

Design of a geodetic database and associated tools for monitoring rock-slope movements: the example of the top of Randa rockfall scar

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Abstract. The need for monitoring slope movements increases with the increasing need for new areas to inhabit and new land management requirements. Rock-slope monitoring implies the use of a database, but also the use of other tools to facilitate the analysis of movements. The experience and the philosophy of monitoring the top of the Randa rockfall scar which is sliding down into the valley near Randa village in Switzerland are presented. The database includes data correction tools, display facilities and information about benchmarks. Tools for analysing the movement acceleration and spatial changes and forecasting movement are also presented. Using the database and its tools it was possible to discriminate errors from critical slope movement. This demonstrates the efficiency of these tools in monitoring the Randa scar.

1 Introduction

Increasing population density in inhabited areas multiplies the number of sites affected by slope movements (Swiss Association of Road and Transportation Experts, 2000). For instance, according to a preliminary inventory compiled by the Office Fédéral de l'Economie des Eaux (OFEE), about 6 to 8% of the Swiss territory is actually affected by landslides (OFEE, 1997). A part of the management strategy for slope instability risk mitigation includes monitoring over sites likely to induce damage in inhabited zones and/or to facilities (Bromhead et al., 1988; Rochet, 1992; Keaton and DeGraff, 1996). Slope movement monitoring, which is often performed over long periods of time, generates large volumes of data, especially following the use of automatic measurement devices (e.g. extensometer, GPS, radar, weather station, etc.). Relevant information ranges from survey data to sensor specifications including monitoring network topology, pictures, map documents, etc. The storage and the maintenance

of all these data lead to the need for efficient, structured and flexible data management systems.

Many problems arise in slope movement monitoring. These problems may have various sources and occur at different levels of the monitoring process (Rochet, 1992). The use of a well-designed database and the related data analysis tools can solve a great part of the monitoring issues and the infrastructure maintenance problems.

Missing or erroneous data are inherent to all data acquisition processes. Thus a database designed for slope instability monitoring must include functions that help to maintain a coherent data set by allowing corrections and avoiding errors. Furthermore, tools linked to a database must be efficient to detect the changes in the general trends of the relevant parameter alert threshold values, in order to give an alarm to the concerned specialists.

On one hand, the analysis of slope movements must indicate if a catastrophe is imminent or can be forecasted. For example, if an acceleration, which can lead to failure, is detected, an alarm process must be triggered. On the other hand, if the movements suffer simple changes without consequences, the monitoring tools may have to be updated, based on an updated analysis of the relevant parameters, or the monitoring system can become irrelevant. Thus, a database must be designed with respect to a monitoring system adapted to each case and based on relevant alarm parameters or analyses. The data must be acquired in real-time or with a delay depending on the type of data and reaction time of instabilities.

Originally the presented database was developed for the monitoring of the scar of the Randa rockfall. In April and May 1991, about 30 million m³ fell down in the valley near the Randa village situated 10 km north of Zermatt in Switzerland. The proposed approach is based on the past 12 years of experience gained from surveying the movements affecting the top of the Randa rock fall scar. As a result, this paper deals with databases that are needed when events present a time delay between the time at which the threshold warning parameter value is reached and the expected catastrophic

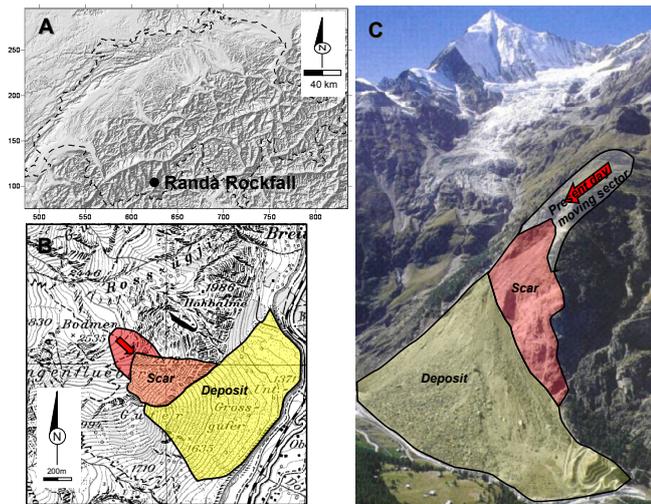


Fig. 1. (A) Location of Randa rockfall (DEM RIMINI Federal office of Topography (DS033032)). (B) Situation of the rockfall and present active area (arrow). (C) Oblique view to the SW of the rockfall which took place on 18 April 1991 (22 million m³) and 9 May 1991 (7 million m³).

event. The present example possesses many similarities with published monitoring systems. Electronic Distance Meter (EDM) is often used as well as triangulation (Archbold et al., 1988; Bromhead et al., 1988; Pingue et al., 1998; Rochet, 1992; Keaton and DeGraff, 1996; Young et al., 2000). At present GPS and radar of synthetic aperture radar (InSAR) are more and more used (Dzurisin, 2003). But a few papers focus directly on databases dedicated to slope monitoring.

The database structure and the associated tools implemented for the handling and analyses of Randa monitoring data are discussed below. New monitoring tools and data correction procedure are also presented. The monitoring system also provides pictures and graphs.

2 The 1991 Randa rockfall and associated movements

2.1 Geological and geomorphological settings

On 18 April 1991, 22 million m³ of rock fell from the steepest cliff of the Matter valley near the village of Randa (10 km north of Zermatt, Switzerland) (Schindler et al., 1993; Sartori et al., 2003). On 9 May 1991 a retrogressive rockslide of about 7 million m³ fell down from the upper part of the 18 April scar (Fig. 1). Fortunately no fatalities were reported except for forty head of cattle. The main road, the railway and thirty-one chalets were destroyed.

The Randa steep cliff is composed of a competent orthogneiss (Bearth, 1964) at its base, in which most of the first event took place. The orthogneiss is overlain by a less competent paragneisses shaping gentler slopes than in the orthogneisses. Sartori et al. (2003) identified several pre-existing field conditions that affected the stability of the area: (1) unfavourable structural settings (large scale discontinu-

ities); (2) the steepness of the cliff; (3) the massive lithology (mainly orthogneiss), (4) the location on a topographic ridge outcropping at the confluence of a glacial cirque and the main valley; and (5) the existence of previous instability events.

The top of the scar, located within the paragneisses, is now affected by slow ongoing deformation, with a maximum velocity of 1–2 cm per year. The moving rock mass represents approximately 2.5 million m³ (Eberhardt et al., 2001). This moving rock mass underlines the retrogressive erosion process affecting the slope. Based on geodetic data, the calculated displacement vectors are in accordance with a sliding mechanism along preexisting structures which acted during the 9 May 1991 rockslide.

2.2 Slope movement monitoring

After the first event in April 1991, a monitoring network with alarm devices was installed. It consisted of three components: (1) a geodetic network, (2) an extensometer network, (3) meteorological stations (Jaboyedoff, 1995; Ornstein et al., 2001). After the events of April and May 1991, geodetic points were installed above and at the top of the rock fall scar. During the last 12 years, between 15 and 25 points have been monitored at least 4 times a year by EDM (Distomat Wild DI1001; Leica, 1990) (Fig. 2). The EDM measurements are performed from the valley bottom, at 1400 m a.s.l., up to 2500 m a.s.l., at distance ranging from 1 to 2 km. This network was completed by the installation of 25 extensometers, of which twenty are measured manually once a year and five are automatically recorded with time step ranging from minutes to days. Triangulations have been performed during the last 5 years on a limited set of points. This method gives additional information on the direction of the displacement vectors, which are partially estimated through EDM data.

3 Methodology

3.1 Questions to be answered

The goal of a monitoring system is to monitor relevant parameters and to give warnings for catastrophic events. The parameters must be descriptors of the evolution of slope stability and must give the necessary information on precursory movements or others warning parameters such as rainfall (Crosta, 1998), acceleration leading to rupture (Azimi et al., 1988; Fukuzono, 1990) or high constant velocities. Based on velocity analysis of the Ruinon landslide (Italy), Crosta and Agliardi (2003) showed that it is possible to predict a velocity threshold for warning purposes. Such interpretations are only possible if movements have undergone acceleration.

Three dimensional movement data (x , y , z) are, in theory, required for a fine understanding of the instability; in this case GPS or a standard geodetic network can be used (Evrard et al., 1990a). But often 1-D data are sufficient to give warnings; in this case, extensometer data or EDM can be used (Guerricchio et al., 1988; Gili et al., 2000).

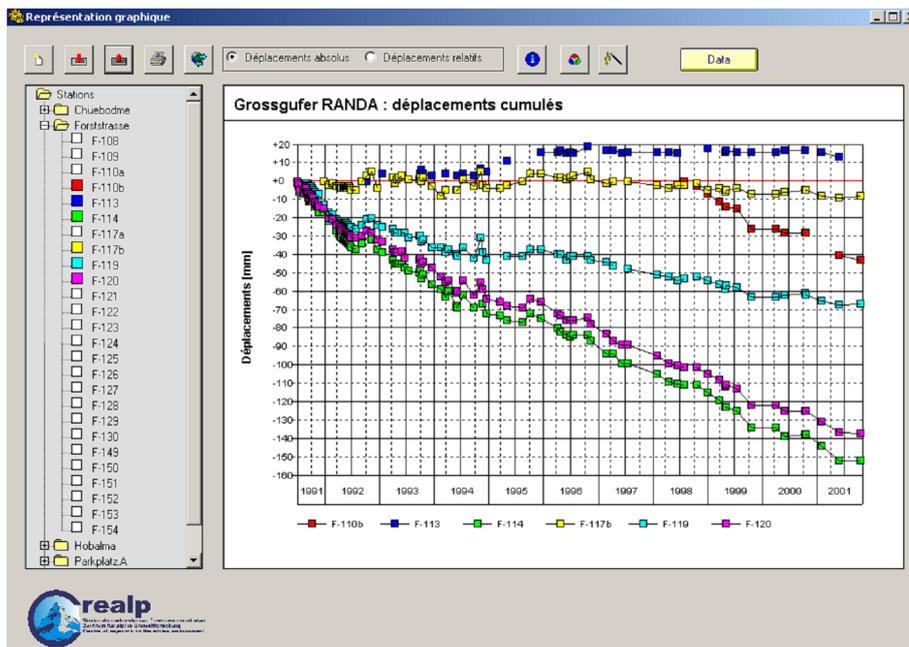


Fig. 2. Window of data displayed by the RandaDB. On the left, tree structure of the data sets (like in Microsoft Windows explorer) with names of benchmarks. On the right, displacement graph of the points measured from the same point by EDM. The RandaDB is developed under Windev (2000). The negative values indicate the distance covered by the benchmark down to the valley.

In the surroundings of the Randa rockfall scar, the slope movement monitoring, mainly by EDM, has demonstrated that the data storage must be flexible because of the changes induced by: 1) the ageing or degradation of material that may be produced by slope movement, 2) human errors, or 3) changes in the positioning of reference points. The climatic parameters (humidity, snow, clouds etc.) may also disturb optical measurement devices. The Randa rockfall area monitored by EDM have led to problems like: distance error caused by temperature gradient or variation of air optical properties with humidity for long sighting, reflector changes, changes in station movement, or new location points.

The geographical situation of the slope instability has a great influence on the selection of monitoring methods and devices. The case of Randa scar, which is mainly located above 1800 m a.s.l., is a good example because of the thick snow pack. As a consequence most of monitoring work has to be executed from the valley, which had led to choose the EDM measurement method. The choice of suitable monitoring parameters depends on the knowledge of the instability mechanisms, which also helps to place monitoring devices in appropriate places. The interpretation of measurements is greatly refined if the mechanisms are well understood. Because monitoring is often initiated during critical conditions, the understanding of the instability mechanism is refined with time. As a consequence, the monitoring system is updated and instability knowledge increases. New interpretation of mechanisms can lead to a new design of the monitoring devices, therefore the monitoring methods must be modified to be consistent with the instability mechanism (Rochet, 1992). The 3-D geodetic network implemented within and above the Randa scar has been designed according to the interpretation of EDS data.

If the monitoring programme lasts for a long period of time, measurement points are often damaged or abandoned and then replaced with new points. These breaks in measurement continuity have to be resolved. To respect the data integrity three tasks must be performed:

1. The data stored in the database have to remain accessible with time, in order to be used in new monitoring software or in new monitoring criteria (e.g. change in alarm parameters).
2. For 1-D data, the acquisition of total movement data must start on the same date, in order to compare data and to map movements.
3. Analysis of data must be performed rapidly, to detect any changes in the behaviour of instability.

In the light of the first years of the Randa scar monitoring, an appropriate data management must be based on software comprising a database and associated tools dedicated to data correction and data analysis. Such a program must facilitate the data maintenance and reduce the delay between data acquisition and data analysis. Data without any correction must first be documented. The method used to maintain the data sets and history must be recorded within the database. Also, measurement point descriptions and specifications, including pictures, must be available for an effective monitoring.

3.2 Database design

Part of the database design was directly the result of the Randa scar monitoring; the structures and the tools were designed to be applied to other types of landslides. The fundamental associated monitoring tools are time-displacement

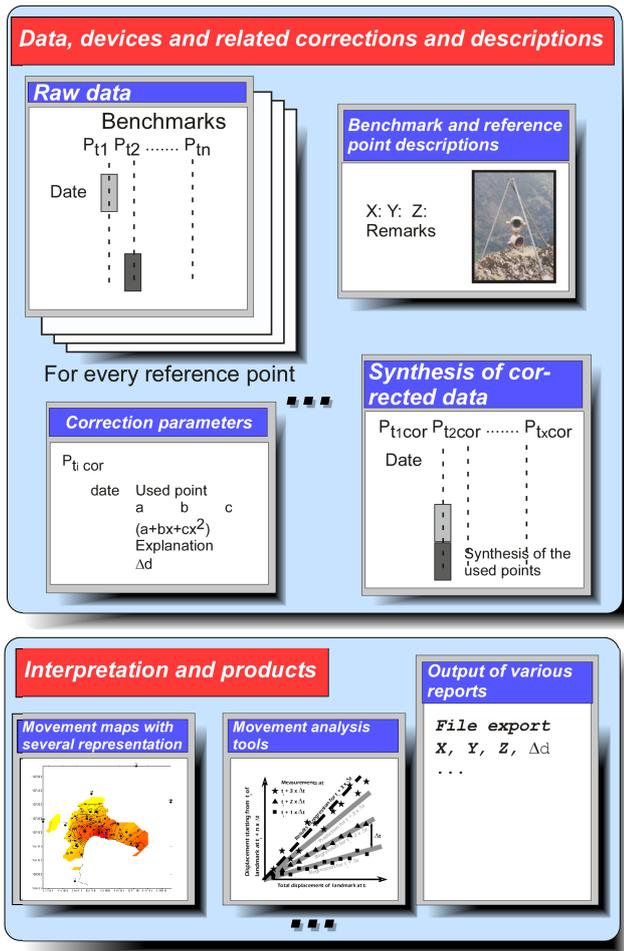


Fig. 3. Simplified structure of the tables and tools implemented in the RandaDB for slope movements monitoring. The data and devices and interpretation products contain more informative tables than shown. The procedure consists of first archiving original data, correction parameters and all events affecting the monitoring, and exporting these and producing various monitoring documents.

plots, time-precipitation plots, and maps of the point trajectories or displacement values projected on a plane such as the $x-y$ plane (Wieczorek et al., 1990). These allow a quick view of slope evolution. It can also be complemented with other documents such as pictures, reports, extra measurements, etc. All benchmarks used for the measurement can be documented by text, history, pictures and working dates. A point distance measurement is linked to a fixed position of electronic distancemeter from which measurements are performed.

The measurement campaigns are stored by date in different forms (x , y , z or distance d); benchmarks are described and corrections are recorded (Fig. 3). Both measurement campaigns and benchmarks can be easily found within data tree structures (Figs. 2 and 4). Movements for one or more points can be represented on time-displacement graphs (Fig. 2). The data can be represented on various maps using different kinds of symbols that represent the displace-

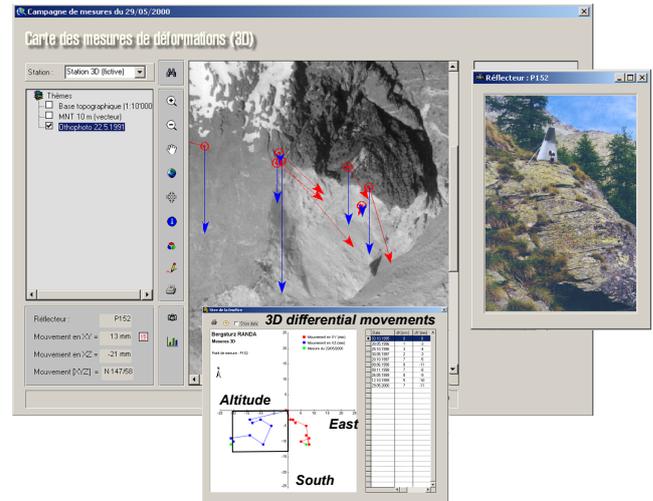


Fig. 4. Illustration of the output possibility of the RandaDB using Mapobjects library (MapObjects, 2000). The orthophoto is displayed with arrows representing the horizontal displacement and the vertical displacement of each point. For 1-D data, different sized circles represent the displacement values. Benchmarks are described by pictures and texts. The displacements can also be represented in a projection of the 3-D space.

ments. For instance, arrows depict the 3-D movement paths of benchmarks. Information and documents related to points can be viewed by clicking on each of them (Fig. 4).

Arrows or variable sized circles that are superimposed on various backgrounds (e.g. digital elevation model or geo-referenced map, air photos) can represent 1-D, 2-D as well as 3-D displacements (Fig 4).

It is then possible to plot benchmark displacements versus another benchmark displacements. Data for a given date can be exported as x , y , z or apparent displacement (d) to be input in an interpolating and mapping program.

4 Results

Numerous tools and procedures were developed in using the above-mentioned principles for the monitoring of Randa movements. The present section presents the principal results of this approach applied to the Randa case study.

The database software for the Randa rockslide (RandaDB) was developed at the CREALP (Centre de recherche sur l'environnement alpin) using the Windev environment (Windev, 2000). Windev provides an effective database management and programming languages for data treatment. The Mapobjects library (MapObjects, 2000) is implemented for map representation.

4.1 Displacements

The EDM displacements were calculated using an assumed stable point near the slope movement area. Because of air

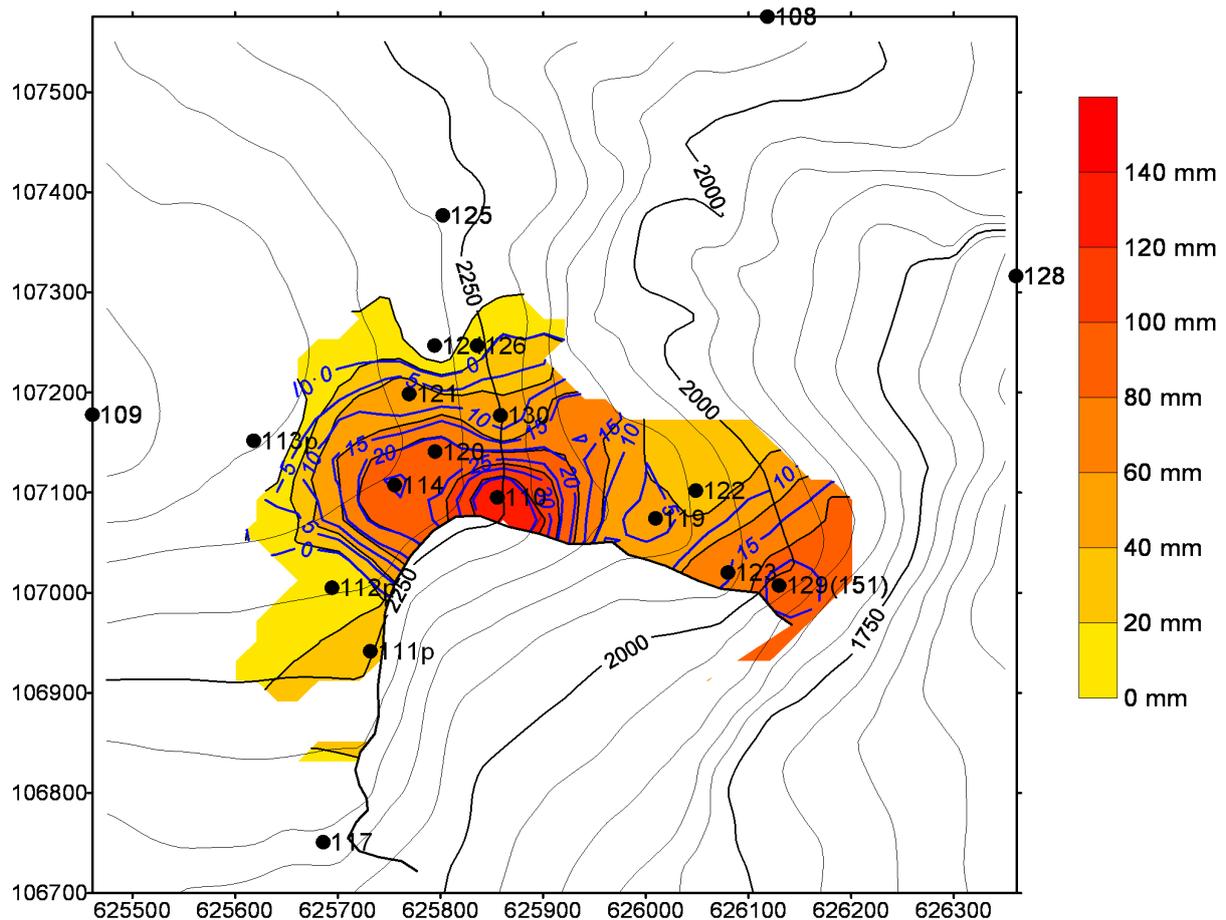


Fig. 5. Example of the tool implemented in the RandaDB using Surfer (1999) allowing the interpretation of movement homogeneity by using map of displacements during two periods of time: 1 July 1991 to 21 February 1997 and 1 July 1991 to 30 May 1994. The coloured background represents the displacement map from 1 July 1991 to 21 February 1997, and the isolines are the difference of movements from 1 July 1991 to 30 May 1994. The mapping of the movement values of benchmarks down to the valley are represented in absolute values (positive). Points indicate the label of the benchmarks. Note that if velocities of displacement were constant, the ratio of displacements measured from 1 July 1991 to 21 February 1997 and from 1 July 1991 to 30 May 1994 would be in a ratio of 3. Because of the decelerating movement up to approximately 1995, the ratio is a little over 2. However the spatial distributions of displacements are identical, indicating that no relative acceleration has occurred.

stratification and hot winds, the atmospheric correction is difficult to be estimated.

During 1992–1993 the highest recorded movement velocity (EDM from valley) was about 2 mm/year. Since the middle of 1994 the velocity has stabilized at around 1 mm/year (Fig. 2). As indicated in the EDM displacement map (Fig. 5), movements are greater around the scar and increase locally on top of hills. The 3-D data show that the overall movement direction is down towards the southeast (Fig. 4).

4.2 Data maintenance

For an optimal analysis, data sets must be corrected for systematic measurement errors, like the difference between successive data caused by accidental measurement point displacement or benchmark replacement, etc. It is possible to correct data for a time range by using a linear or a polynomial interpolator.

In the case of an unexplained large discrepancy between two successive measurements, the displacement error can be estimated by using linear regression in a time-displacement graph depicting the previous values of displacements. The corrected value is obtained using the regression to estimate the displacement value at the time of the erroneous data (Fig. 6). When a new point is introduced, in order to have an effective monitoring, the initial displacement must be estimated with respect to the time at the beginning of the displacement recording, t_0 (Fig. 7). This can be achieved, by correlating its movement with other points, preferably nearby and only after several point measurements have been attained. The regression line calculated between the pre-existing point (123F) displacements and the new one (129F) makes it possible to estimate the value that must be added to the new point (129F) displacement. Thus the first value of displacement at t_0 for (129F) is equal to the regression line

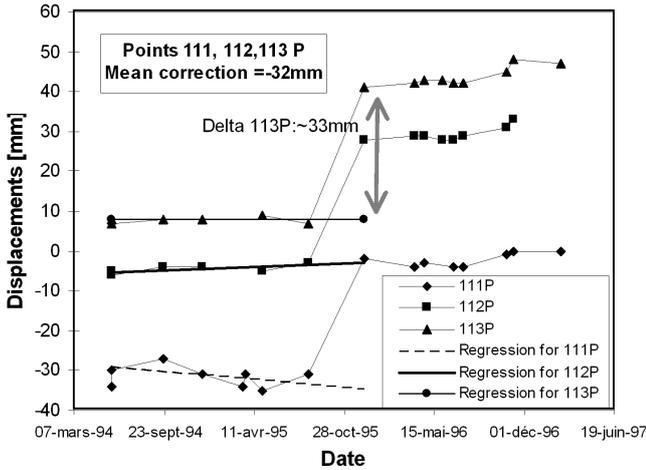


Fig. 6. Example of a correction procedure when a fixed point (P) of measurement moved by accident. All the benchmark (111P, 112P, 113P) measures are affected, thus the discrepancy with the expected value can be estimated. In this case the data were reduced by 32 mm from October 1995.

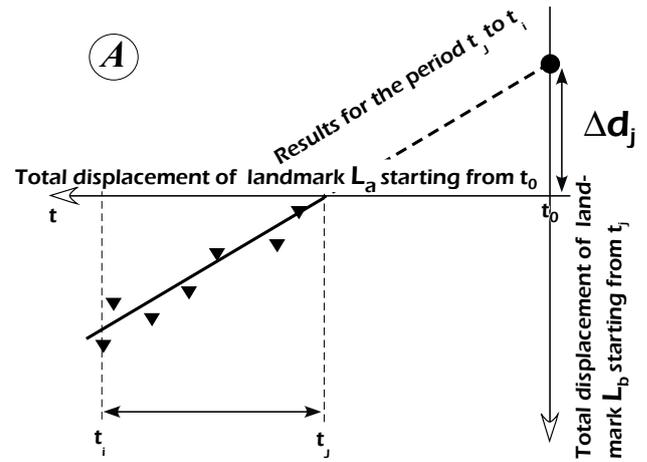
intercept. This procedure is appropriate only if movements are proportional. It is better to use regression line type such as Reduced Major Axis or principal axis (Davis, 1986), in order to take into account uncertainty on both axes.

The maintenance must lead to data sets which are corrected as much as possible for all errors and having the same t_0 . Note that GPS is only partially affected by the problems above because GPS measurements give absolute positions, thus limiting the possibility of errors. However, positioning of the GPS device can be also affected by human ones or caused by a malfunctioning of the device.

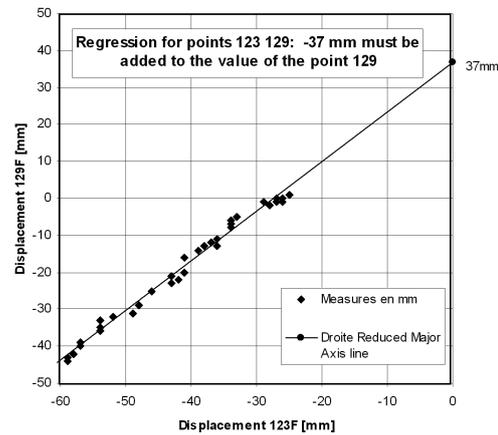
All data acquisition is affected by error, thus a proper data maintenance results in identifying and managing punctual errors or systematic errors. The first is quite easy to manage, but the second needs solution as proposed above.

4.3 Variation and homogeneity of velocities

A plot of the total displacements at a time t_i , starting from t_0 , of all available successive benchmarks measurement displacement sets versus those for $t_{i+n} - t_i$, gives changes of the movements. Assuming benchmarks velocities are all in a constant ratio, the plot displays sets of aligned points passing through the origin and where slope increases with time (Fig. 8A). If some points are not aligned, it means that they have undergone acceleration compared to the other points. This representation allows verifying if the movements for different points are proportional; making velocity variations quickly detectable, or defining a signature of the entire movement for monitoring purposes. Curved lines or dispersion of points displacements are indicative of acceleration of the displacement in a certain area that can be considered as a first level of alarm if the points were aligned in the previous survey. If all the points accelerate together, this tool is irrelevant considering only a linear fitting.



(A)



(B)

Fig. 7. Example of a correction procedure for new benchmark used for data maintenance in the RandaDB. (A) Synthetic example: The measurements of displacement of the benchmark L_b begins at time t_j , its total displacement from time t_0 is estimated using a neighbouring benchmark L_a that has been recorded since t_0 . Δd_j is the displacement to be added to the new benchmark displacements to obtain the same total displacement since t_0 . (B) Example for a new point 129F using the 123F as a reference.

Figure 8B displays an example from the Randa database, which indicates that movements are homogenous for the measurements campaigns before the 29 July 1998. All the measurement of each campaign are aligned, no relative movements are observed. It means that the rock mass is slowly moving down and decelerating as indicated on Figs. 2 and 5 This deceleration state can be used as the reference state of the instability, implying that any detected relative movements or acceleration can be considered as a warning.

4.4 Displacements maps

The comparison of maps established for different dates gives a basic estimation of overall slope movement. An improvement on this method is to compare the differences of

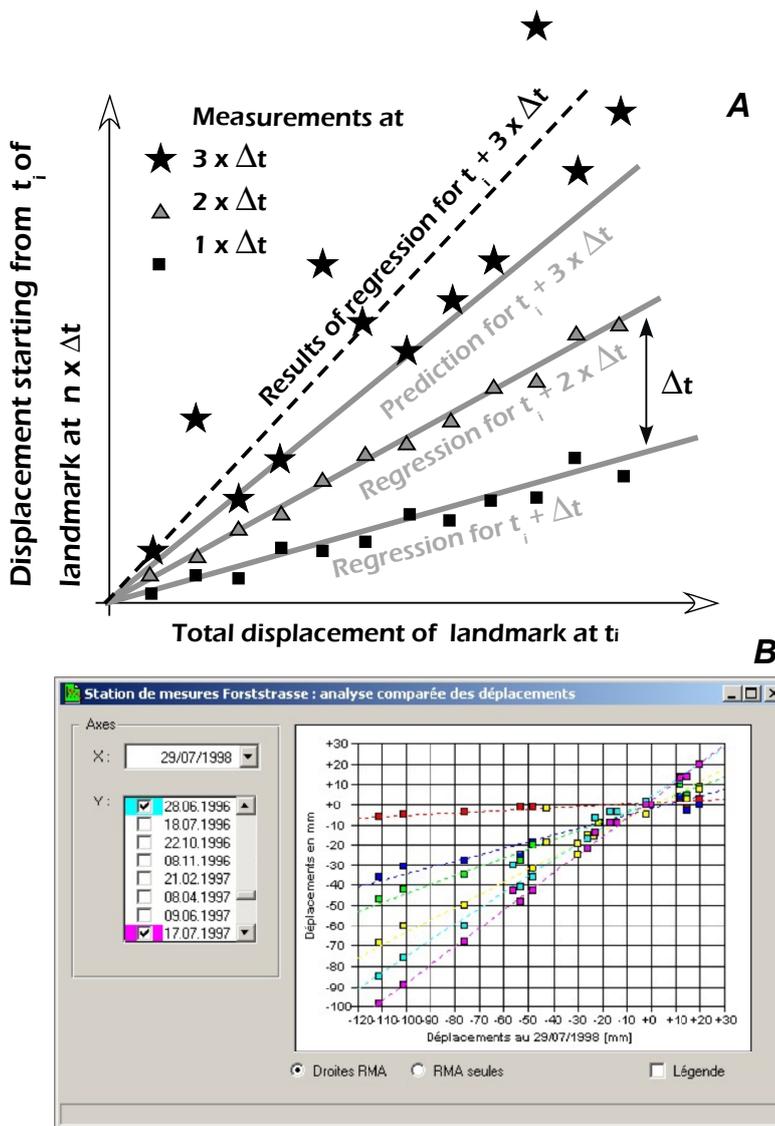


Fig. 8. (A) Synthetic example of the tool implemented in the RandaDB analysing the interpretation the homogeneity of movements. The velocities are assumed to be constant for the two first displacement measurements made after $1 \times \Delta t$ and $2 \times \Delta t$ starting from t_i , indicating a weak dispersion. After $3 \times \Delta t$ the velocities are more dispersed and some points suffered acceleration, indicating a change in behaviour. (B) Example from the RandaDB (note that that movements are negative); displacement measurement sets prior to or dated 17 July 1997 are compared to a more recent (2 July 1998) displacement measurement sets. The graph indicates that movements have been proportional during the previous years.

displacements between different periods (Figs. 9 and 5). It allows a quick visualisation of the spatial changes in benchmarks velocities, which can be an indicator of instability mechanism change and/or in precursory movements of a movement crisis. By mapping difference in velocity, the zone of increasing velocity can be detected by an increase of area of a given value.

Any map produced must be inserted in the database as archived documents in order to be used as reference. This always implies using the same interpolator, the same data point position, and a database that can maintain and correct a constant set of data points.

4.5 Movements forecasting

Benchmarks that are representative of the overall slope movements can be studied in a more detailed way (Fig. 10). Simple and effective monitoring can be achieved through a forward-looking curve for point displacement. These curves

fit to the benchmark displacement data, which include different types of seasonal variations. For instance, the Randa data set has seasonal (or daily) variations resulting from the change of air temperature due to long distance geodesic infrared measurements (Figs. 2 and 10). In Fig. 10, the forecast curve (time-displacement) fits to displacements using a power law with an additional seasonal variation expressed by a sine. A discrepancy between forecast values and actual measurements might lead to a first level of alarm. If the difference cannot be explained by methodological problems (Fig. 10), the discrepancy can be considered as a second level alarm. A detailed analysis of the movement changes with respect to the assumed mechanism must be completed in order to give the next level of alarm. Note that systematic variations can produce imperceptible drifts to some measurements. These appear only after several measurement campaigns. Thus, the curve must be periodically readjusted overall.

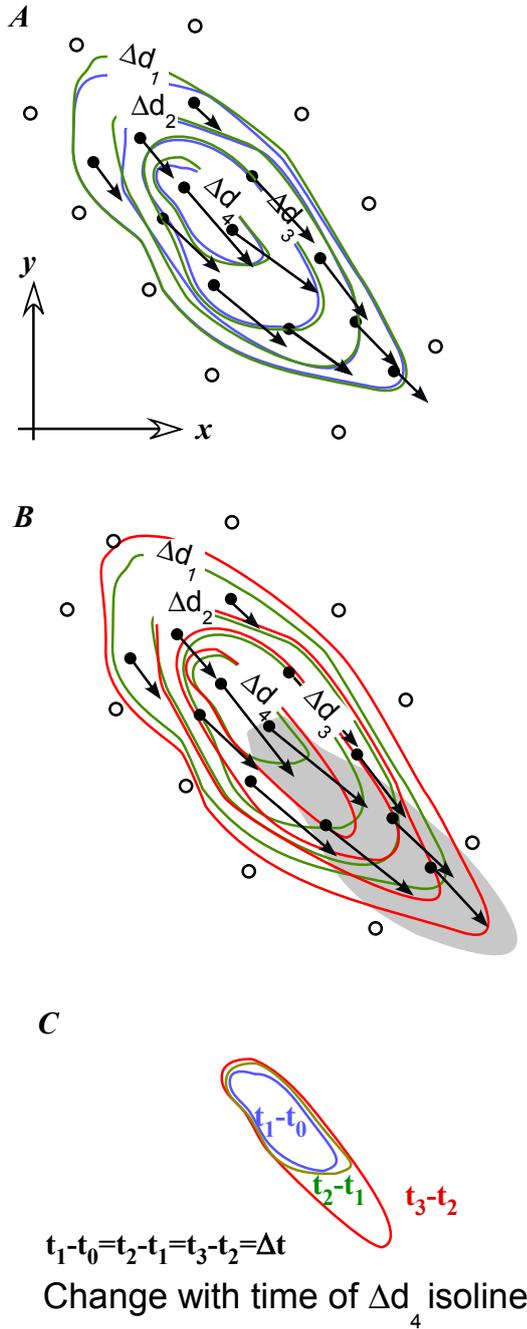


Fig. 9. Schematic illustration of monitoring by mapping movements. The arrows show the velocities (lengths are proportional to the displacements, and direction indicate movement direction). The three isolines sets are displayed for successive identical periods of time Δt . The displacements increments are displayed by chronological order respectively blue, green and red. **(A)** Two mapping with no variation in the distribution of velocities, i.e. isolines for $t_1 - t_0$ and $t_2 - t_1$ are similar. **(B)** Illustration of a warning situation: Velocity increase for some points and dispersion of velocity (the red isolines correspond to $t_3 - t_2$ measurement period and $\Delta d_1 < \Delta d_2 < \Delta d_3 < \Delta d_4$) compared to the previous displacement data, indicating a change in mechanism, the greyed area indicates the zones where the movements have accelerated. **(C)** Detail of (B) showing a rapid change of shape of isoline which highlights differential movements and movement accelerations.

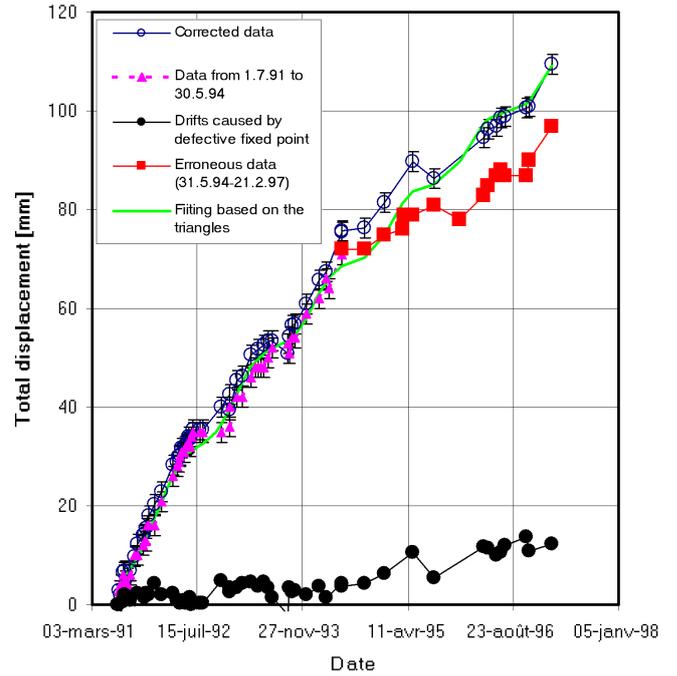


Fig. 10. Example of movement forecasting that leads to identification of defective devices (a fixed point was moving). The forecast takes into account a seasonal distance variation and a slow decrease of the movement using the relationship for the considered point $\Delta d = 0.438 \times t^{0.72} + 2 \sin(2\pi t/365)$. Note the slow drift of the erroneous data.

5 Discussion

Slope monitoring over a long period of time implies increasing number of erroneous data, and benchmark defects lead predictably to very confused data sets. As a consequence, distinction between error and changes in the slope behaviour might be missed. Thus a database and associated tools are mandatory.

During the monitoring, warning parameters must be chosen and defined. Threshold values or rules must be established for variables, such as benchmark displacements, benchmark displacement velocities, rainfalls or others. These values must be updated over time. In the case of Randa, the warning parameters are the mean velocity of each benchmark deduced from the year preceding the measurement.

If a subsequent analysis demonstrates erroneous corrections, then original data must be accessible. If possible, all movements must refer to an initial time t_0 .

Monitoring is often a routine job where simple procedures are repeated. Alarm variables can be as simple as high displacement velocities, but this is not sufficient for a true and effective monitoring system. The development of an instability mechanism model is a key point for a middle term catastrophe forecast. As demonstrated by the monitoring of the Randa scar, the tools presented above are effective for such a purpose. These tools can help to extract information on movement changes. Forecasting curves proved to be

especially useful. For instance, at Randa, the forecast for one of the fastest point was found to be larger than the measured value (Fig. 10). On the other hand, the map of slope displacement or apparent slope displacement indicates no change in the overall slope movement (Fig. 2). Because the distance measurements are corrected with respect to a fixed point, a detailed analysis of the measured value and the map of movements revealed that the fixed point was also moving. As a consequence, all the data of the Randa scar monitoring system were corrected assuming that the mean of three other points were considered as fixed.

In the case of large landslide movements, it is necessary to frequently update the monitoring devices and analysis. The geometry of the instability varies with time and the meaning of measurements also varies due to the large movements of benchmarks. Thus an expert must continuously update the model by constant reinterpretation. For long term implementation of monitoring systems, the expected geometry and instability mechanism must be assessed. Such a forecast can be based on field observations or observed displacements after a few months or a few years. In the case of Randa, the successive displacement maps clearly show that the movements are homothetic in time (Fig. 5), which means that all velocities are changing proportionally to each other. As a consequence the map of movements displays always the same shape. In other words, if benchmark velocities are constant the displacement map for successive identical time interval leads to similar maps (Fig. 9).

The possible mechanisms affecting the slope can be deduced from the movement maps. The displacement isolines traced on the topography define in three dimensions nearly planar surfaces within the upper part of the Randa scar. This can be explained by assuming a simple shear band affecting the slope in the direction $135^{\circ}/40^{\circ}$. This assumption must be refined with time using the 3-D data that are still too uncertain. The more distant a point is from the sliding base, the faster it moves. In addition, fractures perpendicular to the sliding direction are observed above the Randa scar in the sloping ledges. These open fractures confirm the above interpretation. Thus the higher a benchmark is above the basal surface and, simultaneously more down the slope it is located; the larger is its displacement (Fig. 11).

Understanding of any steady state movement, acceleration or change in the overall instability movements is of primary importance. For instance, a rockslide in a steep cliff moving 10 cm/day will most likely end up to a rockfall within a few days or months, because in 3 months the movement will reach an equivalent of roughly 10 m of displacement. For instance, the second Randa rockslide occurred after only 40 cm of displacement at one of the monitored tension cracks.

Triangulation, which uses angle, and trilateration, which uses distance to estimate point position, are tools that can be implemented in a monitoring plan. On-line monitoring is now a more and more common tool (Evrard et al., 1990b), but it is based on certain limits defined by a preliminary modelling. Meteorological and piezometric data can be introduced as monitoring variables.

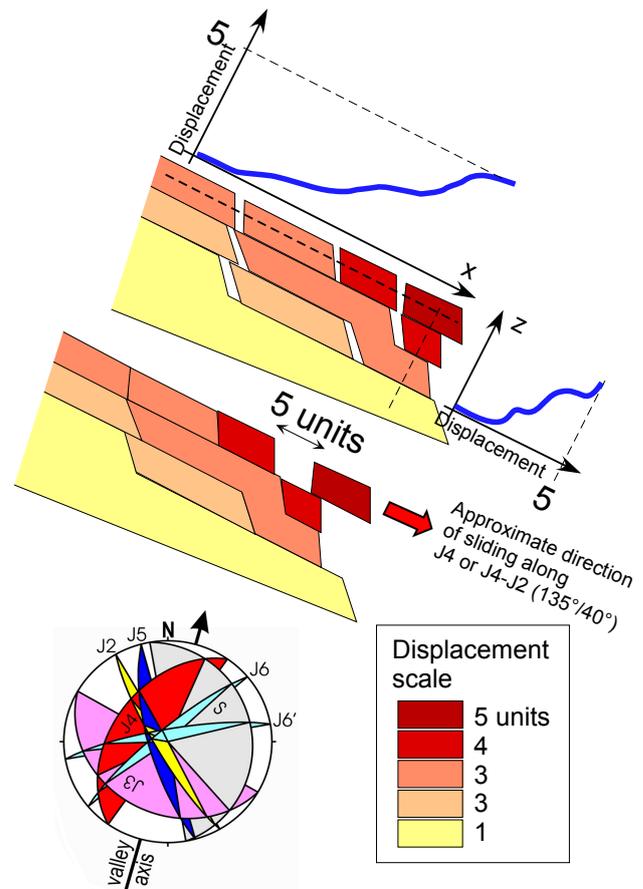


Fig. 11. Assumed mechanisms and associated displacements above and in the upper part of the Randa scar. The direction measured by 3-D devices and 1-D interpretation is approximately $135^{\circ}/40^{\circ}$, which may correspond to the direction J4 ($125^{\circ}/45^{\circ}$) or by the wedge formed by J4/J2 ($135^{\circ}/43^{\circ}$) (structure data from Sartori et al., 2003).

6 Conclusion

The slope monitoring principles presented above have the advantage of linking the database maintenance and simple monitoring tools needed for analysing movements. The main components of a monitoring system include data storage, data corrections, different ways to represent data and the forecast of warning parameters in space and time in order to estimate threshold limits.

The data analysis of the Randa scar, using the software RandaDB, has been greatly improved by the quick verification of data since this software requires a systematic approach in data treatment and analysis. Using the Randa monitoring system, it was shown that no significant accelerations were reported. Furthermore the development of the monitoring tools, which make it possible to distinguish between measurement errors and movement pattern changes, certainly helps in forecasting the next rockslide.

The development of this software based on the principle linking data management and monitoring tools is useful in the Alps areas because the monitoring of landslides

mostly follows the same principles that prevail the development of the RandaDB software. In other regions, volcanic and seismic dangers are often monitored using geodetic devices (Archbold et al., 1988; Pingue et al., 1998; Young et al., 2000). Such a monitoring system based on RandaDB will certainly find applications in those cases because the economic implications of volcanic catastrophes are enormous.

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