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Regional assessment of extreme sea levels and associated coastal flooding along the German Baltic Sea coast

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Abstract. Among the Baltic Sea littoral states, Germany is anticipated to endure considerable damage as a result of increased coastal flooding due to sea-level rise (SLR). Consequently, there is a growing demand for flood risk assessments, particularly at regional scales, which will improve the understanding of the impacts of SLR and assist adaptation planning.

Existing studies on coastal flooding along the German Baltic Sea coast either use state-of-the-art hydrodynamic models but cover only a small fraction of the study region or assess potential flood extents for the entire region but rely on global topographic data sources and apply the simplified bathtub approach. In addition, the validation of produced flood extents is often not provided.

Here we apply a fully validated hydrodynamic modelling framework covering the German Baltic Sea coast that includes the height of natural and anthropogenic coastal protection structures in the study region. Using this modelling framework, we extrapolate spatially explicit 200-year return water levels, which align with the design standard of state embankments in the region, and simulate associated coastal flooding. Specifically, we explore (1) how flood extents may change until 2100 if dike heights are not upgraded, by applying two high-end SLR scenarios (1 and 1.5 m); (2) hotspots of coastal flooding; and (3) the use of SAR imagery for validating the simulated flood extents.

Our results confirm that the German Baltic coast is exposed to coastal flooding, with flood extent varying between 217 and 1016 km² for the 200-year event and a 200-year

event with 1.5 m SLR, respectively. Most of the flooding occurs in the federal state of Mecklenburg-Western Pomerania, while extreme water levels are generally higher in Schleswig-Holstein. Our results emphasise the importance of current plans to update coastal protection schemes along the German Baltic Sea coast over the 21st century in order to prevent large-scale damage in the future.

1 Introduction

Sea-level rise (SLR) will increase the frequency and intensity of extreme water levels, thus increasing the risk of coastal flooding with potentially far-reaching socioeconomic impacts (Kirezci et al., 2020; Hinkel et al., 2014). In the European Union and the United Kingdom, 100 000 people are already exposed to floods every year, and the average annual flood damages are estimated to be EUR 1.4 billion. These numbers are likely to increase by the end of the century (Vousdoukas et al., 2020). Across Europe, the Baltic Sea and the North Sea are projected to experience the highest increase in extreme sea levels (ESLs) towards the end of the century (Vousdoukas et al., 2017). In the Baltic Earth Assessment Reports (BEAR), Rutgersson et al. (2022) suggest that Germany, among other Baltic Sea littoral states, is likely to suffer severe damage from increased coastal flooding due to climate change.

In the Baltic Sea, ESLs occur on various spatio-temporal scales, and all phenomena contributing to ESLs are mainly

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generated by meteorological and, to a limited extent, astronomical factors (Weisse and Hünicke, 2019). The most important contributions to ESLs come from storm surges, wind waves, and preconditioning, the latter of which leads to increased water volumes in the Baltic Sea before the onset of a storm (Weisse et al., 2021; Suursaar et al., 2006; Madsen et al., 2015). Due to the microtidal regime, storm surges can persist for relatively long periods, ranging from hours to several days (Wolski and Wiśniewski, 2020; MacPherson et al., 2019).

Facing a growing demand for flood risk assessments at regional scales, a better understanding of the consequences of natural hazards such as storm surges and associated extreme sea levels (ESLs) is required. Such assessments can enable society and decision-makers to selectively invest in adaptation options and support policy-making (Vousdoukas et al., 2018; Rutgersson et al., 2022). The key component in improving our understanding of the impacts of SLR and ESL constitutes the detailed mapping of coastal floodplains. Highquality inundation maps, including information on flood depth and extent, are critical for coastal impact assessments, since the density of valuable assets often tends to increase towards the coast (Vousdoukas et al., 2016). This need has also been identified by European legislation: since 2007, the EU Flood Directive 2007/60/EC (2007) has required member states to identify and map areas, assets, and populations at risk from flooding along coastlines and water courses and to take measures to reduce this risk. For this purpose, the Directive obliges member states to produce high-quality inundation maps that account for ESL, SLR, and existing coastal protection infrastructure.

State-of-the-art coastal flood maps should consider oceanographic forcings, projected SLR, detailed topographic data including anthropogenic coastal protection measures such as dikes, and the effects of land cover on flood propagation. Oceanographic forcings determining flood characteristics are the magnitude of the surge (i.e. the peak water level), the duration of the surge (particularly in microtidal environments such as the Baltic Sea), and wave setup (Höffken et al., 2020; Hendry et al., 2019). Topographic datasets constitute a major source of uncertainty in large-scale coastal flood modelling, particularly because natural and anthropogenic flood barriers such as dikes are often not sufficiently well resolved (Leszczyńska et al., 2022; Vousdoukas et al., 2018). Problematically, disregarding human adaptation constitutes the largest bias in regional- to global-scale flood risk assessments, and data availability is often a major concern (Hinkel et al., 2021). Even if data of good quality exist for a specific region, the simulation of coastal flood extents at a regional scale is computationally expensive, often leading to the application of simplified, static bathtub models in coastal flood impact assessments. The bathtub model maps all coastal areas hydrologically connected to the sea below a certain (extreme) water level as inundated, without accounting for the alteration of flow as a consequence of varying surface roughness or the temporal evolution of the surge (e.g. duration). Even though these models may perform well depending on the geomorphic setting (Kumbier et al., 2017), several recent studies have pointed out that the bathtub model can overestimate coastal flood extents (Vousdoukas et al., 2016; Lopes et al., 2022; Didier et al., 2019).

Due to the drawbacks of static inundation models, more complex hydrodynamic models are increasingly used to map coastal floodplains from local to continental scales (Vousdoukas et al., 2016; Lopes et al., 2022; Didier et al., 2019; Bates et al., 2021; Leijnse et al., 2021). However, the application of such models is complex, sensitive to the model setup (surface roughness parameterisation, solvers etc.) and computationally demanding, particularly when modelling at higher resolutions. This constitutes a dilemma, as the quality of regional-scale assessments can be affected by coarse resolutions, which result in inaccuracies associated with the representation of natural and anthropogenic flood barriers such as beach ridges, dunes, seawalls, or dikes. In addition, comprehensive datasets on the location and characteristics of coastal protection infrastructure are missing in many parts of the world. In this context, Vousdoukas et al. (2018) highlight the urgent need for broad-scale but highly detailed datasets of coastal protection standards. Additional uncertainties arise from the lack of data to validate modelled flood extents, the extrapolation of return water levels beyond the length of tide gauge records, and the projections of future sea levels (Vousdoukas et al., 2016, 2018).

Model validation is one of the major challenges in coastal flood risk analysis, as it provides an awareness of model limitations. Recent advances in the application of hydrodynamic models for flood impact assessments have not been matched by advances in model validation (Rollason et al., 2018); in particular, the lack of validation data of appropriate spatial and temporal coverage constitutes a major problem (Molinari et al., 2019). For instance, studies that have modelled on global to continental scales have validated simulated flood extents with data covering only a very small part of the entire study region (Vousdoukas et al., 2016; Sampson et al., 2015).

Along the German Baltic Sea coast, existing studies on coastal flooding either use state-of-the-art hydrodynamic models, but cover only a small fraction of the study region (Höffken et al., 2020; Vollstedt et al., 2021), or assess potential flood extents for the entire region but rely on global topographic data sources and apply the bathtub approach (Schuldt et al., 2020). In addition, the validation of produced flood extents is not provided. There is a need to simulate coastal flooding on a regional scale, considering the limitations of large-scale coastal flood mapping mentioned before. This is particularly true for limitations associated with topographic data sources and the incorporation of coastal protection infrastructure, which constitute the main bottlenecks for the quality of coastal flood risk assessments (Vousdoukas et al., 2018; Hinkel et al., 2021).

The German Baltic Sea coast comprises a total length of 2538 km, and approximately one-third of this coast is protected by dikes (Sterr, 2008; van der Pol et al., 2021). Today, state embankments in both federal states, Schleswig-Holstein (SH) and Mecklenburg-Western Pomerania (MP), are designed to be high enough to prevent flooding during a storm surge with a return period of 200 years plus a buffer of 0.5 m to account for SLR (Melund, 2022; StALU, 2012). On the other hand, regional dikes have a variable but generally lower standard of protection (Melund, 2022). During the last two decades, the concept of so-called climate dikes has been introduced as a paradigm in embankment construction. The climate dike accounts for uncertainties related to SLR projections by having wider dike crests, which allows for a comparatively easy and rapid increase in dike heights without reconstructing the dike base. The climate dike can easily be increased in height by up to 1.5 m, and suggested extension options include 0.5 and 1.0 m (Melund, 2022).

Here, we simulate coastal flooding along the German Baltic Sea coast for a storm surge that aligns with the design standard of state embankments in the region, i.e. the 200-year return water level. This study aims at (1) exploring how the flood extent may change until the end of the century, if existing dikes are not upgraded, by applying two high-end SLR scenarios (1 and 1.5 m); (2) identifying hotspots of coastal flooding in the study region; and (3) evaluating the use of SAR imagery for validating the simulated flood extents. To the knowledge of the authors, this study constitutes the first regional-scale assessment using a high-resolution, fully validated, and offline-coupled hydrodynamic modelling framework that incorporates natural and anthropogenic flood barriers to assess extreme sea levels and associated coastal flooding along the German Baltic Sea coast.

We apply a newly developed hydrodynamic modelling framework for the entire German Baltic Sea coast. Specifically, we use high-resolution topographic data sources extracted from a 1 m × 1 m light detection and ranging (lidar)derived digital elevation model (DEM) to identify the height of natural and anthropogenic coastal protection structures such as dunes and dikes. Further, we have access to detailed data on the location of all dikes in the study region. We consider the temporal evolution of storm surges and the effects of spatially varying surface roughness on flood propagation, and we explore the suitability of Sentinel-1 synthetic aperture radar (SAR) data to validate a large fraction of our study region. The applied modelling framework offline couples a hydrodynamic coastal inundation model (50 m resolution) of the German Baltic Sea coast with a hydrodynamic coastal ocean model (200 m resolution) covering the entire western Baltic Sea. The coastal ocean model produces spatially varying boundary conditions for the coastal inundation model. Using this setup, we perform four storm surge simulations representing (1) a 200-year event, (2) a 200-year event and 1 m SLR, (3) a 200-year event and 1.5 m SLR, and (4) a simulation of the storm surge that occurred in the study region on 2 January 2019. We use the 2019 storm surge to validate the water levels generated by the coastal ocean model and the flood extent generated by the coastal inundation model. We compare flood maps derived from our coastal inundation model with flood extents generated from SAR satellite imagery. These images were acquired on the same day of the surge, only a few hours after it peaked. More importantly, they cover a significant part of the study area. Since local tide gauges have not yet recorded a storm surge with a 200-year return period, we constructed these events by using extreme value statistics and extracting mean surge shapes from a hindcast simulation of the coastal ocean model.

2 Study area

The Baltic Sea constitutes a semi-enclosed brackish water basin that is comparatively shallow (between 53 and 55 m on average depending on the dataset (Jakobsson et al., 2019)) and has its only connection to the North Sea through the Kattegat and Skagerrak (Fig. 1). The Baltic Sea is characterised by a microtidal regime (tidal range varying between 0.1 and 0.2 m (Sterr, 2008)), low salinity, strong stratification, and anoxic conditions in many areas (Meier et al., 2022). The German part of the Baltic Sea is located in the south-west of the sea's catchment and comprises the federal states of Schleswig-Holstein (SH) and Mecklenburg-Western Pomerania (MP) (Fig. 1). The coastal length of the German Baltic Sea is approximately 2538 km, of which 649 km is located in SH and 1889 km in MP (van der Pol et al., 2021). The German Baltic Sea coast is characterised by fjords, lagoons, islands, beaches, and soft cliffs. In contrast to the emerging northern Baltic Sea coast, parts of the southern Baltic Sea coast are subsiding as a consequence of glacial isostatic adjustment. While subsidence is generally variable and mostly well below 1 mm yr^{-1} , it can locally reach up to $2 \,\mathrm{mm} \,\mathrm{yr}^{-1}$ (Richter et al., 2012; Weisse et al., 2021; Dangendorf et al., 2022). Due to the spatial variability and limited consistent information, we excluded subsidence from the present analysis.

The German Baltic Sea mostly experiences storm surges during strong easterly winds. The highest surge to date occurred in 1872, reaching peak water levels between 2.4 and 3.4 m above the German ordnance datum (NHN) in the federal state of SH. During this event, 31 people died and 15 000 lost their homes. The 1872 surge was a turning point for adaptation planning along the German Baltic Sea coast, resulting in new design standards for embankment constructions in the aftermath of the storm (Hofstede and Hamann, 2022).

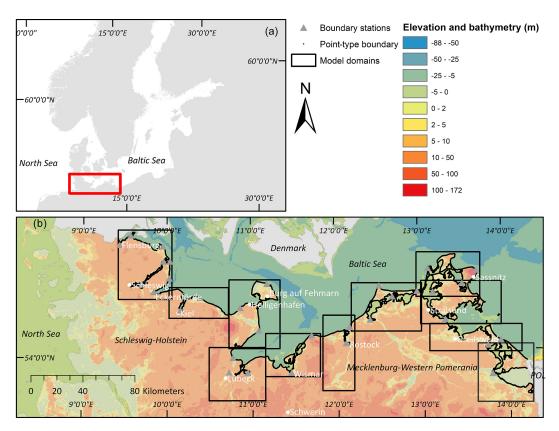


Figure 1. (a) Overview map of the study region. (b) The German Baltic Sea and a depiction of the coastal inundation model setup. Bathymetry data shown in this figure were taken from the Baltic Sea Bathymetry Database (Helcom Secretariat) and the elevation data from the Global Multi-resolution Terrain Elevation Data 2010: US Geological Survey Open-File Report 2011–1073, 26 pp. http://pubs.usgs.gov/of/2011/1073/ (last access: 26 July 2021), available via Helcom.

3 Methods and data

3.1 Overview of the modelling framework and simulated scenarios

In order to simulate flood characteristics along the German Baltic Sea coast, we employ a new modelling framework where we offline couple two hydrodynamic models, a coastal ocean model (GETM) with a coastal inundation model (LISFLOOD-FP). The coastal ocean model provides boundary conditions, which we use to simulate coastal flood characteristics in the inundation model. We apply this model setup to simulate four events and scenarios: (1) the storm surge that occurred on 2 January 2019, which we use to validate the coastal ocean and the inundation model; (2) a 200-year event that is used to determine the design heights of dikes along the German Baltic Sea coast; (3) the 200year event and 1 m SLR; and (4) a high-end scenario of the 200-year event including 1.5 m SLR (SLR until the year 2100). We add SLR linearly to the 200-year event (Hieronymus et al., 2018). The SLR scenarios correspond to the regional-scale medium-confidence projections of Shared Socio-economic Pathway (SSP)5-8.5 (ranging between the 50th and 83rd percentiles) of the Sixth Assessment Report of the Intergovernmental Panel on Climate Change (IPCC) for the tide gauges in Lübeck-Travemünde and Wismar (Fox-Kemper et al., 2021). Both sea-level-rise scenarios are used in current coastal protection planning within the scope of the climate dike concept as a so-called "building reserve". The building reserve allows the dike heights to be increased by up to 1.5 m at the end of this century with comparatively little effort (Melund, 2022).

3.2 Numerical setups

3.2.1 Coastal ocean model: GETM

The General Estuarine Transport Model (GETM; Burchard and Bolding, 2002) constitutes a structured coastal ocean model (Klingbeil et al., 2018) that solves the Reynolds-averaged Navier–Stokes (RANS) equations in the Boussinesq approximation. In this study, we employ GETM to perform a hindcast simulation of the sea-level evolution from 1961 to 2018 using its vertically integrated mode.

We employ a setup of the western Baltic Sea in 200 m resolution based on the bathymetry obtained from the European Marine Observation and Data Network (EMODnet;

https://emodnet.ec.europa.eu, last access: 20 July 2020). As atmospheric forcing we use the UERRA-HARMONIE system of the Uncertainties in Ensembles of Regional (https://cds.climate.copernicus.eu/cdsapp#!/ ReAnalysis dataset/reanalysis-uerra-europe-complete?tab=overview, last access: 21 February 2021) project, i.e. hourly wind (in 10 m height) and sea-level pressure fields in 11 km resolution. The wind speeds of the forcing have been increased by 7 % to improve the model's ability to match the observed peak water levels during the storm surges (Lorenz and Gräwe, 2023). The friction in the model is depth dependent using the law of the wall with a roughness length of $z_0 = 5 \times 10^{-5}$ m. This value is smaller than usual, but the model still tends to underestimate ESLs (see Sect. 4.1). The total variation diminishing scheme Superbee is used as the advection scheme (Pietrzak, 1998). The 200 m setup uses boundary conditions of a nesting hierarchy starting with a coarse setup of the north-west Atlantic Ocean (see Gräwe et al., 2015, for a detailed description of the nesting). The boundary conditions are extracted from a 1 NM setup of the North and Baltic seas, which uses ERA5 (Hersbach et al., 2020) as atmospheric forcing (winds and pressure, hourly, 32 km resolution), but neglect tides for this study as a consequence of the microtidal regime of the study region (Gräwe and Burchard, 2012; Arns et al., 2020). Since the UERRA data ended in 2018, the 2019 surge used here to validate the simulated flood extent is computed using the German Weather Service (DWD) forecast (3-hourly, 7 km; Zängl et al., 2015). In order to keep the computation of the modelled return levels consistent to one dataset, the 2019 surge is not included in the return-level computation.

3.2.2 Coastal inundation model: LISFLOOD-FP

LISFLOOD-FP (hereafter referred to as Lisflood) is a raster flood inundation model that is used to simulate fluvial or coastal flood propagation (Bates et al., 2013). Lisflood includes several solvers that simulate the propagation of the flood wave along channels and across floodplains using simplifications of the shallow-water equations. This makes it a reduced-complexity model that is a faster alternative to full shallow-water models but that provides results of similar accuracy (Neal et al., 2011; Bates et al., 2013). Therefore, Lisflood is particularly useful for large-scale inundation modelling that would otherwise be too computationally expensive (Vousdoukas et al., 2016; Bates et al., 2021).

3.2.3 Model setup in Lisflood

From the available solvers in Lisflood, we applied the floodplain solver "acceleration", which is a simplified form of the shallow-water equations, excluding only the convective acceleration term. The flow is calculated using acceleration as a function of friction, water slopes, and local water acceleration (see Bates et al. (2013) for the respective equations). The time step varies throughout the simulation according to the Courant–Friedrichs–Lewy condition and is related to the cell size and water depth (Bates et al., 2013).

In order to model the entire German Baltic Sea coast in 50 m resolution, we divided the study region into 11 model domains, each covering an area of on average 1739 km² (Fig. 1). Model domains were defined considering waterlevel variability across the study area so that each model domain is characterised by comparatively homogeneous water levels. We used a point-type boundary to force the model, with a boundary point placed every 50 m along the model coastline (55 169 points in total) (Table 1). In order to account for spatial variations in water levels within each model domain, we defined a total of 32 "boundary stations" (Fig. 1), for which hydrographs (water-level time series) were modelled in the coastal ocean model (GETM) for the four storm surge scenarios (2019 surge, 200-year event, 200-year event with 1 m SLR, and 200-year event with 1.5 m SLR). The 32 boundary stations used in this study are located at different locations as compared with existing tide gauges. Each boundary point received the boundary conditions (the hydrograph used to force the coastal inundation model in Lisflood) from the nearest boundary station (Fig. 1). In the cases where the nearest flood boundary station to a boundary point located on the open coast was situated inside protected fjords or lagoons, or vice versa, we manually corrected that point to ensure that open-coast boundary points were not forced with hydrographs of sheltered locations.

The model elevation and bathymetry were compiled as follows: we first aggregated the 10 m lidar-derived elevation data to 50 m in order to make it consistent with the bathymetry data (Table 1). Then, both datasets were merged by differentiating between sea (bathymetry) and land (elevation) areas. A resolution of 50 m is insufficient to reliably resolve natural and anthropogenic coastal protection barriers, as dike crests are typically narrower than 50 m, so grid sizes smaller than 10 m are recommended (Vousdoukas et al., 2012a, b).

We incorporated detailed information on dikes (location and height) into the modelling framework by using a highresolution lidar-derived (1 m × 1 m) DEM and comprehensive datasets on the location of both state dikes and regional dikes from local state authorities (Table 1). The incorporation of dikes constitutes one of the major improvements of the applied modelling framework as compared with previous regional- or continental-scale assessments (Vousdoukas et al., 2016). Elevation data and coastal protection levels are considered to be the main bottlenecks for the quality of coastal flood risk assessments (Vousdoukas et al., 2018; Hinkel et al., 2021). Without correcting for dike heights in a 50 m DEM, the simulated flood extent will be overestimated, as the elevations of the dike heights are averaged out due to the resolution (Vousdoukas et al., 2012a, b). The difference between a DEM with and without dike height correction is shown in Fig. A1 in the Appendix. The integration of dikes

Table 1. Datasets used to set up the coastal inundation model in Lisflood.

Dataset	Resolution	Source	Accuracy
Bathymetry	50 m	Federal Maritime and Hydrographic Agency (BSH)	NA*
Elevation	10 m	ATKIS [®] DEM 10 (lidar); State Office of Geoinformation, Surveying and Cadestre MP and State Office for Surveying and Geoinformation SH	0.5–2.0 m
Elevation	1 m	ATKIS [®] DEM 1 (lidar); State Office of Geoinformation, Surveying and Cadestre MP and State Office for Surveying and Geoinformation SH	< 30 cm horizontally and 15-20 cm vertically in flat terrain
Land cover	100 m	Corine (© European Union, Copernicus Land Monitoring Service 2018, European Environ- ment Agency (EEA))	Geometric accuracy $< 100 \text{m};$ thematic accuracy $> 85 \%$
Coastline	Shapefile	van der Pol et al. (2021)	NA
Dike SH	Shapefile	ATKIS [®] ; State Office for Surveying and Geoinformation SH	The dataset contains full coverage of flood protection dikes in SH. We used the layers rel01 and geb03. Selected shapes are Hochwasserdeich, Hauptdeich, Landesschutzdeich, Überlaufdeich, Leitdeich, Schlafdeich, Mitteldeich, Binnendeich, Hauptdeich 1. Deichlinie and 2. Deichlinie.
Dike MP	shapefile	State Office for Agriculture and the Environment Mittleres Mecklenburg, Coastal Division (internal data)	State dikes and regional dikes of water and soil associations

^{*} NA = not assessed

into the 50 DEM was done as follows. First, we extracted elevations from the 1 m \times 1 m DEM within a 100 m buffer around the coastline and the dike shapes of SH and MP. We extracted 1 m \times 1 m elevations around the coastline in order to accurately resolve natural flood barriers such as dunes and beach ridges but also to account for a variety of hard coastal protection measures, such as revetments and seawalls. The 100 m buffer was used to account for inaccuracies related to the data on dike positions and the DEM. In the next step, we aggregated these datasets to 50 m by using maximum elevations and merged them with the elevation bathymetry data.

We derived the surface roughness of the study region by using land cover data from Corine (© European Union, Copernicus Land Monitoring Service 2018, European Environment Agency (EEA)) (Table 1) to assign Manning's n coefficients. These coefficients are commonly employed to parameterise the bottom friction of various land cover types in hydrodynamic simulations (Garzon and Ferreira, 2016). Since Manning's n coefficients are not available for all Corine classes present in the study region, we reduced the number of classes by reclassification. The reclassification scheme is provided in Table A1 in the Appendix. We

then assigned five configurations of Manning's n coefficients to the remaining 10 categories. We followed the approach described in Höffken et al. (2020). First, we searched the literature for a variety of Manning's n surface roughness coefficients for each of the 10 land cover classes. We then categorised them into high, low, and moderate values (Table 2). Some studies have used uniform Manning's n coefficients or setups where the only separation in surface roughness was made between land and water areas (Table 2). We added these two setups to the configurations of Manning's n coefficients that we used for model calibration (Table 2).

In this study, model outputs (including the validation run representing the 2019 storm surge) refer to the maximum water depth (and thus extent) predicted by the model for each pixel over the course of the simulation period.

3.3 Sensitivity analysis

We tested the sensitivity of our model to variations in surface roughness coefficients using the different values of Manning's n coefficients shown in Table 2. For this purpose, we modelled the storm surge from January 2019 and selected the three model domains that contained the largest flood extent

Table 2. Configuration of Manning's *n* coefficients for selected land cover classes.

Land use class	High	Low	Moderate	Uniform	Land/water
Agriculture	0.06^{1}	0.03 ^{3,5}	0.04 ¹	0.035^{8}	0.03^2
Forest	0.2^{2}	$0.1^{4,5}$	$0.15^{3,6,7}$	0.035^{8}	0.03^{2}
Urban	0.15^{1}	0.015^{6}	0.07^{3}	0.035^{8}	0.03^{2}
Wetland	0.08^{1}	0.035^{1}	0.06^{1}	0.035^{8}	0.03^{2}
Sea and ocean	0.03^{2}	0.012^2	0.02^{2}	0.035^{8}	0.02^{2}
Inland waterbodies and water courses	0.06^{2}	0.02^{2}	0.035^2	0.035^{8}	0.035^2
Green urban areas	0.12^{1}	$0.035^{1,7}$	0.07^{1}	0.035^{8}	0.03^{2}
Natural grasslands	0.042^{1}	0.034^4	0.035^{7}	0.035^{8}	0.03^{2}
Traffic	0.032^{1}	0.013^{5}	$0.016^{6,7}$	0.035^{8}	0.03^{2}
Unvegetated coastal sediment	0.09^{4}	0.025^{6}	$0.04^{1,3}$	0.035^{8}	0.03^{2}

¹ Bunya et al. (2010). ² Garzon and Ferreira (2016). ³ Wamsley et al. (2009). ⁴ Liu et al. (2013). ⁵ Papaioannou et al. (2018). ⁶ Hossain et al. (2009). ⁷ Dorn et al. (2014). ⁸ Liang and Smith (2015).

(1, 7, and 8 from left to right in Fig. 1). The analysis showed that our model results are robust against variations in surface roughness coefficients (Table A2). Specifically, variations in flood extent between the highest and lowest configurations in Manning's n coefficients vary from $0.09 \, \mathrm{km^2}$ (1.5 %, domain 1) to $2.32 \, \mathrm{km^2}$ (9.5 %, domain 7), while differences in domain 8 amount to $1.04 \, \mathrm{km^2}$ (3.1 %). Variations in water depth between the highest and lowest configurations in Manning's n coefficients are even smaller, varying between 1 cm (domains 1, 8) and 2 cm (domain 7) (2.1 % and 5.1 %, respectively). Due to the small differences in flood characteristics, we employed Manning's moderate n coefficients in our model setup (Table 2).

3.4 General extreme value (GEV) statistics

To describe the distribution of ESL and return periods (RPs), we employed the commonly used general extreme value (GEV) distribution (Coles, 2001). We use the time series of annual storm season maxima (July to June) of 30 tide gauges and 32 boundary stations (see also Sect. 3.2.3). The GEV is defined by

$$F(t, \mu, \sigma, \xi) = \exp\left\{-\left[1 + \xi\left(\frac{t - \mu}{\sigma}\right)\right]^{-1/\xi}\right\},\tag{1}$$

where t is the sea level, μ the location parameter, σ the scale parameter, and ξ the shape parameter. For each gauge and boundary station used in this study, the GEV is fitted against the time series of the annual storm season's maxima. In this study we use the Python code of Reinert et al. (2021) for the fitting. Using this model, we extrapolate 200-year return water levels for each tide gauge and boundary station. Note that slow, long-term variations in the mean sea level have been subtracted from the time series by a linear fit. Therefore, we only consider ESLs relative to the mean sea level in the statistics. Due to the microtidal regime of the Baltic Sea, the derived return periods and water levels correspond only to

the surge component (i.e. non-tidal residual) and neglect tidal contributions to ESL, as the latter (tide–surge interactions) is negligible in the study region (Arns et al., 2020).

3.5 Construction of the 200-year event

Since there are no tide gauge observations of storm surges with a return period of 200 years in the study region, we constructed the hydrographs (water-level time series) for such events as input for the coastal inundation model: within the modelled time frame from 1961 to 2018, we extracted for each boundary station all events exceeding a water level of 1 m above mean sea level, which is the threshold for an ESL defined by the German Federal Maritime and Hydrographic Agency for the German Baltic Sea coast. We made sure that at least 48 h separated the peak water levels of individual events. For the boundary stations where the water level never reached 1 m, annual maxima were used instead, e.g. in the Saaler Bodden lagoon. Each extracted time series has a time step of 1 h. Its water level is normalised by the maximum water level of the individual event and then interpolated to a time step of 15 min using cubic interpolation. The water-level time series of each extracted surge starts 3 d before the peak water level and ends 3 d after. This allows us to compute the mean evolution of a surge by taking the mean of all events per station (Fig. 2a). By multiplying the mean, normalised evolution by the respective 200-year return level obtained from the GEV analysis, we constructed the hydrographs of the 200-year event (see Fig. 2b–f as examples). These hydrographs are then used as boundary conditions for the coastal inundation model.

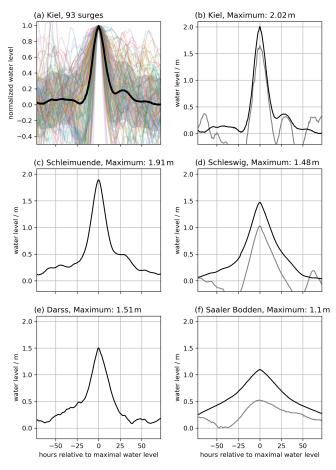


Figure 2. (a) The normalised surge shape (thick black line), its standard deviation (grey area), and the individual time series (coloured) for the Kiel boundary station. (b) The constructed 200-year event for the Kiel boundary station. (c) The constructed 200-year event for the Schleimünde boundary station. (d) The constructed 200-year event for the Schleiminde boundary station. (e) The constructed 200-year event for the Darss boundary station. (f) The constructed 200-year event for the Saaler Bodden boundary station. The grey lines in (b), (d), and (f) are the hydrographs of the 2019 surge from the respective boundary stations as a comparison.

3.6 Data used for model validation

3.6.1 Tide gauge data to validate coastal ocean model

To evaluate the coastal ocean model's performance, we compared the modelled ESL to tide gauge observations listed in Table 3. In Sect. 4.1 we compare the model's capability to reproduce observed ESL (30-year return water level) and its ability to extrapolate 200-year return levels. We used the full record length of each tide gauge, which is often longer than the hindcast period simulated with the coastal ocean model. Since only Gedser (5) and Hornbaek (8) are significantly longer, we included the GEV return levels based on the overlapping time series for both tide gauges in Table 4. For each gauge, the SLR and other slow, long-term changes

in the mean sea level have been removed by subtracting a linear fit of the time series.

3.6.2 Geodetic levelling data to validate dike heights in coastal inundation model

We used 9519 high-accuracy real-time kinematic (RTK) GPS points of the official dike crest geodetic levelling provided by the Schleswig-Holstein State Agency for Coastal Protection, National Park and Marine Conservation (LKN) to validate the dike heights extracted from the DEM 1 (1 m \times 1 m horizontal resolution; Table 1). We compared the LKN RTK points and the dike crest elevations of the coastal inundation model within a 25 m buffer around the dike line used in this study (Table 1). Next, in order to minimise errors of location (RTK points vs. 50 m \times 50 m cells), we removed all cells of the coastal inundation model that were outside the value range of the geodetic dike levelling.

3.6.3 Sentinel-1 SAR imagery used to validate flood extents simulated with the coastal inundation model

We used Sentinel-1 SAR imagery to compute the flood extent of the storm surge that occurred on 2 January in the study area. The SAR image covers a large portion of the coastline of the federal state of SH and is therefore used for evaluating the output of the coastal inundation model. SAR satellite missions enable the monitoring of flood extent over large geographical areas and at high spatial resolution independent of cloud coverage and illumination conditions. The specular reflectance of radar pulses at the water surface results in low return signals at the sensor, which enables the delineation of water bodies from other surfaces (Clement et al., 2018). The European Space Agency's Sentinel-1 (S-1) mission offers high-spatial-resolution C-band SAR imagery, whereby the operation of the twin satellites S-1A and S-1B enables observations at an increased repeat cycle of 6 d, which is useful for capturing short-term events. S-1B acquired the region of interest in ascending orbit and interferometric wide (IW) swath instrument modes on 2 January 2019 at 17:08 UTC. We used Google Earth Engine to access the calibrated and orthocorrected ground-range-detected (GRD) product at 10 m spatial resolution (Google Earth Engine (GEE), 2022). After visual inspection, we used VV (vertical transmit and vertical receive) polarisation and applied a focal median filter with a radius of 70 m to reduce noise. We then applied a simple threshold of $-16 \, dB$ to compute a binary water map and used the shorelines of lakes, supposedly unaffected by the surge, to visually assess its suitability.

Table 3. Overview of the tide gauges used in this study, their record lengths, and their locations. We defined gaps in the time series as missing data with lengths greater than 1 d. The data are obtained from the European Marine Observation and Data Network (EMODnet; https://emodnet.ec.europa.eu, last access: 20 July 2020).

Number	Tide gauge name	Record lengths (yyyy/mm/dd)	Long/lat	Number of gaps
0	Althagen	1953-11-01 to 2020-12-31	12.42/54.37	3
1	Barhoeft	1954-11-01 to 2020-12-31	13.03/54.43	1
2	Barseback	1982-04-26 to 2020-12-31	12.90/55.76	None
3	Eckernförde	1989-11-01 to 2020-12-31	9.84/54.47	3
4	Flensburg	1954-11-01 to 2020-12-25	9.43/54.79	4
5	Gedser	1891-09-01 to 2020-12-31	11.93/54.57	None
6	Greifswald	1963-11-01 to 2020-12-31	13.45/54.09	None
7	Heiligenhafen	1989-06-01 to 2020-12-31	11.01/54.37	6
8	Hornbaek	1891-01-01 to 2020-12-31	12.46/56.09	None
9	Kappeln	1991-11-01 to 2020-12-31	9.94/54.66	None
10	KielHoltenau	1964-11-01 to 2020-12-31	10.16/54.37	None
11	Klagshamn	1929-11-13 to 2020-12-31	12.89/55.52	None
12	Koserow	1972-11-01 to 2019-11-13	14.00/54.06	9
13	LTKalkgrund	1990-05-01 to 2020-12-31	9.89/54.82	None
14	Langballigau	1991-11-01 to 2020-12-31	9.65/54.82	None
15	Neustadt	1991-11-01 to 2020-12-31	10.81/54.10	1
16	Rostock	1968-11-01 to 2020-12-31	12.15/54.08	2
17	Sassnitz	1954-08-01 to 2020-12-31	13.64/54.51	None
18	Schleimünde SP	1990-11-01 to 2020-12-31	10.04/54.67	None
19	Schleswig	1991-11-01 to 2020-12-31	9.57/54.51	None
20	Simrishamn	1982-05-31 to 2020-12-31	14.36/55.56	None
21	Skanor	1992-02-17 to 2020-12-31	12.83/55.42	None
22	Stralsund	1961-11-01 to 2020-12-31	13.10/54.32	2
23	Timmendorf	1961-11-01 to 2020-12-31	11.38/53.99	1
24	Travemünde	1949-11-01 to 2020-12-31	10.87/53.95	1
25	Ueckermuende	1965-11-01 to 2020-12-31	14.07/53.75	3
26	Viken	1976-04-22 to 2020-12-31	12.58/56.14	None
27	Warnemünde	1953-11-01 to 2020-12-31	12.10/54.17	5
28	Wismar	1957-11-01 to 2020-12-31	11.46/53.90	5
29	Wolgast	1965-11-01 to 2020-12-31	13.77/54.04	2

4 Results and discussion

4.1 Validation of extrapolated extreme sea levels

From the hindcast simulation between 1961 and 2018, we first compared the observed and modelled ESL events with water levels exceeding 1 m above mean sea level (Fig. 3). For each tide gauge (Table 3), we computed the bias and standard deviation between observed and modelled ESLs (see Fig. 3a for the Kiel Holtenau (10) tide gauge as an example). Overall, the model has a negative bias, underestimating the ESLs by 11 cm on average (Fig. 3b).

Despite the negative bias of the ESLs, the model can reproduce the GEV distributions for most tide gauges (see Fig. 4 for Kiel Holtenau (10) and Table 4 where the return levels for a 30-year event and 200-year event are listed). For the tide gauges with the longest time series, Gedser (5) and Hornbaek (8), the model shows higher return levels than the observations when the observed time series are clipped to the model time period before analysis. The reason for this is that the

largest surge at these locations lies outside the modelled time period, which changes the tail of the GEV distributions. This is a common problem in extreme value statistics. An event with a high return level can change the tail of the distribution and thus the extrapolation. Similarly, from the negative bias of the model for the high surges (Fig. 3a) one would expect that the GEV statistics also show a clear negative bias for the high return periods. However, one overestimated event (the highest surge in the hindcast period) is enough to "fix" the tail of the GEV distribution in this case. Nevertheless, the 95 % confidence intervals from the model statistics include the return levels of the GEV of the clipped time series. For Althagen (0), Kappeln (9), and Schleswig (19), the model overestimates the return levels since the lagoons are not sufficiently resolved at the resolution of 200 m. Because of the deviations at these locations, we used the tide gauge observations as input for the coastal inundation model instead.

For Barseback (2) and Viken (26) the model underestimates the return levels, as also shown in Fig. 3b, with Barse-

Table 4. List of the return levels (RLs) of the 30- and 200-year return periods including the 95 % confidence intervals for different tide gauges along the Baltic Sea coast. RLs were estimated from observations (obs.) and model results (mod.).

Number	Tide gauge name	Obs. 30-year $RL m^{-1}$	$Mod. 30$ -year $RL m^{-1}$	Obs. 200-year $RL m^{-1}$	Mod. 200-year $RL m^{-1}$
0	Althagen	0.92 ± 0.24	1.36 ± 0.19	1.10 ± 0.36	1.71 ± 0.44
1	Barhoeft	1.35 ± 0.08	1.33 ± 0.10	1.44 ± 0.12	1.53 ± 0.12
2	Barseback	1.31 ± 0.13	0.91 ± 0.08	1.45 ± 0.19	1.00 ± 0.16
3	Eckernförde	1.69 ± 0.21	1.74 ± 0.16	1.91 ± 0.39	2.05 ± 0.28
4	Flensburg	1.63 ± 0.11	1.72 ± 0.16	1.80 ± 0.21	2.03 ± 0.29
5	Gedser	1.51 ± 0.08	1.51 ± 0.13	1.71 ± 0.11	1.74 ± 0.26
5	Gedser (overlap)	1.39 ± 0.09	1.51 ± 0.13	1.51 ± 0.15	1.74 ± 0.26
6	Greifswald	1.49 ± 0.12	1.49 ± 0.12	1.65 ± 0.22	1.68 ± 0.23
7	Heiligenhafen	1.72 ± 0.35	1.61 ± 0.17	2.15 ± 0.83	1.90 ± 0.35
8	Hornbaek	1.49 ± 0.06	1.43 ± 0.13	1.64 ± 0.07	1.60 ± 0.25
8	Hornbaek (overlap)	1.47 ± 0.06	1.43 ± 0.13	1.56 ± 0.07	1.60 ± 0.25
9	Kappeln	1.36 ± 0.13	1.66 ± 0.18	1.51 ± 0.23	2.01 ± 0.34
10	KielHoltenau	1.73 ± 0.18	1.73 ± 0.16	1.99 ± 0.36	2.03 ± 0.29
11	Klagshamn	1.13 ± 0.08	1.20 ± 0.11	1.27 ± 0.16	1.37 ± 0.21
12	Koserow	1.42 ± 0.05	1.38 ± 0.11	1.47 ± 0.07	1.56 ± 0.26
13	Langballigau	1.66 ± 0.26	1.69 ± 0.16	1.93 ± 0.56	1.99 ± 0.28
14	LTKalkgrund	1.67 ± 0.29	1.67 ± 0.15	2.03 ± 0.65	1.96 ± 0.27
15	Neustadt	1.64 ± 0.22	1.69 ± 0.17	1.85 ± 0.43	1.99 ± 0.33
16	Rostock	1.55 ± 0.19	1.52 ± 0.15	1.83 ± 0.39	1.75 ± 0.31
17	Sassnitz	1.22 ± 0.08	1.24 ± 0.08	1.33 ± 0.14	1.44 ± 0.13
18	Schleimünde SP	1.62 ± 0.24	1.69 ± 0.15	1.88 ± 0.49	1.98 ± 0.26
19	Schleswig	1.39 ± 0.11	1.91 ± 0.28	1.48 ± 0.21	2.44 ± 0.65
20	Simrishamn	1.09 ± 0.09	1.19 ± 0.11	1.19 ± 0.15	1.37 ± 0.23
21	Skanor	1.27 ± 0.21	1.31 ± 0.10	1.46 ± 0.44	1.48 ± 0.19
22	Stralsund	1.41 ± 0.13	1.39 ± 0.11	1.59 ± 0.23	1.58 ± 0.21
23	Timmendorf	1.65 ± 0.18	1.65 ± 0.18	1.93 ± 0.38	1.94 ± 0.38
24	Travemünde	1.71 ± 0.13	1.72 ± 0.18	1.92 ± 0.24	2.03 ± 0.36
25	Ueckermuende	1.00 ± 0.12	0.90 ± 0.13	1.15 ± 0.25	1.11 ± 0.30
26	Viken	1.49 ± 0.16	1.32 ± 0.13	1.69 ± 0.31	1.49 ± 0.25
27	Warnemünde	1.54 ± 0.16	1.50 ± 0.15	1.81 ± 0.35	1.74 ± 0.32
28	Wismar	1.69 ± 0.13	1.68 ± 0.19	1.89 ± 0.23	1.97 ± 0.40
29	Wolgast	0.96 ± 0.11	0.95 ± 0.15	1.09 ± 0.23	1.21 ± 0.40

back (2) having the largest negative bias (note that both tide gauges are located in Sweden). The spatial distribution of the 30- and 200-year return levels (Fig. 5) shows that the highest ESLs occur at the coast, with decreasing ESLs from west to east. This pattern has already been described in the literature (e.g. Gräwe and Burchard, 2012; Wolski et al., 2014) and is primarily due to the fetch length, which is longer for the coast of SH compared with MP during (north)-easterly winds.

4.2 Validation of dike height extraction from DEM 1

Knowledge on the elevation of dike heights constitutes a major challenge for large-scale flood risk simulations, as data on flood protection standards are scarce, and impacts have been shown to be the most sensitive to variations in adaptation strategies (Hinkel et al., 2014; Vousdoukas et al., 2016; Scussolini et al., 2016; Hinkel et al., 2021). Consequently, the validation of dike heights in DEMs used to simulate coastal

flooding is crucial for assessing the validity of modelled flood extents.

Here we show that the use of high-resolution $(1 \text{ m} \times 1 \text{ m})$ DEMs for extracting dike heights can still lead to deviations between modelled (DEM) and measured (RTK) dike crest elevations. However, we attribute the great majority of these deviations to issues related to the positioning of the raster cells, which were extracted within a 25 m buffer around the dike line (see Sect. 3.6.2). The comparison of $50 \text{ m} \times 50 \text{ m}$ cells around curvy dike lines with RTK points can lead to positional errors, where cells overlaying the RTK points may represent neighbouring hills or troughs rather than the actual dike line. This is also supported by the fact that the lowest dike heights in the coastal inundation model tend to produce higher differences when compared with the RTK measurements (and vice versa) (Fig. A2 in the Appendix).

Given the differences in scale (points vs. $50 \,\text{m} \times 50 \,\text{m}$ cells), we find that the RTK measurements and the dike

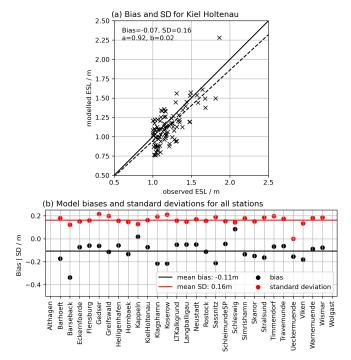


Figure 3. Validation of the hydrodynamic model's extreme sea levels exceeding 1 m height above mean sea level. (a) Comparison of the ESLs for the Kiel Holtenau tide gauge (10) in terms of bias and standard deviation. The dashed line shows a linear fit where the slope denotes whether the model's bias is depending on the height of the ESL. (b) Summary of the biases and standard deviations for all stations. The mean bias along all stations is -11 cm, indicating that in general the ESLs are underestimated by the model. Note that no water levels above 1 m are observed for the Althagen (0) and Wolgast (29) tide gauges.

heights used in the coastal inundation model are generally in good agreement (Fig. A3 in the Appendix). The minimum (-2 m) and maximum (3.7 m) deviations can be substantial, while the root mean square error between both datasets is 0.65 m and the mean absolute error 0.37 m. However, 62% of the values lie in the range between -0.2 m and +0.2 difference and 77% between -0.5 and +0.5 m difference.

4.3 Validation of maximum sea levels during the 2 January 2019 event

Comparing the peak water levels of the coastal ocean model with the observed maxima of the 2019 event (Fig. 6) shows that the model can capture the spatial pattern of water-level variability. Overall, the model underestimates peak water levels by 5 cm on average (more for the higher water levels), yet overestimates the water levels in most lagoons, as already stated in Sect. 4.1. The root mean square error between modelled and measured peak water levels is 15 cm.

4.4 Comparison of simulated coastal flood extents with SAR-derived imagery

We used the flood extent derived from S-1 SAR imagery (acquired at 17:08 UTC on 2 January 2019) to evaluate the validity of the flood extents simulated with the coastal inundation model. The suitability of SAR imagery for validating coastal flood extents has previously been described in Eilander et al. (2023). SAR imagery was acquired 1.5 to 3 h after the modelled peak of the surge in Schleimünde (18) (entrance of the ca. 41 km long subglacial meltwater channel called the Schlei, which is located between the Baltic Sea and the city of Schleswig) and Timmendorf (23) (north of Lübeck) and 3.5 h after water levels were peaking in the Kiel fjord. Only at the end of the Schlei, in Schleswig (19), did the modelled peak of the surge occur at 23:15 UTC, more than 6 h after the SAR imagery was taken. When the SAR image was acquired, the water level at Schleswig (19) was at 0.64 m, which is 0.35 m below the peak water level of the 2019 event at the same location (0.99 m) (see also Fig. A4 of the Appendix for water-level time series of the 2019 event for selected boundary stations).

The comparison of flood extents derived from SAR imagery and hydrodynamic models is compromised by several issues. Mapping surface water from SAR imagery is based on contrast, resulting from low return signals due to specular reflectance of radar pulses at the water surface. Consequently, problems may arise if this assumption is compromised. This may be the case if other surface types result in similar weak backscatter signals (e.g. smooth tarmac, dry sandy soils, or wet snow), which may result in overestimating surface water areas. On the contrary, relatively high backscatter of surface water areas (e.g. caused by emerged vegetation or roughening of the water surface due to wind or rain) reduces the contrast between water and adjacent terrestrial surfaces, typically resulting in an underestimation of surface water area (Chini et al., 2021). In another comparison, Mason et al. (2009) found that the similar signal of unflooded short vegetation and adjacent floodplains was a main error in waterline positioning. Strong winds accompanying the 2019 event may likely have caused a softening of the contrast between open water and other surfaces with similar backscattering characteristics, which might explain some of the differences between the flood extents derived from the SAR imagery and the coastal inundation model. Another source of uncertainty may arise from the operation of small-scale flood gates in the study region. For instance, the closing of flood gates may have resulted in the accumulation of riverine water and corresponding inundation inland, while the opening of flood gates to promote inundation of certain areas during surges (e.g. for reasons of nature conservation) may have resulted in flooding of areas protected by hard defences, which would not have been considered by the hydrodynamic model.

In addition, the pre-processing of the elevation data (within a 100 m buffer around the coastline, each 50 m

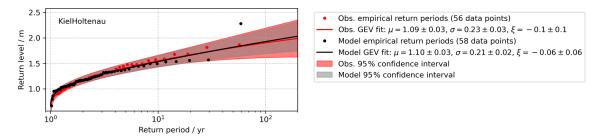


Figure 4. GEV distributions derived from the observations (red) and the coastal ocean model (black) for the Kiel Holtenau (10) tide gauge.

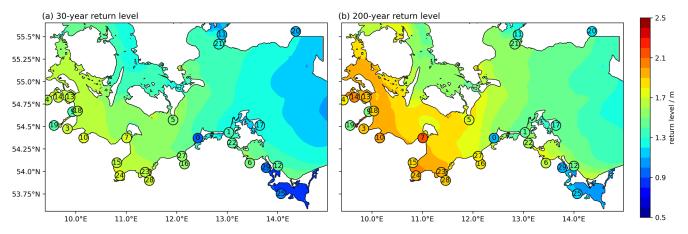


Figure 5. Modelled 30-year (a) and 200-year return levels (b) for the western Baltic Sea. The circles denote the tide gauge locations and the values correspond to those listed in Tables 3 and 4. Note that the values are overestimated for sheltered lagoons and semi-enclosed water bodies with narrow inlets such as the Schlei (located between the Baltic Sea and the city of Schleswig (19); see Figs. 1, 7) and the Saaler Bodden (Althagen, 0) because of the limited resolution (Table 4). Note that some of the tide gauges listed in Tables 3 and 4 are located north of the domain shown.

grid cell is given the maximum elevation identified in the $1\,\mathrm{m} \times 1\,\mathrm{m}$ DEM) results in a general overestimation of nearshore elevations, and 50 m resolution is often too coarse to accurately resolve many narrow beaches in the study region. We must note, therefore, that we do not consider the comparison of SAR-derived inundation maps and the simulated coastal flood extents of the coastal inundation model as validation in the strict sense of the term but rather as a first-order evaluation of the model's performance.

Without accounting for the uncertainties mentioned above when validating flood extents on large spatial scales, as was done for this study, the resulting skill indices can leave a misleading impression on model performance. Therefore, we tried to account for these uncertainties by adjusting the SAR-derived and modelled flood extents prior to comparison. The flooded area was calculated as follows: first, we excluded all flooded areas inside the 100 m coastline buffer in the SAR-derived and coastal inundation model floodplains. In addition, beach lakes and lagoons cut off from the Baltic Sea by sluice gates or beach ridges (indicated by the ATKIS® digital landscape model as stagnant water) were also excluded. We compared the flooded area of both datasets (SAR and the coastal inundation model) by calculating the percent-

age of correctly predicted flood extent (agreement in inundated area between the SAR imagery and coastal inundation model), missed flood extent (inundation observed in the SAR imagery but not in the coastal inundation model), and overpredicted flood extent (inundation simulated with the coastal inundation model but not observed in the SAR imagery) (Fig. 7). These indices are based on the indices commonly used in the current literature to estimate the validity of the output of hydrodynamic models (Vousdoukas et al., 2016; Alfieri et al., 2014). For the area covered by the SAR image (Fig. 7), the storm surge from 2 January 2019 produced a (corrected) flood extent of 2.18 and 2.38 km² for the SAR imagery and the coastal inundation model, respectively. Relative to the SAR data, we calculated that 50 % of the inundated area was correctly predicted, 50 % was overpredicted, and 50 % was missed.

Problematically, the correction of the flood extent prior to comparison with the SAR imagery within a 100 m buffer around the coastline leads to a substantial reduction in the flooded area along peninsulas, such as the flood-prone sand spits of the study region (Fig. 7b vs. c). This, again, negatively biases the skill indices presented above.

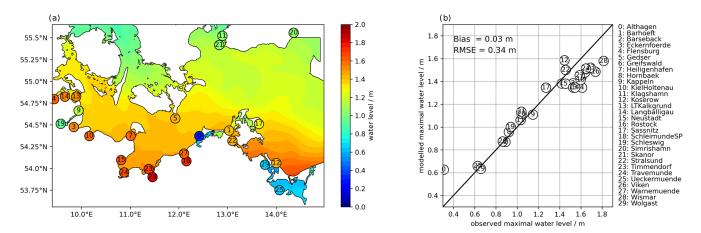


Figure 6. (a) Modelled and observed maximum water levels (coloured circles) for the storm event of 2 January 2019 across the study region. (b) Direct comparison of the observed maximum water levels with the modelled maximum water levels for the 30 tide gauges. The mean bias of the model to the tide gauges is -5 cm; i.e. on average the model underestimates the maximum water levels. Note that some of the stations listed in Tables 3 and 4 and are north of the domain shown.

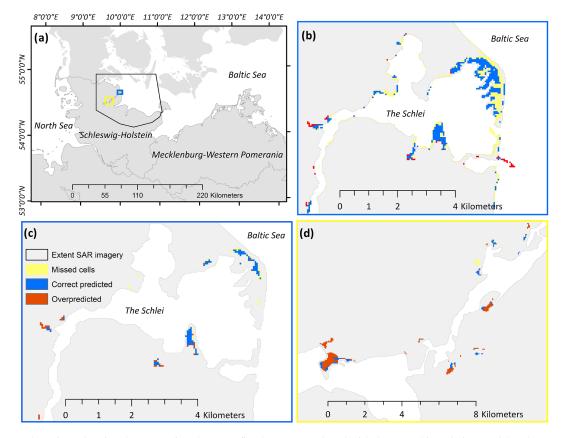


Figure 7. Example regions showing the comparison between flood extents produced with the coastal inundation model and extracted from the SAR imagery for the storm surge of 2 January 2019. (a) Overview map showing locations of example regions and spatial coverage of SAR imagery. (b) Comparison of flood extents prior to any of the corrections described above. (c) Comparison of flood extents after correction for the same region as depicted in (b). (d) Comparison of flood extents for another section of the Schlei, near the city of Schleswig. The skill indices provided above are based on the corrected flood extents for the whole coverage of the SAR imagery (a), as shown in (c) and (d).

In the absence of measured in situ data on coastal inundation characteristics during storm surges, we propose the use of remote sensing products as a promising alternative for the validation of hydrodynamic models. Yet, the validation of SAR-derived flood maps is challenging. The use of remote sensing products for flood extent validation may be better suited for smaller, connected floodplains, where the above limitations can be more easily addressed. The strong potential for such applications has previously been shown by Eilander et al. (2023) and Vousdoukas et al. (2016). We show that the focus on specific areas with connected flood extents would also result in a higher agreement between the SAR imagery and our simulation (Fig. 7b).

A more general challenge for the comparison of satellitederived flood extents with hydrodynamic model results is the generally short duration of ESL. Even if the latter is already quite long-lasting along the Baltic Sea coast compared to macrotidal environments (Wolski and Wiśniewski, 2020; MacPherson et al., 2019), the limited duration reduces the chance of suitable matches with satellite observations. Nevertheless, the growing number of satellite missions may improve the availability of relevant observations in the future.

We believe that incorporating satellite-derived flood extents in the portfolio of potential validation data may increase the opportunities to validate hydrodynamic flood models. However, such products are still dependent on algorithm development and limited by spatio-temporal coverage.

4.5 Flood characteristics of simulated storm surge scenarios

Our results confirm that the German Baltic Sea coast is exposed to coastal flooding (Fig. 8, Table 5). Without upgrading existing coastal protection infrastructure until 2100, the flood extent of a 200-year event can increase by almost a factor of 5 when SLR reaches 1.5 m. For the four storm surge scenarios, the majority of the flooded area is located in the federal state of MP, varying between 85 % and 89 % (Table 5).

In contrast to the flood extent, the average maximum inundation depths for the 2019 surge and the 200-year event are considerably higher in SH compared with MP (Table 5). The largest differences are found for the 200-year event, where water depths are 40 cm higher in SH. The differences converge under the influence of SLR. For both SLR scenarios, inundation depths are higher in MP. The analysis of flood depths during storm surges can be crucial, as it constitutes a major driver of potential flood damages to buildings and infrastructure (Merz et al., 2010; de Moel and Aerts, 2011).

Following Lopes et al. (2022), we show that the flood depth and extent along the German Baltic Sea coast are highly dependent on ESL and local geomorphological features, which can also explain the observed differences between the federal states of SH and MP. For instance, the 200-year event is characterised by peak water levels that are on average 0.46 m lower in MP compared with SH (2.03 m in

SH and 1.57 m in MP, Table 4). In both SLR scenarios, the flood depth is slightly higher in MP, which we attribute to lower elevations within the floodplain. Within the flood extent of the 200-year event with 1.5 m SLR, the mean elevation is 1.1 m NHN in MP compared with 1.27 m NHN in SH.

The importance of geomorphology for inundation patterns in the study region also becomes evident when comparing flood extents between the federal states of SH and MP (Table 5). Despite lower peak water levels during the 2019 event and the 200-year event without SLR, MP contains much larger flood extents, which is not exclusively due to the fact that MP comprises a substantially longer coastline and thus the potential for larger floodplains. In SH, the flood extent per kilometre of coastline varies between 0.03 km² km⁻¹ for the 2019 surge and 0.23 km² km⁻¹ for the 200-year event with 1.5 m SLR. In contrast, the normalised coastal length flood extent in MP varies between 0.05 km² km⁻¹ for the 2019 event and 0.46 km² km⁻¹ for the highest surge scenario.

We identify several hotspots of coastal flooding along the German Baltic Sea coast. Overall, the majority of the flood extent is located along sheltered lagoons and estuaries. In SH, hotspots are found along the Flensburg fjord, the Schlei, Eckernförde (domain 1, counted from left to right in Fig. 1), Fehmarn (domain 3), and along the Trave estuary until the city of Lübeck (domain 4). In MP, the largest flood extents are identified in Rostock-Warnemünde (domain 6), Fischland-Darß-Zingst (domain 7), western and central Rügen (domains 8 and 9), north of Greifswald (domain 10), the island of Usedom, and the Peene estuary located in the south of the eastern most model domain (domain 11). Across both federal states, the largest coherent flood extents are observed in MP and comprise the lagoons of Fischland-Darß-Zingst, Rügen, and Usedom (Fig. 8).

4.6 Assessing the validity of the model results

We find that the coastal ocean model, which provides the boundary conditions for the coastal inundation model, overestimates both the extrapolated ESL (the modelled 200-year return water level) and the peak water levels of the 2019 surge inside protected lagoons of Fischland-Darß-Zingst in MP and the Schlei in SH (Figs. 1, 6; Table 4). We therefore use ESL extrapolations from the tide gauges Althagen (0), Kappeln (9), and Schleswig (19) as input for the coastal inundation model. We note that this change is only valid for this specific application and should not be considered part of the modelling framework. Thus, the provision of boundary conditions from coarser-resolution hydrodynamic models for sheltered coastal environments constitutes an extra source of uncertainty in large-scale coastal flood risk assessments that should be accounted for in future applications.

Only a few studies have examined the impact of storm surge duration and intensity on flood characteristics, but Höffken et al. (2020) have shown for a case study in the German Baltic Sea that flood extents can vary by 20 % when sea levels

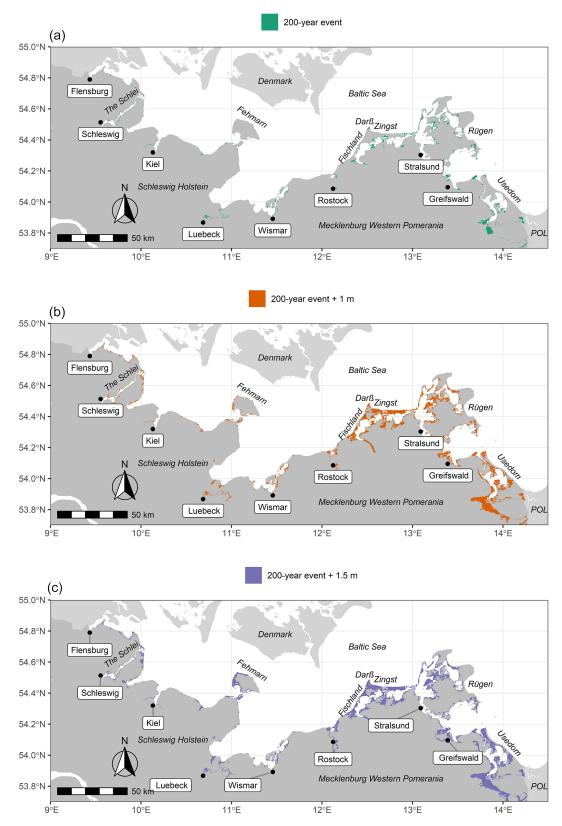


Figure 8. Flood extent for a storm surge with a return period of 200 years (a), the same surge with 1 m SLR (b), and the same surge with 1.5 m SLR (c).

		MP	SH	Total
Storm surge 2019	Flood extent (km ²)	101.26	16.44	117.61
	Average max flood depth (m)	0.59	0.9	0.63
200-year event	Flood extent (km ²)	191.54	25.31	216.84
	Average max flood depth (m)	0.63	1.03	0.67
200-year event + 1 m	Flood extent (km ²)	673.33	79.77	753.09
	Average max flood depth (m)	1.31	1.25	1.31
200-year event + 1.5 m	Flood extent (km ²)	868.48	147.62	1016.10
	Average max flood depth (m)	1.64	1.5	1.62

Table 5. Flood characteristics for the four storm surge scenarios for MP, SH, and the entire German Baltic Sea coast.

rise. The Baltic Sea is characterised by a microtidal regime, which means that high water levels during storm surges can remain for several days, and various storm surge intensities are observed (MacPherson et al., 2019). Consequently, storm surge hydrographs are spatially and temporally (between different storm surge events) variable (also compare Fig. 2a). While we account for the spatial variability by calculating the mean storm surge hydrographs for 32 flood boundary stations (Fig. 1) across the study region, the temporal variability is not accounted for as we apply mean surge shapes. We note, therefore, that the flood extents shown in this study can be both larger or smaller depending on the intensity and duration of the surge.

We consider the flood maps presented here to be conservative for several reasons. First, the exclusion of waves in coastal flood modelling can lead to underestimations of flood depth and extent. A long series of breaking waves can substantially increase peak water levels due to wave setup and swash (also referred to as wave run-up) (Weisse et al., 2021; Melet et al., 2018). For example, in a study from the Gulf of Finland, the contribution of wave setup to extreme water levels approached up to 50 %, while the maximum absolute contribution to peak water levels varied between 70 and 80 cm in exposed areas (Soomere et al., 2013). Although these values should be lower in the German Baltic Sea, where recorded maximum significant wave heights are considerably lower than in the Gulf of Finland (Alari, 2013), waves can affect coastal flood extents. This has also been shown on a pan-European scale (Vousdoukas et al., 2016). We still excluded wave setup in our analysis. A technical reason is that the coastal ocean model has a resolution of 200 m and thus cannot resolve the nearshore (wave-breaking zone) sufficiently to reproduce wave setup. In addition, we calibrated the coastal ocean model by increasing the wind speed by 7%. This allowed us to minimise the error in predicting the peak water levels, and, using this method, we consider missing processes such as wave formation, lack of resolution, and errors in the atmospheric model. Yet, the reason for the underestimation of high events is not entirely clear (Fig. 3). Despite the 7% increase in wind speed, the underestimation may be partially explained by the missing process of wave setup. Another reason could be that the few storms that were responsible for the underestimated surges are not reproduced well by the atmospheric data (Lorenz and Gräwe, 2023). Nevertheless, the model GEV distribution overestimates the 200-year return levels as compared with the tide gauges. Thus, we expect the uncertainty in wave setup on the flood maps to be small, especially for the 200-year cases with and without SLR.

In addition, there is still no conclusive information on potential future changes in the wave climate, and existing results show strong spatio-temporal variability (Weisse et al., 2021).

Another reason why we believe our model results can be considered conservative is that we overestimate dike heights in the coastal inundation model. The representation of subgrid-scale coastal adaptation measures such as dikes in coastal flood modelling constitutes a major challenge that can affect simulated flood extents (Hinkel et al., 2014, 2021; Vousdoukas et al., 2016). We have shown that dike crest elevations used in the coastal inundation model are generally in good agreement with high-accuracy RTK measurements (see Sect. 4.2). However, there are cells that deviate substantially from the overlying RTK points, challenging the validity of the flood extents presented here. In order to test the sensitivity of the coastal inundation model to variations in dike crest elevations, we set up the coastal inundation model using the average RTK dike height for each pixel overlaying the dike line. Running the adjusted model for the 200-year event produced a flood extent of 27.8 km², which is 9.8 % higher than in the original setup (compare Table 5). Therefore, we believe that the dike crest elevations used in the coastal inundation model are reliable. However, we must stress that in this study, we had access to high-resolution elevation data, which is very rare in large-scale flood modelling, suggesting that the sensitivity of most flood models to variations in dike crest elevations or elevation models in general is likely to be higher.

Finally, the results presented here do not account for morphological responses to rising water levels, such as the po-

tential for shoreline change, dune collapse, and dike breaching. Dike failure can occur due to hydraulic loads induced by waves and water level (Marijnissen et al., 2021), and thus, flooding behind embankments may not exclusively occur due to wave overtopping or overflow. Large-scale dike overflow is observed particularly for the 200-year event with 1.5 m SLR, as the dike heights in the coastal inundation model represent the status quo (i.e. without potential future increases in dike height). However, the ongoing implementation of socalled climate dikes in the study region allows the dike elevation to be increased by up to 1.5 m (Melund, 2022). Further adjustments even allow an increase in dike heights by up to 2 m (Hofstede and Hamann, 2022). Ignoring the potential of dike failure, we therefore expect that the increase in dike heights by 1.5 m could offset a fraction of the additional flood extent caused by the SLR scenario. On the other hand, the increase in dike heights will be very costly (although probably cheaper than the expected flood damage costs (Hinkel et al., 2014)) and may not be applicable to many regional dikes that are characterised by variable safety standards (Melund, 2022). Therefore, and in the light of the findings presented here, we agree that the development and identification of new and complementary measures to mitigate increasing coastal flood risks constitute one of the most prominent challenges facing coastal communities today (Morris et al., 2018).

5 Conclusions

In this study, we show that the current design heights of dikes along the German Baltic Sea coast are not sufficient to prevent flooding during a storm surge with a 200-year return period under high-sea-level-rise scenarios. Hotspots of coastal flooding are mainly located in the federal state of Mecklenburg-Western Pomerania, where the lagoons of Fischland-Darß-Zingst, Rügen, and south of Usedom (Szczecin lagoon) are particularly exposed (Fig. 8). With foresight, the state authorities in Schleswig-Holstein and Mecklenburg-Western Pomerania have initiated the upgrade of state dike heights until the end of the century, which will allow comparatively easy increases of up to 1.5 m (Melund, 2022; Hofstede and Hamann, 2022; StALU, 2012). However, the effectiveness of dike height increases to compensate for high-SLR scenarios has not been demonstrated yet. In addition, the associated costs as well as the future of many regional dikes with variable design heights (Melund, 2022) remain uncertain. Some regional dikes may become the responsibility of the federal states, but their maintenance and the increase in height required to compensate for high-SLR scenarios (such as 1.5 m) may foster the need to rethink contemporary coastal protection measures towards new, more nature-based solutions.

In line with previous studies, we find that model validation (of all model components used) remains one of the biggest problems in large-scale flood modelling. With respect to the validation of flood extents, we show that besides the often used vertical aerial photography, other remote sensing products such as SAR imagery may provide a promising alternative. Currently available spatio-temporal resolutions and problems associated with the detection of surge-driven coastal flooding versus wet soils, e.g. as a consequence of rainfall, compromise current applications, particularly on large spatial scales as presented in this study. The growing number of satellite missions may improve the availability of suitable observations in the future.

We suggest future research in the region to improve our understanding of (1) potential future changes in wave climate and associated impacts on coastal flood extents, (2) morphodynamic responses of natural and anthropogenic flood barriers to high water levels and wave loading with a particular focus on dike breaching, and (3) water-level dynamics and the vulnerability of low-lying sheltered lagoons and inlets to storm surges and SLR.

Appendix A

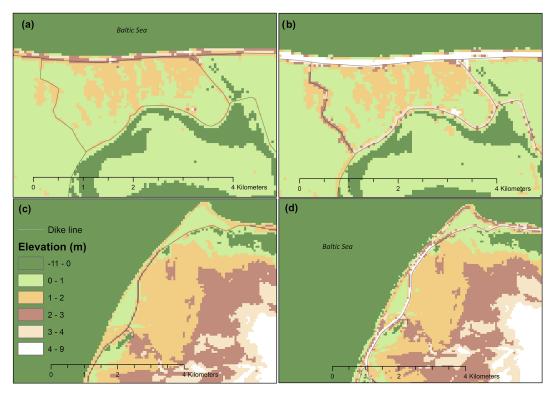


Figure A1. The difference between a DEM that was corrected for dikes (**b**, **d**) and an uncorrected DEM (**a**, **c**). Panels (**a**) and (**b**) depict the area and dikes around Zingst (MP), and panels (**c**) and (**d**) show the northwestern coastline of the island of Fehmarn (SH).

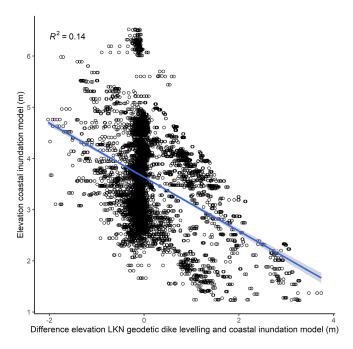


Figure A2. Correlation between the dike heights of the coastal inundation model and the difference between dike heights derived from the geodetic dike levelling (LKN) and dike heights of the coastal inundation model. The blue line represents the linear fit.

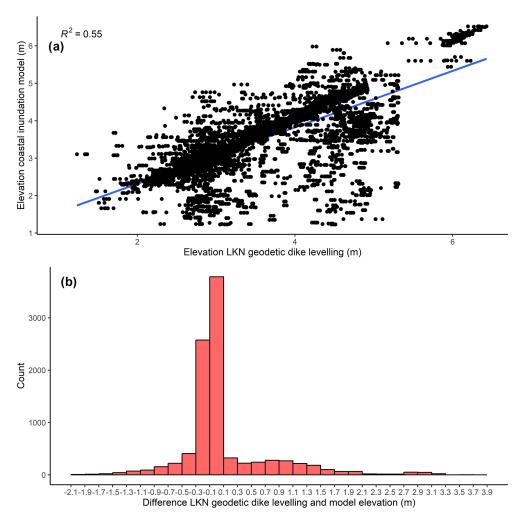


Figure A3. (a) Correlation between the dike heights of the coastal inundation model and the dike heights derived from the geodetic dike levelling (LKN). (b) Histogram showing the distribution of the error between the dike heights of the geodetic dike levelling and the coastal inundation model.

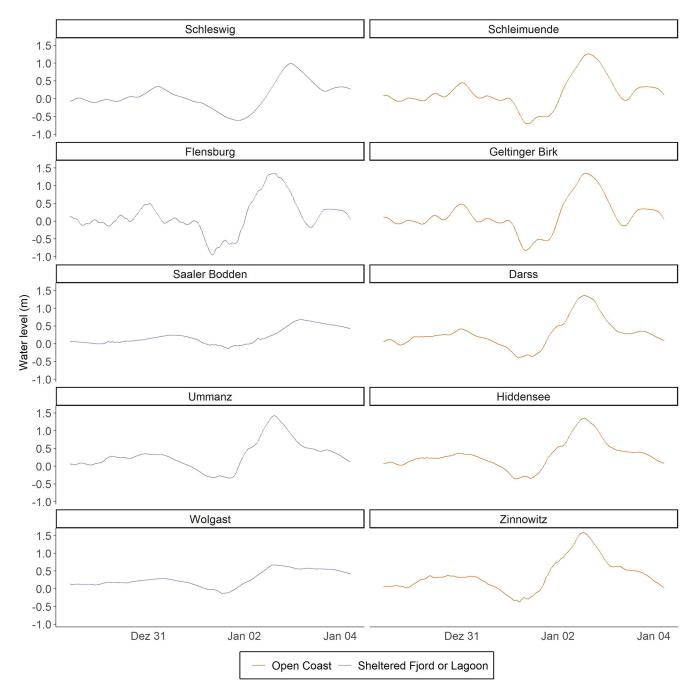


Figure A4. Water-level time series as simulated with the coastal ocean model for selected boundary stations during the January 2019 storm surge.

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Table A1. Reclassification scheme of Corine land cover classes.

Corine land class	New land class
Continuous urban fabric	Urban
Discontinuous urban fabric	Urban
Industrial or commercial units	Urban
Road and railway networks and associated land	Traffic
Port areas	Urban
Airports	Urban
Mineral extraction sites	Urban
Dump sites	Urban
Construction sites	Urban
Green urban areas	Green urban areas
Sport and leisure facilities	Green urban areas
Non-irrigated arable land	Agriculture
Fruit tree and berry plantation	Agriculture
Pasture	Agriculture
Complex cultivation patterns	Agriculture
Land principally occupied by agriculture,	Agriculture
with significant areas of natural vegetation	
Broadleaf forest	Forest
Coniferous forest	Forest
Mixed forest	Forest
Natural grassland	Natural grassland
Moors and heathland	Wetland
Transitional woodland-shrub	Natural grassland
Beaches, dunes, and sand	Unvegetated coastal sediment
Sparsely vegetated areas	Natural grassland
Inland marshes	Wetland
Peat bog	Wetland
Salt marshes	Wetland
Intertidal flats	Unvegetated coastal sediment
Water courses	Inland waterbodies/courses
Water bodies	Inland waterbodies/courses
Coastal lagoons	Inland waterbodies/courses
Estuaries	Inland waterbodies/courses
Sea and ocean	Sea and ocean

Table A2. Flood characteristics for varying configurations of Manning's n coefficients.

	Domain 1		Domain 7		Domain 8	
Manning setup	Flood extent (km ²)	Mean flood depth (m)	Flood extent (km ²)	Mean flood depth	Flood extent (km ²)	Mean flood depth (m)
Low	5.78	0.47	24.36	0.39	33.21	0.58
High	5.69	0.46	22.04	0.37	32.17	0.57
Land/water	5.79	0.47	24.12	0.38	33.4	0.58
Uniform	5.78	0.45	23.77	0.38	33.13	0.58
Moderate	5.73	0.46	22.84	0.38	32.58	0.58

Data availability. The flood boundary stations, associated water-level time series representing the four storm surge scenarios (the 2019 surge, 200-year event, 200-year event plus 1 m SLR, and 200-year event plus 1.5 m SLR), simulated flood characteristics (flood extent and depth), spatially explicit results of the extreme value analysis for every grid cell of the coastal ocean model, modelled monthly peak water levels between 1961 and 2018 for every grid cell of the coastal ocean model, and modelled time series of water levels during the 2019 storm surge and for the entire hindcast period are freely available from Kiesel et al. (2023) (https://doi.org/10.5281/zenodo.7886455).

Author contributions. JK, ATV, UG, and ML designed the concept of the research. JK and ML set up the methods, ran the hydrodynamic models, and analysed and plotted the results. MK processed and analysed the SAR imagery. JK wrote the manuscript with contributions from ML and MK. ATV, UG, ML, and MK reviewed and edited the manuscript.

Competing interests. The contact author has declared that none of the authors has any competing interests.

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