Nat. Hazards Earth Syst. Sci., 22, 3005–3014, 2022 https://doi.org/10.5194/nhess-22-3005-2022 © Author(s) 2022. This work is distributed under the Creative Commons Attribution 4.0 License.



Brief communication: Impact forecasting could substantially improve the emergency management of deadly floods: case study July 2021 floods in Germany

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Received: 26 January 2022 – Discussion started: 2 February 2022 Revised: 16 June 2022 – Accepted: 15 August 2022 – Published: 15 September 2022

Abstract. Floods affect more people than any other natural hazard; thus flood warning and disaster management are of utmost importance. However, the operational hydrological forecasts do not provide information about affected areas and impact but only discharge and water levels at gauges. We show that a simple hydrodynamic model operating with readily available data is able to provide highly localized information on the expected flood extent and impacts, with simulation times enabling operational flood warning. We demonstrate that such an impact forecast would have indicated the deadly potential of the 2021 flood in western Germany with sufficient lead time.

1 Introduction

River flooding directly affects, on average, 125 million people annually, by evacuation, homelessness, injury, or death (Douben, 2006), and flood exposure and losses are projected to increase owing to climate change and population and socio-economic growth (Dottori et al., 2018). Forecasting and early warning are essential cornerstones of disaster risk reduction as anchored in the Sendai Framework for Disaster Risk Reduction (UNDRR, 2019). However, the official and legally binding operational river flood forecasts in Germany, operated be the different federal states, provide only expected water levels or discharges at specific river gauges. The same holds true for the Global Flood Awareness System (GloFAS), developed by the European Commission and the European Centre for Medium-Range Weather Forecasts (ECMWF). The European Flood Alert System (EFAS; https://www.efas.eu, last access: 30 January 2022) provides warnings with spatial information, but these forecasts have a rather coarse spatial resolution of 100 m, do not consider dikes or other flood protection measures, and are based on pre-calculated hazard maps, not actual flood dynamics. Local decision-makers, disaster managers, and potentially affected citizens need more detailed flood information for emergency management decisions. Examples are the decision to issue a disaster alert, to evacuate an urban area, to strengthen levees that may breach, to protect the most critical infrastructure objects, or to allocate emergency resources to expected damage hotspots. Potentially affected people need to know whether there may be danger to their health and lives, whether their houses and assets may be at risk of flooding or even destruction, and how much time they have to save their lives and reduce damage to their assets. The approach proposed in this study can provide this information.

Impact-based forecasting has recently gained attention in disaster risk research (Merz et al., 2020; Taylor et al., 2018; Zhang et al., 2019). It aims at extending the forecast to include event impacts, such as the number and location of affected people and buildings, damage to buildings and infrastructure, or disruption of services. When obtaining specific and spatially resolved information on the expected event impact, as well as behavioural recommendations on what to do, people tend to be more motivated to accept warnings and to respond in a more effective way (Kreibich et al., 2021; Weyrich et al., 2018). River flood forecasting systems, with lead times of several hours to days, are operational in many countries (e.g. Emerton et al., 2016). Flood warnings are commonly issued when given thresholds in terms of river water level or streamflow are exceeded. River flood impact forecasting systems have recently been proposed by Bachmann et al. (2016), Brown et al. (2016), and Dottori et al. (2017). One of the main challenges is the provision of timely and accurate estimates of inundation characteristics (Merz et al., 2020). To circumvent this simulation challenge, pre-defined relationships between river peak discharges and expected impacts (Dale et al., 2016) and hydrodynamic surrogate model based on machine learning have been proposed (Hofmann and Schüttrumpf, 2020, 2021).

Germany, the Netherlands, and Belgium were hit by an extreme rainfall event in July 2021 leading to record-breaking peak flows at many gauges with estimated damage in the order of EUR 30 billion for Germany alone. Out of the 184 fatalities in Germany, 133 occurred along the river Ahr – a Rhine tributary (https://de.wikipedia.org/wiki/Hochwasser_ in_West-_und_Mitteleuropa_2021, last access: 30 January 2022). Here, we show that a simple and rapid hydrodynamic flood inundation model could extend the current hydrological forecasting systems by spatially explicit information on inundation areas, depths, and flow velocities based on the forecasted gauge discharges or water levels. In that way, critical locations for life threatening flow conditions, for vehicle instabilities, and structural failure of buildings and infrastructure could be derived from the inundation and flow velocity maps.

These maps could provide valuable and much more concrete information about the severity and the impact of the foreseen flood event, which can be used for a more targeted disaster management. They can also assist in better warning and response recommendations for the population and thus help to reduce damage and particularly fatalities. We show that such an impact forecasting can be performed using a hydrodynamic model that is easily set up based on readily available data and has model runtimes that allow the application in operational flood forecasting and warning systems.

2 Hydrodynamic model

We implemented the hydrodynamic model RIM2D for the river Ahr (Fig. 1) in a hindcast setting for the July 2021 flood. RIM2D is a 2D raster-based model solving a simplified version of the shallow water equation, the so-called "local inertial approximation" of Bates et al. (2010). The approach has been used in a large number of applications of fluvial flood-plain inundation and has been proven to provide realistic flow simulations (e.g. Falter et al., 2016; Neal et al., 2011). We selected this approach and implemented it in RIM2D because the 2D raster-based concept using simplified shallow water equations still provides the best compromise between

required accuracy, model complexity, and model runtime, as Bates (2022) states in a recent review on models for flood prediction. As the original solution by Bates et al. (2010) is prone to instabilities for small grid cell sizes and under near-critical to super-critical flow conditions (de Almeida and Bates, 2013) often occurring during flash floods, the numerical diffusion as proposed by Almeida et al. (2012) was additionally implemented. This comes, however, at the cost of underestimated flow velocities (de Almeida and Bates, 2013). This effect becomes more pronounced with increasing Froude numbers. For low dynamic fluvial floodplain inundation events with flow in the subcritical range, these effects are negligible. Under flash flood conditions these limitations need to be considered when interpreting the inundation results. Simulated flow dynamics and velocities should be considered a low boundary estimation, with chances of higher velocities in reality (Shaw et al., 2021).

RIM2D implements the same numerical core as the wellknown model LISFLOOD-FP (de Almeida and Bates, 2013) but is coded in CUDA Fortran and implemented to run on large NVIDIA Tesla graphical processor units (GPUs). This enables massive parallelization of the numerical computations at low costs compared to large multi-core computing clusters.

3 Data requirements

RIM2D operates directly on spatial raster data in the format of Esri ASCII raster. The core information is a digital elevation model (DEM) of a resolution suitable for the model domain and hydraulic situation to be simulated. For the selected model domain of the reach of the river Ahr from the towns of Altenahr to Sinzig (i.e. to the inflow to the river Rhine, overall about 30 river kilometres), a resolution of 10 m was selected. This resulted in a raster grid with 675 rows and 2092 columns with overall 1412100 grid cells. The DEM was obtained from the German Federal Agency for Cartography and Geodesy (BKG). The DEM was directly used as the basis for the flow simulation without any further modifications. This means that the river bed is not realistically represented in the hydrodynamic model. The model river bed is rather a representation of the average water surface of the river, which is in the case of the Ahr typically less than 1 m. This simplification is acceptable for the aim of the model to simulate extreme flows exceeding the average flow depths by far. This approach is also justified by the fact that RIM2D operates with water levels and not water depths or discharges as boundary conditions. Due to this, the water levels at the model boundary will always be correct, despite the assumed bed elevation, and overbank flow and floodplain inundation will be initiated at the right time and places. The benefit of this simplification is the applicability of the model approach to any river reach without detailed local knowledge of river bathymetry. This enables an easy, semi-automated, and low-



Figure 1. Overview of the Ahr catchment and the simulation domain (river reach Altenahr–Sinzig). (a) Location of the catchment draining to the Altenahr gauge and the simulation domain within Germany. (b) The catchment draining to the Altenahr gauge and the hydrodynamic simulation domain. (c) Boundary water depths used for simulating the flood event of 2021. "Forecast" indicates the forecasted water depth hydrograph issued by the Landesumweltamt Rheinland-Pfalz, and "reconstructed" indicates the preliminary (in January 2022) reconstructed water depth hydrograph at the Altenahr gauge of the event. The hydrographs start on 14 July and the time indicated at the time axis. Data sources: DEM 10 m resolution: © GeoBasis-DE/BKG 2012 (https://www.bkg.bund.de/, last access: 30 January 2022); OSM river network and buildings: © OpenStreetMap contributors 2021. Distributed under the Open Data Commons Open Database License (ODbL) v1.0.

cost application and transfer of the model approach to practically everywhere where a DEM with sufficient resolution in relation to the river width is available.

RIM2D requires a hydraulic roughness parameterization. This was derived from the CORINE land use classification from the European Environmental Agency, which is openly available for the whole of Europe. The raster data set was reclassified to three classes: built-up/sealed areas, forest, and all other land use classes (farmland, pastures, water bodies, etc.). These classes were assigned with typical Manning's roughness values from the literature: sealed surface areas: n = 0.02 (for simulating flow over tarmac or concrete in the built-up areas); forest: n = 0.2; all other classes including the river channel and floodplains: n = 0.03. The resulting spatially distributed roughness values were resampled to a

raster with the same dimension and resolution as the DEM. The validity of this classification approach has been shown in large-scale applications of LISFLOOD-FP (Bates, 2022; Wing et al., 2021; Shaw et al., 2021; Bates et al., 2010, 2021; de Almeida et al., 2018; Savage et al., 2016; Stephens and Bates, 2015; de Almeida and Bates, 2013; Almeida et al., 2012) and the hinterland inundation module of the regional flood model (RFM) (Vorogushyn et al., 2021); Falter et al., 2015; Sairam et al., 2021; Farrag et al., 2022), i.e. in studies where detailed roughness calibration is not possible due to the size of the model domain and missing calibration data.

To simulate realistically the flow around buildings and in urban settings, the locations of buildings were extracted from the OpenStreetMap (OSM) building layer. The vector shape file was rasterized to a grid with the same resolution and extent as the DEM. The raster cells of the DEM where the building raster indicates a building are excluded from the hydraulic routing; thus flow around buildings is simulated. This means that the model simulates the flow in the streets explicitly, and an appropriate low roughness was selected for the built-up areas. This is in contrast to modelling approaches not considering the buildings as obstacles, which thus require a high roughness for the built-up area in order to compensate for the model simplification ("urban porosity approach") (Neelz and Pender, 2007).

Initial water depths were derived by a steady-state simulation prescribing a fixed water level at the inflow of the river channel into the modelling domain, with free outflow, i.e. normal depth, at the lower boundary. The simulation was continued until a constant water profile along the river reach was established. As a consequence of this procedure and the missing bathymetry, only water levels exceeding the water levels of the initial water depths can be simulated. This is, however, acceptable for the purpose of simulating flood flows largely exceeding the average river flow. In order to test the sensitivity of the inundation simulation to the missing river bed, a control simulation was performed, in which the DEM elevation of the initially wet cells, i.e. the river bed in the model, is lowered by the water depths corresponding to the 2-year return period flow at the gauges in the reach. For both gauges this amounts to 0.85 m; thus the elevation of the wet cells was reduced uniformly by 0.85 m. This depth corresponding to the bankfull discharge is a conservative estimate for the channel depth below the water surface not included in the DEM.

As input to RIM2D we used the official water level forecast of the flood warning centre Rhineland-Palatinate at the Altenahr gauge and the reconstruction of the actual water levels in metres above sea level. The reconstruction was necessary because the gauge was destroyed during the event. For the actual fluvial flood simulations these water levels are prescribed to the inflow cells into the domain. These cells were set on the river channel on the boundary of the domain, which is clearly visible in the DEM. In order to account for overbank flow, those cells neighbouring the river channel and with elevations below the maximum water level of the flood hydrograph were additionally selected. Water depths were assigned to those cells only when the river water levels exceeded the DEM elevation. The forecast of the water levels at the Altenahr gauge was issued with a lead time of 24 h before the flood event, with a maximum water depth of 5.74 m and a hydrograph duration of 30 h. In order to validate the simulation results, an additional simulation using the preliminary hydrograph (in January 2022) of the flood event at the Altenahr gauge reconstructed by the Landesumweltamt Rheinland-Pfalz (LfU¹) was performed. This shows a peak water depth of 10.2 m (Fig. 1c), i.e. 4.46 m higher than the forecast. The large difference between the forecast and the

¹Environmental Office of the federal state Rhineland-Palatinate.

reconstruction is not only caused by an underestimation of the flow by the hydrological forecast but to a large extent also by clogging of bridges, one of which is directly located downstream of the Altenahr gauge.

4 Results and discussion

Figure 2 shows the simulated maximum water depths for the flood forecast hydrograph and the reconstructed hydrograph as inflow boundary. Both maps show large inundation areas, particularly in the towns and villages situated in the flood-plains alongside the river Ahr. The inundation depths and extent of the reconstructed scenario are, however, much higher than those from the forecast because of the higher peak water level. This is illustrated by the inundation depths for the heavily affected commune of Ahrweiler (inset of Fig. 2b).

The model performance was validated using the post-event mapping of the inundated areas by the LfU, which were compared to the simulated inundation area based on the reconstructed hydrograph (Fig. 2b). A high agreement of the simulation with the maps can be observed, supporting confidence in the simulation results. The binary pattern comparison metric $F^{(2)}$ as proposed by Aronica et al. (2002), also termed critical success index or threat score (Horritt et al., 2007; Sampson et al., 2015; Lim and Brandt, 2019), is evaluated to 0.845, which is a very high performance value for hydrodynamic inundation simulations. Furthermore, water depths derived from 75 high water marks at buildings reported by the inhabitants (red dots in Fig. 2b) were used for the evaluation of the simulated water depths. In this context it is noteworthy that the reported water depths refer to different vertical datums, like the street, the pedestrian walk, or the doorstep, which needs to be considered in the evaluation of the comparison. The bias between the reported water depths and the simulated depths is evaluated to -0.39 m, with an RMSE of 0.66 m. Considering the uncertainty in the datum of the reported water depths, the unavoidable simplification of the terrain in the 10 m resolution DEM used, and the uncalibrated simulation, such differences can be excepted. Considering the inundation water depths between 1.5 and 3 m in the area where water marks were recorded, the model performance is considered good and sufficient for an early warning. Almost identical results are obtained with the simulations on a lowered river bed (Sect. S1 in the Supplement). The lowered bed has a noticeable impact on channel water depths and flow velocities but little on the overbank flow and floodplain inundation. The channel capacity is hence comparably small in relation to flood volume. The presented model performance thus supports the validity of the simplified assumptions in the model setup, including the non-consideration of the real river bed bathymetry.

Figure 3a shows the maximum simulated effective flow velocities using the reconstructed flood hydrograph. The effective flow velocities were quantified as the geometric vector

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Figure 2. Maximum inundation depths during the flood in July 2021 simulated with RIM2D using (**a**) the water level forecast for the Altenahr gauge and (**b**) the reconstructed hydrograph of the event as shown in Fig. 1. The green outlined areas indicate the inundation areas mapped by the LfU of Rhineland-Palatinate. Data sources: DEM 10 m resolution: © GeoBasis-DE/BKG 2012 (https://www.bkg.bund.de/, last access: 30 January 2022); OSM roads and buildings: © OpenStreetMap contributors 2021. Distributed under the Open Data Commons Open Database License (ODbL) v1.0.

sum of the flow velocities in the x and y directions of the grid. The simulated flow velocities are plausible for such a dynamic event, ranging from 2 to 5 m s^{-1} in the river course and 0.1 to 2 m s^{-1} in the built-up areas. The model typically simulates increased flow velocities between the buildings, which is plausible for flow in an urban setting (inset in Fig. 3a).

As an example for an additional impact forecast based on the hydraulic simulations, a location-specific indicator for human instability was derived as a product of simulated inundation depths and flow velocities. Jonkman and Penning-Rowsell (2008) reported the critical threshold of moment instability for humans in water flows at $1.32 \text{ m}^2 \text{ s}^{-1}$. Hence, even more detailed warnings could be issued for humans' losing control in flowing water and being carried away with a high risk of drowning. An example is shown in Fig. 3b, in which the maximum value of the product of water depth and flow velocity is used as indicator for human instability. Here, a conservative critical value of $1 \text{ m}^2 \text{ s}^{-1}$ was chosen to account for possible underestimation of the flow velocities caused by the numerical approach of the model (see Sect. 2), but also for differences in person weights and fitness, and to consider variation in the roughness of the ground. Alternative approaches for estimating the risk of human fatalities in floods are published by REDSCAM (2000), Penning-Rowsell et al. (2005), Jonkman et al. (2008), and Milanesi et al. (2015). Additional indicator maps can be derived from the water depth and flow simulations, e.g. for vehicle instability using the approaches of Bocanegra and Francés (2021), Martínez-Gomariz et al. (2019), or Milanesi and Pilotti (2020), or for structural failure of buildings using the approach of Kelman and Spence (2004) or Jansen et al. (2020). These maps can be automatically derived from the simulation results, i.e. could be made available along with the forecast and inundation maps. It is also possible to use the maps of water depths and velocity as input into impact models that combine exposure and vulnerability information. In this way, direct or indirect adverse consequences, such as failure of critical infrastructure or economic loss to build-



Figure 3. Flow velocities and human instability indicator for the flood event in July 2021 using the reconstructed hydrograph: (a) simulated maximum effective flow velocities; (b) areas indicating human moment instability in flowing water derived from the maximum values of the product of water depth and flow velocity. The critical value for human instability was set to $1 \text{ m}^2 \text{ s}^{-1}$, following Jonkman and Penning-Rowsell (2008) and considering potential underestimation of flow velocities by RIM2D. Data sources: DEM 10 m resolution: © GeoBasis-DE/BKG 2012 (https://www.bkg.bund.de/, last access: 30 January 2022); OSM river network and buildings: © OpenStreetMap contributors 2021. Distributed under the Open Data Commons Open Database License (ODbL) v1.0.

ings and infrastructure, could be estimated (Merz et al., 2020; Rözer et al., 2021).

4.1 Computational performance

The 30 h long flood event was simulated in 14 minutes on a NVIDIA Tesla P100 GPU computing unit connected to a Linux server with an Intel Xeon Gold 6140 CPU. This is a simulation runtime equivalent to less than 0.8 % of the simulated event duration. The memory capacity of the GPU unit in terms of computational nodes was used to about 15 % only, leaving room for increasing the model domain or spatial resolution. The achieved simulation runtimes would allow using the model in an operational flood forecast mode. With a lead time of 24 h the simulated inundation areas could be available more than 23 h prior to the event. This would leave sufficient time to include the inundation, flow velocity, and indicator maps in the emergency management and to provide informative and localized warnings to the population.

4.2 Uncertainties

As with any model simulation, there are uncertainties associated with the results. The presented uncalibrated hydrodynamic model will surely add to the already existing uncertainties of the meteorological and hydrological forecasts. However, inferring from previous studies about the uncertainties in flood risk assessments (Apel et al., 2009; de Moel and Aerts, 2011), in which the uncertainty added by the hydrodynamic modelling was identified as the smallest among different uncertainty sources, the forecast uncertainty added by the hydrodynamic modelling can be assumed small compared to the uncertainty that is contained in the water level forecast. This uncertainty can be further reduced if the hydrodynamic model is not set up ad hoc and run uncalibrated as in this feasibility study but set up considering the bathymetry in more detail and also calibrated or validated against historic floods events. Such a more detailed implementation could be easily achieved with the local knowledge of the responsible authorities of the river reaches.

The evaluation of the forecast skill, e.g. by a receiver operating characteristic (ROC) curve, and score would also be desirable, but considering the rarity of such extreme and documented flood events at a particular river reach, it is practically not possible to evaluate forecast skill of the whole forecast chain (meteorology–hydrology–hydraulics) simply because of lack of event data.

For the use in an operational setting, it is also advisable to provide uncertainty maps derived from an ensemble of hydrological forecasts to map the uncertainties inherent in the meteorological forecasts and the hydrological modelling. Because of the computational efficiency of RIM2D these hydrological forecast ensembles can be transferred into probabilistic inundation maps mapping the consequences of the uncertainties in the meteorological and hydrological forecasts as uncertainty in the inundation forecast. However, currently the hydrological forecasts for river gauges in Germany do not provide this kind of information but only the most likely flood hydrograph, which was used for the flood simulation shown in Fig. 2a.

5 Conclusions

The recent flood disaster in western Germany in July 2021 is used to demonstrate the potential benefits of flood impact forecasting. We show that the simplified and easily set up hydrodynamic model RIM2D that uses readily available data delivers plausible inundation areas, depth, and flow velocity simulations in runtimes enabling forecasts and early warning. Moreover, additional impact indicators identifying dangerous hotspots, for instance, in terms of expected building collapse, persons drowning, or floating and toppling cars, can be derived. Such detailed and location-specific information on expected impacts is highly valuable for a targeted and spatially explicit flood disaster management. It also allows more meaningful warnings for the population compared to the standard, gauge-based water level forecast. We argue that the use of this information can substantially improve the current disaster management and warning response. People's lives can be saved even if the hydrological water level forecasts underestimate the actual event, as was the case in the July 2021 event (see Fig. 2a). We believe that the disaster management could have been more targeted than it actually was if the information provided by the simulation based on the hydrological forecast as shown in Fig. 2a would have been available prior to the event. Moreover, it can be hypothesized that the early warning would have made a deeper impact in the affected population and the disaster management units, thus likely reducing the extraordinary high number of fatalities, if communicated in due time.

Due to the model implementation of RIM2D on graphical processor units dedicated for massive parallel computing, the simulation runtimes are in a range suitable for inclusion in an operational flood early warning system. As the model setup is simple and the required data are readily available in many countries, the model can be widely transferred and used. The required hardware environment for the simulation is affordable at low costs, particularly in comparison with large-scale computational clusters. This facilitates implementation of the model at flood forecast centres without considerable investments into large-scale IT infrastructure. Based on the validity of the simulations, the ease of implementing hydraulic forecast models, and the speed of the simulations, we argue that the current forecast practices should be extended with impact forecast models as presented here in order to improve the efficiency of flood warnings and management. The scientific basis, methods, and models for these impact forecasts are ripe for implementation in operational forecast systems.

Code availability. RIM2D is available for non-commercial use at https://git.gfz-potsdam.de/hydro/rfm/rim2d (last access: 8 September 2022). Currently, access has to be granted by the maintainer of the repository (corresponding author). RIM2D will be fully opensource for non-commercial use in the near future.

Data availability. DEM 10 m available from is 2012 © GeoBasis-DE/BKG (https://gdz.bkg.bund.de/index. php/default/digitale-geodaten/digitale-gelandemodelle/ digitales-gelandemodell-gitterweite-10-m-dgm10.html, Bundesamt für Kartographie und Geodäsie, 2022); OSM river network and buildings are available from © OpenStreetMap contributors 2021, distributed under the Open Data Commons Open Database License (ODbL) v1.0. The OpenStreetMap data and the CORINE land use data are open-source. The digital elevation model of the BKG is available upon request for non-commercial use.

Supplement. The supplement related to this article is available online at: https://doi.org/10.5194/nhess-22-3005-2022-supplement.

Author contributions. HA developed RIM2D, set up the Ahr river model, ran the simulations, evaluated the results, and designed and contributed to the manuscript. SV and BM contributed to the RIM2D code development and the design of the study and wrote parts of the manuscript.

Competing interests. The contact author has declared that none of the authors has any competing interests.

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Special issue statement. This article is part of the special issue "Advances in flood forecasting and early warning". It is not associated with a conference.

Acknowledgements. The authors would like to acknowledge the support of the Landesumweltamt Rheinland-Pfalz for providing the boundary conditions for the simulations, the mapped inundation extent, and informal information on the flood event in the Ahr valley in 2021.

Financial support. This research has been supported by the Helmholtz-Gemeinschaft (Klima-Initiative) and the Bundesministerium für Bildung und Forschung (KAHR projekt (grant no. FKZ 01LR2102F)).

The article processing charges for this open-access publication were covered by the Helmholtz Centre Potsdam – GFZ German Research Centre for Geosciences.

Review statement. This paper was edited by Jie Yin and reviewed by two anonymous referees.

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