN/m ³)	18.38
Hydraulic conductivity of saturated, $K_{\rm S}$ (m s ⁻¹)	1.3 × 10 ⁻⁵
Saturated volumetric water content, θ_s	0.5
Residual volumetric water content, θ_r	0.18
Hydraulic diffusivity, D ₀ (m ² /s)	200 K _S
Steady infiltration rate, I_Z (m s ⁻¹)	0.01 K _S



Safety Factor Classes	Landslide site (a)	% of landslide site (c) = a/b	% of predicted area (d)	LR_{class} (e) = c/d	% of LR _{class} = e/f
FS ≤ 1.0	49	33.33	2.99	11.14	70.30
1.0 < FS ≤ 1.2	59	40.14	13.68	2.93	18.51
1.2 < FS ≤ 1.4	13	8.84	14.76	0.60	3.78
1.4 < FS ≤ 1.6	11	7.48	15.87	0.47	2.97
1.6 < FS ≤ 1.8	6	4.08	10.77	0.38	2.39
1.8 < FS ≤ 2.0	3	2.04	10.53	0.19	1.22
2.0 < FS	6	4.08	31.39	0.13	0.82
Sum	147 (b)	100.00	100.00	15.85 (f)	100.00

 Table 3. Summary of TRIGRS results in landslide simulations.



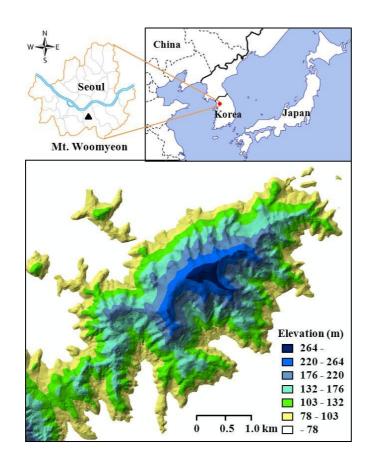
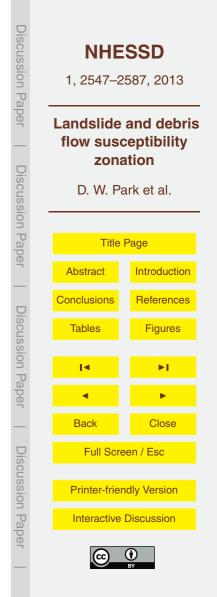


Fig. 1. Location map of the Mt. Woomyeon region in Korea.



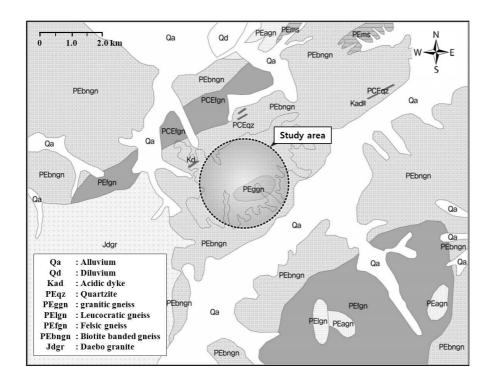
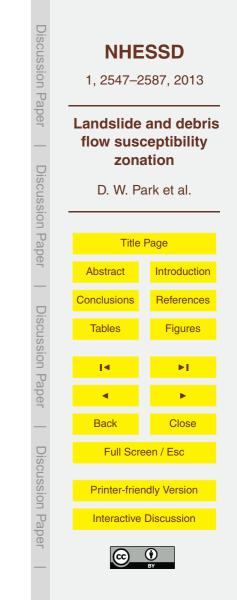
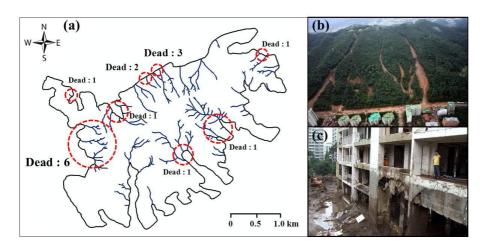
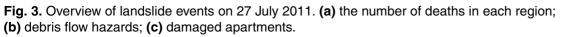


Fig. 2. Geological map of the study area.







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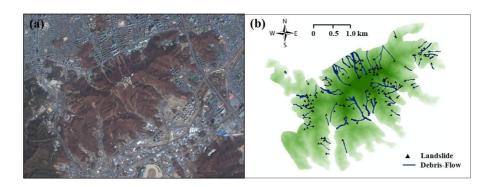


Fig. 4. Landslides and debris flows in the study area (a) landslide scars and debris flow routes; (b) landslide and debris flow mapping.

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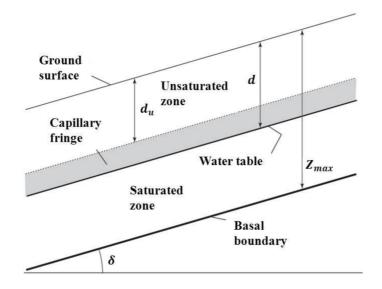
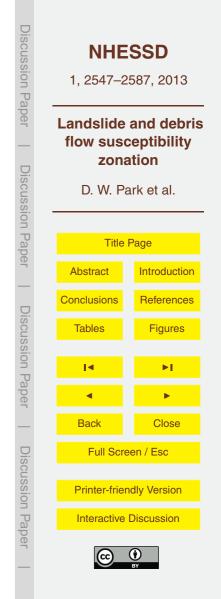


Fig. 5. Conceptual diagram of the TRIGRS model.



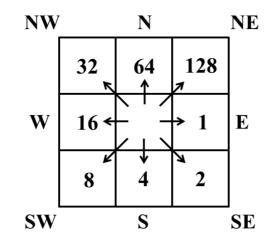


Fig. 6. D8 ESRI direction code overview.

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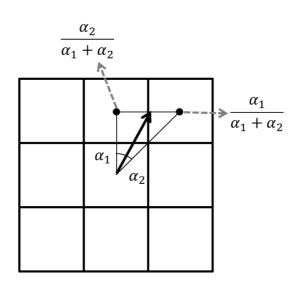


Fig. 7. Concept diagram of D-infinity method.

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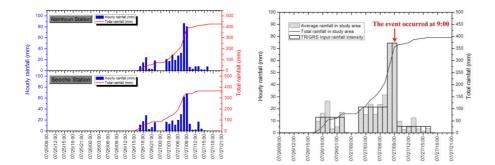
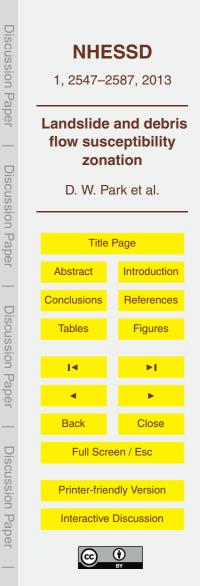


Fig. 8. Hourly and total rainfall distribution on 25–27 July 2011. (a) Rainfall at Namhyun and Seocho stations; (b) TRIGRS input rainfall intensity.



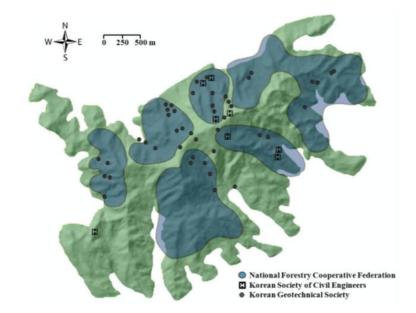


Fig. 9. Locations of investigation boreholes and sampling positions.

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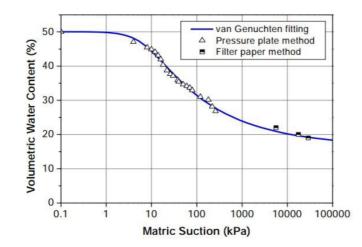
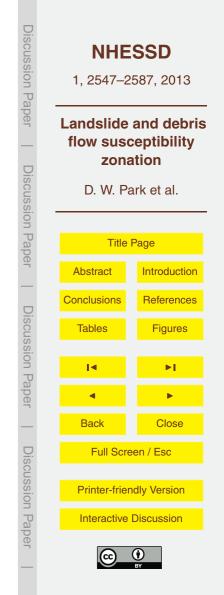


Fig. 10. Soil water characteristic curve in study area.



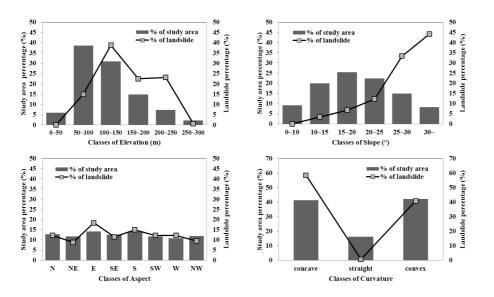
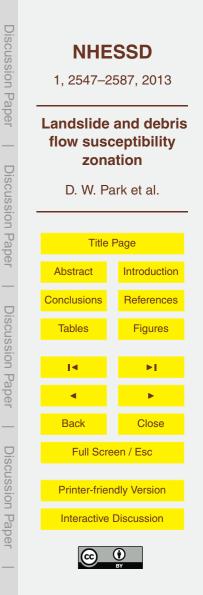
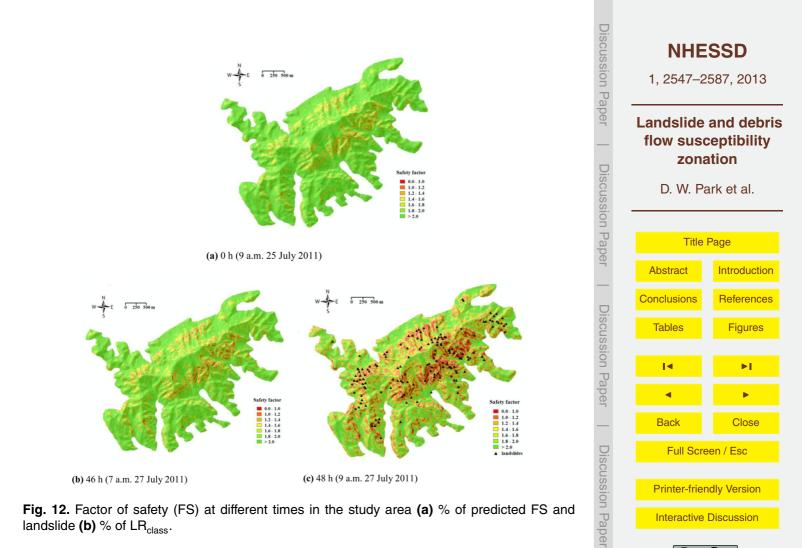


Fig. 11. Classes of topographic parameters (elevation, slope angle, aspect and curvature). (a) 0 h (9 a.m., 25 July 2011); (b) 46 h (7 a.m., 27 July 2011); (c) 48 h (9 a.m., 27 July 2011).





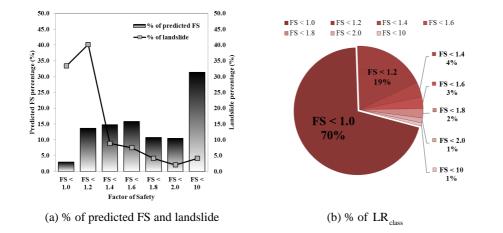
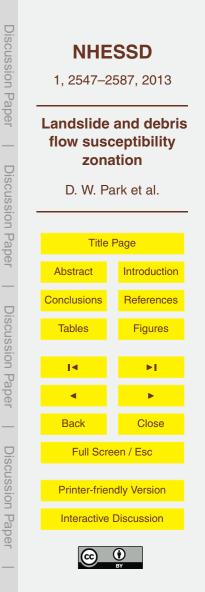


Fig. 13. Results obtained by TRIGRS in each class.



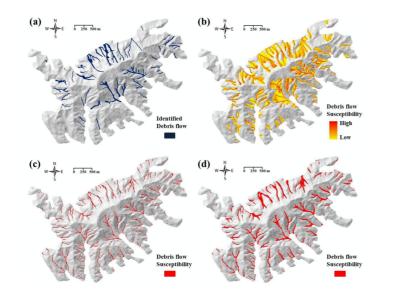


Fig. 14. Debris flow mapping and simulation results **(a)** Debris flow mapping in the study area; **(b)** Flow-R simulation result; **(c)** D-infinity method; **(d)** Multiple flow direction method.

